SHEET LIST

SHEET NUMBER	SHEET NAME
S000	COVER PAGE
S100	GENERAL NOTES
S101	GENERAL NOTES
S102	GENERAL NOTES
S103	TYPICAL DETAILS
S104	TYPICAL DETAILS
S105	TYPICAL DETAILS
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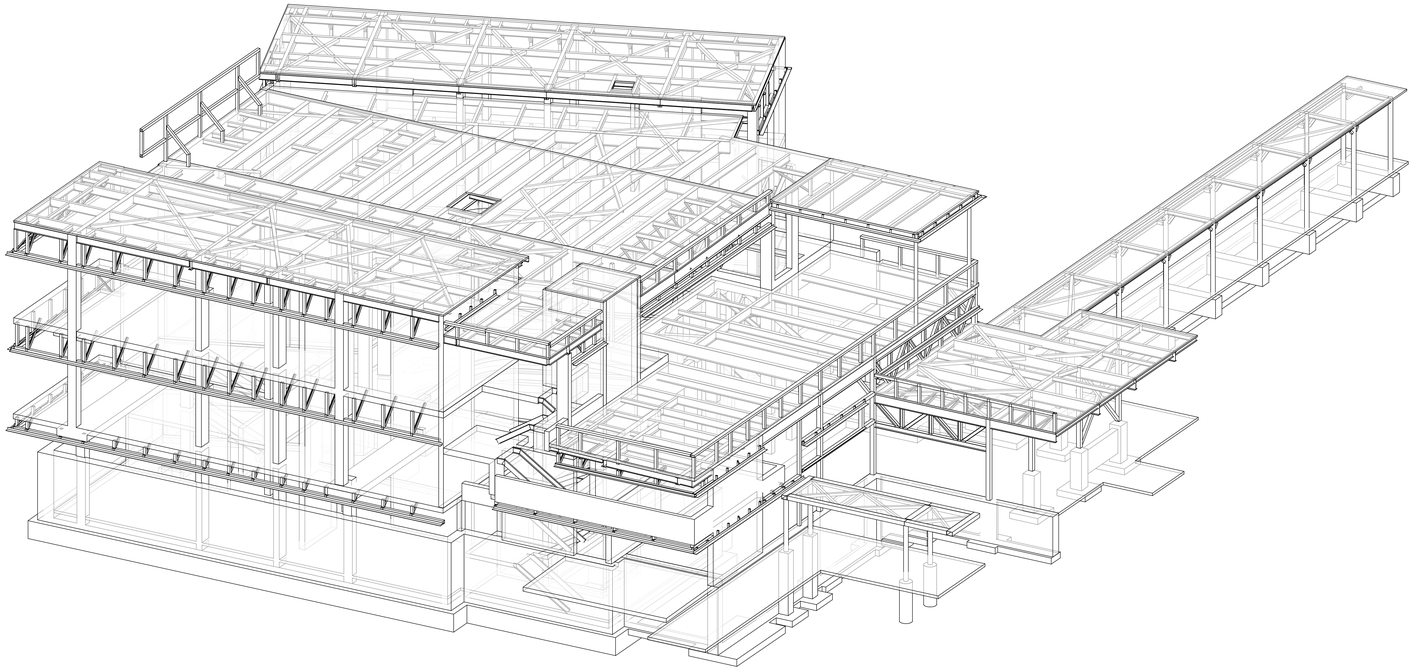
S107	TYPICAL DETAILS
S108	TYPICAL DETAILS
S109	TYPICAL DETAILS
S110	TYPICAL DETAILS
S111	TYPICAL DETAILS
S112	TYPICAL DETAILS
S113	TYPICAL DETAILS
S120	MEZZANINE AND SECOND FLOOR ROOF UPLIFT DIAGRAM
S121	ROOF UPLIFT DIAGRAM
S200	FOUNDATION AND LOWER LEVEL FRAMING PLAN
S210	GROUND FLOOR FRAMING PLAN
S220	UPPER GROUND FLOOR FRAMING PLAN
S230	MEZZANINE LEVEL FRAMING PLAN
S240	SECOND FLOOR FRAMING PLAN
S250	LOW ROOF FRAMING PLAN
S260	HIGH ROOF FRAMING PLAN
S300	COLUMN SCHEDULE
S301	COLUMN SCHEDULE
S310	SCHEDULES
S400	SHEAR WALL KEY PLAN AND SCHEDULE
S401	SHEAR WALL ELEVATIONS
S402	SHEAR WALL ELEVATIONS
S403	SHEAR WALL ELEVATIONS
S406	FRAMING ELEVATIONS
S410	BUILDING ELEVATIONS
S411	BUILDING ELEVATIONS
S412	BUILDING ELEVATIONS
S500	FOUNDATION SECTIONS
S501	FOUNDATION SECTIONS
S502	FOUNDATION SECTIONS
S503	FOUNDATION SECTIONS
S510	GROUND FLOOR SECTIONS
S511	GROUND FLOOR SECTIONS
S520	MEZZANINE FLOOR SECTIONS
S521	MEZZANINE FLOOR SECTIONS
S530	SECOND FLOOR SECTIONS
S531	SECOND FLOOR SECTIONS
S540	ROOF FLOOR SECTIONS
S541	ROOF FLOOR SECTIONS
S600	STAIR PLANS AND SECTIONS
S601	STAIR PLANS AND SECTIONS
S602	STAIR PLANS AND SECTIONS



3D MODEL DISCLAIMER

ELEMENTS REPRESENTED IN 3D MODEL VIEW ARE SHOWN FOR INFORMATION PURPOSES ONLY AND MAY NOT REPRESENT DESIGN INTENT. THIS VIEW IS DIAGRAMATIC ONLY, AND NOT FOR CONSTRUCTION

Key to Detail Location

NO. — Detail Number
NO. — Drawing Number

If this sheet is not 46 7/8" x 33 1/8" (1189 x 841 mm) It is a reduced print - Read dwg. accordingly.

Contractors must check and verify all dimensions on the job and report any discrepancies to the Architect before proceeding with the work.

All prints and specifications are the property of the Architect and must be returned at the completion of the work.

Drawings should not be scaled.

No	Date	Revision/Issued
1	2025-12-15	ISSUED FOR 30% CONSTRUCTION DOCUMENTS
2	2026-03-13	ISSUED FOR BUILDING PERMIT
3	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

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2026-07-08
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COVER PAGE

scale:	1 : 1
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S000
	Revision: 5

A0 FILENAME: Autodesk Docs://2032 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-03 3:04:49 PM

GENERAL NOTES

GN-001
January 7, 2022

1. PROVIDE ALL MATERIAL AND LABOUR REQUIRED FOR THE COMPLETION OF THE WORK. BREAKDOWN OF WORK BY TRADE IS FOR GUIDANCE ONLY AND IS NOT NECESSARILY COMPLETE.
2. THIS IS A METRIC PROJECT. ALL LENGTHS ARE IN MILIMETERS, UNLESS OTHERWISE NOTED.
3. DO NOT SCALE THESE DRAWINGS.
4. COMPLY WITH THE REQUIREMENTS OF THE ONTARIO BUILDING CODE AND THE OCCUPATIONAL HEALTH AND SAFETY ACT AND REGULATIONS FOR CONSTRUCTION PROJECTS.
5. REVIEW STRUCTURAL DRAWINGS IN CONJUNCTION WITH THE DRAWINGS PREPARED BY ALL CONSULTANTS PRIOR TO CONSTRUCTION. REPORT DISCREPANCIES BEFORE PROCEEDING WITH THE WORK.
6. FOLLOW TYPICAL DETAILS (I.E. SD-001) SHOWN ON DRAWING(S) S103-S113, UNLESS OTHERWISE NOTED ON DRAWINGS. TYPICAL DETAILS SHOW STRUCTURAL INTENT RATHER THAN ACTUAL CONDITIONS FOR THIS PROJECT. DETAILS NOTED IN TYPICAL DETAILS APPLY TO SIMILAR CONDITIONS THROUGHOUT THE PROJECT UNLESS OTHERWISE NOTED.
7. ALL SECTIONS, DETAILS, AND STATEMENTS NOTED AS "TYPICAL" APPLY TO LIKE/SIMILAR CONDITIONS IN THE STRUCTURE.
8. COORDINATE ALL OPENINGS, SLEEVES AND EMBEDDED ITEMS IN STRUCTURE WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND LANDSCAPE DRAWINGS. REPORT ANY ITEMS THAT ARE IN CONFLICT.
9. DO NOT CUT OR DRILL ANY OPENINGS IN STRUCTURAL MEMBERS WITHOUT WRITTEN PERMISSION FROM THE STRUCTURAL CONSULTANT UNLESS SPECIFICALLY NOTED ON THE STRUCTURAL DRAWINGS.
10. USE OF THESE DRAWINGS IS LIMITED TO THAT IDENTIFIED IN THE REVISIONS COLUMN. DO NOT CONSTRUCT FROM THESE DRAWINGS UNLESS MARKED "ISSUED FOR CONSTRUCTION".
11. DO NOT USE INFORMATION ON THESE DRAWINGS FOR ANY OTHER PROJECT OR WORKS.

TEMPORARY WORKS

GN-002
December 22, 2021

1. DRAWINGS SHOW COMPLETED STRUCTURE ONLY. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE SCOPE, DESIGN AND ERECTION OF ALL TEMPORARY WORKS THAT ARE NECESSARY TO COMPLETE THE WORK. FULL DESIGN LOADS MAY ONLY BE APPLIED AFTER THE CONCRETE REACHES ITS DESIGN STRENGTH.
2. CONSTRUCTION LOADS ON PERMANENT STRUCTURE SHALL NOT EXCEED DESIGN LOADS INDICATED IN THE CONTRACT DOCUMENTS.
3. DESIGN AND FIELD REVIEW OF ALL TEMPORARY WORKS TO BE CARRIED OUT BY A PROFESSIONAL ENGINEER RETAINED BY THE CONTRACTOR, LICENSED AND INSURED IN THE PROVINCE OF ONTARIO.
4. ANCHOR RODS AND OTHER EMBEDDED ITEMS HAVE BEEN DESIGNED FOR LOADS OF THE COMPLETED STRUCTURE ONLY AND ARE NOT TO BE USED OR RELIED UPON FOR TEMPORARY SUPPORT OR STABILITY DURING ERECTION UNLESS REVIEWED AND APPROVED BY THE CONTRACTOR'S ENGINEER RESPONSIBLE FOR THE ERECTION PROCEDURES.
5. MAKE GOOD ALL EXISTING WORK DISTURBED BY TEMPORARY CONSTRUCTION OPERATIONS.
6. EXTENT OF ALL TEMPORARY SHORING WHICH MAY BE REQUIRED IS NOT SHOWN ON STRUCTURAL DRAWINGS, CONTRACTOR TO DETERMINE. REFER TO SPECIFICATIONS FOR TEMPORARY SHORING REQUIREMENTS.

SUBMITTALS AND SHOP DRAWINGS

GN-003
March 8, 2022

1. SUBMIT FOR REVIEW ALL SUBMITTALS AND SHOP DRAWINGS REQUIRED BY THE SPECIFICATIONS, INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
   A. CONCRETE MIX DESIGNS
   B. PROPOSED CONSTRUCTION AND CONTROL JOINT LOCATIONS IN CONCRETE ELEMENTS
   C. CONCRETE AND MASONRY REINFORCING STEEL.
   D. STRUCTURAL STEEL (ERECTION DRAWINGS AND DETAILS SEALED AND SIGNED BY A PROFESSIONAL ENGINEER)
   E. STEEL JOISTS (ERECTION DRAWINGS, DETAILS AND CALCULATIONS SEALED AND SIGNED BY A PROFESSIONAL ENGINEER)
   F. STEEL DECK (SEALED AND SIGNED BY A PROFESSIONAL ENGINEER)
2. REVIEW OF SHOP DRAWINGS IS ON A RATIONAL SAMPLING BASIS, FOR GENERAL CONFORMITY WITH THE INTENT OF THE STRUCTURAL CONTRACT DOCUMENTS. IT IS NOT A DETAILED CHECK AND MUST NOT BE CONSTRUED AS RELIEVING THE CONTRACTOR OF RESPONSIBILITY FOR MAKING THE WORK ACCURATE AND IN CONFORMITY WITH THE CONTRACT DOCUMENTS.
3. REVIEW OF SHOP DRAWINGS DOES NOT IMPLY ANY CHANGE IN ANY OTHER CONSULTANTS' OR PROFESSIONALS' RESPONSIBILITIES RELATED TO DESIGN OF SPECIFIC ITEMS AS OUTLINED BY THE SPECIFICATIONS.
4. REVIEW OF A SPECIFIC COMPONENT ON NON-STRUCTURAL SHOP DRAWINGS (COMPONENTS ENGINEERED BY OTHERS) DOES NOT INCLUDE REVIEW OF THE ASSEMBLY OF WHICH THE ITEM MAY BE A COMPONENT.
5. IF REQUIRED, CAD FILES OF THE FULL SET OF STRUCTURAL DRAWINGS ARE AVAILABLE "AS-IS", AT A COST OF $100.00 PER SHEET FOR USE IN THE PREPARATION OF SHOP DRAWINGS, PROVIDED THAT THE OWNER AND THE OWNER'S CONSULTANTS ARE NOT HELD RESPONSIBLE FOR ANY ERRORS OR OMISSIONS ON THE DRAWINGS. THESE CAD DRAWINGS ARE NOT TO BE SCALED.
6. ALLOW A MINIMUM OF 10 WORKING DAYS FOR REVIEW OF EACH SUBMISSION OF SHOP DRAWINGS IN THE STRUCTURAL OFFICE. ALLOW MORE TIME WHEN LARGE QUANTITIES OF SHOP DRAWINGS ARE SUBMITTED ALONG WITH A PRIORITY LIST OF SUBMITTALS. SUBMIT IN ORDER WITH THE INTENDED SEQUENCE OF CONSTRUCTION.
7. MAKE CORRECTIONS REQUIRED BY PREVIOUS REVIEW BEFORE RESUBMITTING DRAWINGS. CLEARLY INDICATE ALL CHANGES AND ADDITIONS TO PREVIOUS SUBMISSION. DO NOT ADD NEW DETAILS TO DRAWINGS WHICH HAVE BEEN STAMPED AS REVIEWED OR NOTED.
8. AFTER REVIEW, THE DRAWINGS WILL BE STAMPED AND RETURNED TO SHOW ONE OF THE FOLLOWING:

REVIEWED	RELEASED FOR FABRICATION
REVIEWED AS NOTED	RELEASED FOR FABRICATION AFTER REVISIONS NOTED ARE MADE
REVISE AND RESUBMIT	CORRECT AND RESUBMIT FOR REVIEW PRIOR TO FABRICATION.
NOT REVIEWED	SHOWS WORK WHICH IS NOT WITHIN THE SCOPE OF STRUCTURAL CONSULTING SERVICES.

9. ALL SHOP DRAWINGS SHALL BEAR THE GENERAL CONTRACTOR'S REVIEW STAMP. SHOP DRAWINGS NOT STAMPED BY THE GENERAL CONTRACTOR WILL BE MARKED "REVISE AND RESUBMIT" AND RETURNED WITHOUT BEING REVIEWED.
10. SUBMIT ENGINEER SEALED SHOP DRAWINGS WHEN REQUIRED BY SPECIFICATIONS. SHOP DRAWINGS NOT SEALED, SIGNED, AND DATED WILL BE MARKED "REVISE AND RESUBMIT" AND RETURNED WITHOUT BEING REVIEWED.
11. DO NOT COMMENCE FABRICATION UNTIL RETURNED SHOP DRAWINGS HAVE BEEN EXAMINED.
12. PROVIDE FINAL RECORD DRAWINGS AFTER ALL CORRECTIONS ARE MADE.

QUALITY ASSURANCE

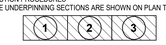
GN-004
March 8, 2022

1. COOPERATE WITH CONSULTANTS DOING GENERAL REVIEW AND WITH ALL INSPECTION AND TESTING AGENCIES. ASSIST IN THEIR WORK AND PROVIDE SAFE ACCESS TO WORK AREAS AS REQUIRED. PROVIDE REASONABLE NOTICE (NOT LESS THAN TWO WORKING DAYS OR 48 HOURS, WHICHEVER IS GREATER.) PRIOR TO CONCEALING ANY WORK THAT REQUIRES INSPECTION, REVIEW, OR TESTING. SCHEDULE THIS WORK TO OCCUR DURING NORMAL BUSINESS HOURS. WORK TO BE GENERALLY COMPLETE AT THE TIME OF THE REVIEW.
2. THE STRUCTURAL CONSULTANT WILL PROVIDE PERIODIC GENERAL REVIEW OF A REPRESENTATIVE SAMPLE OF THE STRUCTURAL WORKS DESIGNED BY THE CONSULTANT TO CHECK GENERAL CONFORMANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS. THESE REVIEWS DO NOT ALLEVIATE THE CONTRACTOR'S RESPONSIBILITY TO IMPLEMENT AND MAINTAIN AN APPROPRIATE QUALITY CONTROL PROGRAM.
3. ENSURE THAT CONSTRUCTION REVIEW IS PROVIDED FOR ALL STRUCTURAL WORK DESIGNED BY PROFESSIONAL ENGINEERS RETAINED BY THE CONTRACTOR. CONSTRUCTION REVIEW TO BE BY THE ENGINEERS RESPONSIBLE FOR THE DESIGN, OR BY OTHER PROFESSIONAL ENGINEERS LICENSED AND INSURED IN THE PROVINCE OF ONTARIO AND DESIGNATED BY THE ENGINEER RESPONSIBLE FOR THE DESIGN. SUBMIT CONSTRUCTION REVIEW REPORTS FOR THE CONSULANT'S RECORDS UPON REQUEST.
4. MAINTAIN A QUALITY CONTROL PLAN FOR STRUCTURAL WORK AND MAKE IT AVAILABLE TO THE CONSULTANT UPON REQUEST. AT A MINIMUM, THE PLAN TO INCLUDE:
   1. NAMES OF PERSONNEL RESPONSIBLE FOR EXECUTION OF THE PLAN.
   2. MEANS AND METHODS FOR CONFIRMING MATERIAL COMPLIANCE WITH SPECIFICATIONS AND ASSOCIATED DOCUMENTATION PROCEDURES.
   3. PROGRAM FOR CONFIRMING AND DOCUMENTING COMPLIANCE OF SUB-TRADE QUALIFICATIONS AND THEIR INDIVIDUAL EMPLOYEES AND SUB-CONTRACTORS.
   4. PROCEDURES FOR REVIEWING COMPLIANCE IN THE FIELD WITH CONSTRUCTION DOCUMENTS INCLUDING DOCUMENTATION OF LOCATIONS REVIEWED, PHOTOGRAPHS TAKEN, AND TIMING FOR REVIEW. THE CONTRACTOR'S REVIEW MUST BE COMPLETED PRIOR TO REVIEW OF THE CONSULTANT.
   5. PROCEDURES FOR RECTIFYING DEFICIENCIES NOTED BY THE CONTRACTOR, CONSULTANTS AND INDEPENDENT INSPECTION AGENCIES.

EXISTING STRUCTURES

GN-006
December 22, 2021

1. TAKE ALL PRECAUTIONS NECESSARY TO PROTECT EXISTING STRUCTURES FROM DAMAGE DURING CONSTRUCTION.
2. VERIFY EXISTING DIMENSIONS AND CONDITIONS ON SITE PRIOR TO CONSTRUCTION. EXISTING CONDITIONS ARE ASSUMED. REPORT ANY VARIATIONS TO THE STRUCTURAL CONSULTANT BEFORE PROCEEDING WITH THE WORK.
3. EXISTING STRUCTURAL INFORMATION IS BASED UPON DRAWINGS PREPARED BY [redacted]
4. INSTALL AND AFTERWARDS REMOVE ALL TEMPORARY SHORING AND BRACING REQUIRED TO ENSURE THE INTEGRITY OF THE EXISTING STRUCTURE DURING CONSTRUCTION.
5. ALL SHORING TO BE DESIGNED BY A PROFESSIONAL ENGINEER RETAINED BY THE CONTRACTOR, LICENSED AND INSURED IN THE PROVINCE OF ONTARIO. PREPARE SHORING DRAWINGS SIGNED AND SEALED BY THAT ENGINEER.
6. SHORING INDICATED ON THE STRUCTURAL DRAWINGS DOES NOT REMOVE THE CONTRACTOR'S RESPONSIBILITY FOR DETERMINING WHERE SHORING WILL BE REQUIRED. INSTALL ALL MEMBERS AS REQUIRED, INCLUDING NEEDLING AND BRACING.
7. ASSESS CONSTRUCTION LOADS APPLIED TO EXISTING STRUCTURES. PROVIDE ADEQUATE SHORING IF THEY EXCEED THE EXISTING BUILDING DESIGN LOADS.
8. MAKE GOOD ALL EXISTING WORK DISTURBED BY THE SHORING OPERATIONS, EXCAVATION AND OTHER CONSTRUCTION PROCEDURES
9. CONCRETE UNDERPINNING SECTIONS ARE SHOWN ON PLAN THUS:



   A. INSTALL ALL NO. 1 SECTIONS FIRST, AND THEN FOLLOW WITH ALL NO. 2 SECTIONS, ETC.
   B. PROVIDE 50 (2") OF DRY PACK GROUT BETWEEN EXISTING FOUNDATION AND TOP OF UNDERPINNING, NO SOONER THAN 24 HOURS AFTER UNDERPINNING IS PLACED.
   C. ALLOW 12 HOURS AFTER GROUTING BEFORE INSTALLATION OF FOLLOWING SECTIONS.
   D. PREVENT LOSS OF MATERIAL FROM UNDERNEATH EXISTING STRUCTURE.
   E. MAKE GOOD ALL EXISTING WORK DISTURBED BY THE UNDERPINNING OPERATIONS.
   F. UNDERPINNING DETAILS AND PROCEDURE TO BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PLACE WHERE THE PROJECT IS LOCATED.
   G. SUBMIT STAMPED DOCUMENTS FOR REVIEW BY LEA.
   H. UNDERPINNING ENGINEER TO PROVIDE SITE REVIEW AND SUBMIT REPORTS FOR RECORD.
   I. PREVENT LOSS OF MATERIAL FROM UNDERNEATH EXISTING STRUCTURE.
   J. MAKE GOOD ALL EXISTING WORK DISTURBED BY THE UNDERPINNING OPERATIONS.
10. EXISTING CONCRETE STRUCTURE
   A. REMOVE PORTIONS OF EXISTING CONCRETE STRUCTURE AS REQUIRED.
   B. PROVIDE TEMPORARY SHORING AND BRACING AS REQUIRED.
   C. RETAIN A PROFESSIONAL ENGINEER TO DESIGN THE TEMPORARY SHORING AND BRACING AND TO REVIEW THIS WORK ON SITE.
   D. OBTAIN CONSULTANT'S APPROVAL BEFORE CORING OR CUTTING EXISTING SLABS, BEAMS OR WALLS.
   E. RETAIN AN INDEPENDENT TESTING COMPANY TO LOCATE EXISTING REINFORCEMENT AND CONDUIT IN THE AREAS OF PROPOSED OPENINGS AND TO MARK LOCATIONS ON THE SURFACES OF SLABS AND WALLS ON WHICH THE CORES AND CUTS ARE TO BE STARTED USING A NON-DESTRUCTIVE METHOD.
   F. REMOVE TOPPINGS PRIOR TO LOCATING REINFORCEMENT AND CONDUIT.
   G. MARK LOCATIONS AND SIZES OF CORES AND OPENINGS AND LOCATIONS OF REINFORCEMENT AND CONDUIT USING INDELIBLE MARKERS AS FOLLOWS:
      a. RED FOR TOP BARS
      b. GREEN FOR BOTTOM BARS
      c. BLACK FOR CORES, OPENINGS AND CONDUIT.
   H. RELOCATE PROPOSED OPENINGS AND REPEAT PROCESS AT NO EXTRA COST TO THE CONTRACT IF PROPOSED LOCATIONS ARE NOT ACCEPTABLE TO CONSULTANT.
   I. SAVE THE COMPLETE LENGTH OF ALL CORES. LABEL EACH CORE WITH LOCATION TAKEN. MAKE ALL CORES AVAILABLE FOR REVIEW BY CONSULTANT. DISPOSE OF CORES ONLY WITH APPROVAL OF CONSULTANT. SEE DETAILS ON STRUCTURAL DRAWINGS FOR SAWCUTTING PROCEDURE.
   J. IF NEW REINFORCEMENT IS REQUIRED AT AN OPENING, INSTALL REINFORCEMENT BEFORE CUTTING OPENING OR SHORE UP STRUCTURE UNTIL NEW REINFORCEMENT IS INSTALLED.
   K. ROUGHENING EXISTING SURFACES:
      a. WHERE DRAWINGS CALL FOR A ROUGHENED SURFACE, BUSH HAMMER ENTIRE SURFACE TO A FULL AMPLITUDEOF AT LEAST 5MM.
   L. PATCHING: PATCH EXISTING CONCRETE WHERE NECESSARY TO PROVIDE REQUIRED SMOOTH, FLAT SURFACES FOR REINFORCEMENT AND FOR OTHER TRADES.
11. EXISTING STEEL STRUCTURE
   A. MODIFICATION / REMOVAL OF EXISTING STEEL WORK
      a. A SET OF STRUCTURAL DRAWINGS OF THE EXISTING BUILDING MAY BE VIEWED AT THE OFFICES OF THE CONSULTANT OR THE STRUCTURAL ENGINEER.
      b. DISMANTLE AND CUT EXISTING STRUCTURAL STEEL AS REQUIRED. PROVIDE TEMPORARY SHORING AND BRACING REQUIRED FOR THESE OPERATIONS. RETAIN A PROFESSIONAL ENGINEER TO DESIGN THE TEMPORARY SHORING AND TO REVIEW THIS WORK ON SITE.
      c. REMOVE FROM SITE EXISTING STEEL WHICH IS DISMANTLED BUT NOT DESIGNATED FOR REUSE. IT SHALL BECOME THE PROPERTY OF THE CONTRACTOR.
   B. CLEAN EXISTING STRUCTURAL STEEL, WHICH IS AFFECTED BY THE WORK AND IS TO REMAIN IN PLACE, DOWN TO BARE METAL, PRIOR TO ITS INSPECTION SO THAT ITS CONDITION MAY BE ASCERTAINED. NOTIFY CONSULTANT WHEN MEMBERS ARE READY FOR INSPECTION.

FOUNDATIONS

GN-007
December 22, 2021

1. STRUCTURAL DESIGN IS BASED ON THE GEOTECHNICAL REPORT PREPARED BY ENGLOBE REPORT NO. 02210387.006 DATED JANUARY 19, 2026 AND ENGLOBE GEOTECHNICAL LETTER (MASW TEST, REFERENCE NO. 02210387.007) DATED FEBRUARY 25, 2026.
2. REFER TO THE GEOTECHNICAL REPORT FOR INFORMATION ON GEOTECHNICAL CONDITIONS, FOUNDATION RECOMMENDATIONS, AND FOR ALL EARTHWORK INCLUDING EXCAVATION, BACKFILL, AND SUBGRADE PREPARATION. ALSO REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS
3. CONSTRUCT ALL FOOTINGS ON STRATA CAPABLE OF SUPPORTING THE DESIGN BEARING RESISTANCE NOTED:
   - 300 kPa AT ULS (ULTIMATE LIMIT STATES DESIGN)
   - 200 kPa AT SLS (SERVICEABILITY LIMIT STATES DESIGN)

   FOR RAFT FOUNDATION THE DESIGN BEARING RESISTANCE NOTED:
   - 250 kPa AT ULS (ULTIMATE LIMIT STATES DESIGN)
   - 150 kPa AT SLS (SERVICEABILITY LIMIT STATES DESIGN)
4. FOR FROST PROTECTION, MINIMUM DISTANCE FROM FINISHED GRADE TO UNDERSIDE OF PERIMETER FOOTINGS AND FOOTINGS IN UNHEATED AREAS TO BE NOT LESS THAN 1200.
5. BEARING ELEVATIONS SHOWN ON STRUCTURAL DRAWINGS ARE ANTICIPATED MINIMUM DEPTHS. FINAL BEARING ELEVATIONS AND CAPACITY MUST BE REVIEWED AND APPROVED BY THE GEOTECHNICAL CONSULTANT PRIOR TO CONSTRUCTING FOOTINGS. ANY DEVIATIONS FROM THE CONTRACT DOCUMENTS TO BE REVIEWED WITH THE STRUCTURAL CONSULTANT PRIOR TO PLACING FORMWORK.
6. MODULUS OF SUBGRADE REACTION FOR DESIGN OF RAFT SLAB IS 6000 kPa/m FOR 25mm. MODULUS OF SUBGRADE REACTION FOR DESIGN OF SLAB ON GRADE IS 18000 kPa/m. CONSTRUCT SUBGRADE UNDER THE SLAB ON GRADE IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.
7. CENTRE FOOTINGS AND PIERS UNDER CENTROID OF COLUMNS, OR WHERE THERE ARE NO COLUMNS ABOVE, CENTER UNDER WALLS OR GRADE BEAMS, UNLESS OTHERWISE NOTED.
8. UNLESS OTHERWISE NOTED, THE LONGER DIMENSION OF RECTANGULAR SPREAD FOOTINGS TO BE PARALLEL TO THE LONGER COLUMN DIMENSION.
9. LOCATE ALL EXISTING UNDERGROUND SERVICES PRIOR TO EXCAVATION.
10. THE LINE OF SLOPE BETWEEN ADJACENT EXCAVATIONS FOR FOOTINGS OR TRENCHES SHALL NOT EXCEED A RISE OF 7 IN A RUN OF 10, UNLESS OTHERWISE APPROVED BY THE GEOTECHNICAL CONSULTANT. REFER TO TYPICAL DETAILS.
11. KEEP EXCAVATION DRAINED AND FREE OF WATER AT ALL TIMES. REFER TO GEOTECHNICAL REPORT FOR REQUIRED DEWATERING PROCESS.
12. PROTECT FOOTINGS, WALLS, SLABS-ON-GRADE AND ADJACENT SOIL AGAINST FREEZING AND FROST ACTION AT ALL TIMES DURING CONSTRUCTION. DO NOT POUR CONCRETE AGAINST FROZEN EARTH.
13. DO NOT USE EARTH FORMS UNLESS APPROVED THE GEOTECHNICAL ENGINEER AND THE STRUCTURAL CONSULTANT. FOR ELEMENTS CAST AGAINST SOIL, INCREASE SIZE SHOWN ON THE DRAWINGS AS REQUIRED TO OBTAIN 75 (3") CONCRETE COVER AGAINST SOIL.
14. DO NOT BACKFILL AGAINST WALLS RETAINING EARTH UNTIL ELEMENTS PROVIDING LATERAL SUPPORT, INCLUDING SLABS ON GRADE AND SUSPENDED LEVELS, ARE COMPLETED AND CONCRETE HAS REACHED 70% OF ITS DESIGN STRENGTH.
15. FOR ELEMENTS THAT ARE TO BE BACKFILLED ON BOTH SIDES, PLACE BACKFILL SIMULTANEOUSLY ON BOTH SIDES SUCH THAT HEIGHTS DO NOT VARY BY MORE THAN 300 (1'-0") FROM ONE SIDE TO THE OTHER.
16. FORM ALL SIDES OF FOOTINGS UNLESS OTHERWISE NOTED OR APPROVED BY GEOTECHNICAL ENGINEER. INCREASE COVER TO REINFORCING.

DESIGN CRITERIA

GN-005
JANUARY 1, 2025

1. STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE 2024 ONTARIO BUILDING CODE (OBC) SUPPLEMENTED BY THE 2020 NATIONAL BUILDING CODE OF CANADA STRUCTURAL COMMENTARY.
2. CLIMATIC DATA USED TO DETERMINE THE DESIGN LOADS IS FROM THE MMAH SUPPLEMENTARY STANDARD OF THE OBC SPECIFICALLY FOR TORONTO (NORTH YORK).
3. DESIGN LOADS SHOWN ON PLAN OR SECTION ARE SPECIFIED (UNFACTORED) LOADS TO BE USED FOR ULS DESIGN, UNLESS OTHERWISE NOTED. FOR POINT LOADS, CONSIDER IT A LIVE LOAD IF ONLY ONE LOAD IS GIVEN. LOAD FACTORS TO CONVERT LOADS TO SLS FOR WIND AND SNOW ARE SHOWN IN THEIR RESPECTIVE SECTIONS.
4. SELF WEIGHT IS DUE TO THE WEIGHT OF THE STRUCTURE ITSELF. IT VARIES WITH THE STRUCTURAL SYSTEM AND INCLUDES CONCRETE TOPPINGS ON STEEL DECK.
5. SUPERIMPOSED DEAD LOADS (SDL) ARE NON-STRUCTURAL DEAD LOADS DUE TO NON-STRUCTURAL TOPPINGS, FINISHES, PARTITIONS, ROOFING MATERIALS, SUSPENDED EQUIPMENT, PAVERS, SOIL, ETC.
6. DEAD LOAD (DL) IS THE SELF WEIGHT OF THE STRUCTURE PLUS THE SUPERIMPOSED DEAD LOAD.
7. GRAVITY LOADS ARE SHOWN ON PLANS. LIVE LOAD REDUCTION HAS NOT BEEN USED.
8. NET FACTORED UPLIFT LOADS ON ROOFS ARE SHOWN ON PLANS.
9. FIRE
   A. STRUCTURAL DESIGN ASSUMES NON-LOAD RESTRICTED ULC FIRE RATED ASSEMBLIES, AND APPROPRIATE MATERIALS MUST BE USED
10. IMPORTANCE: HIGH
11. **LOADS DUE TO SNOW AND RAIN**
   SNOW AND ASSOCIATED RAIN (S) – OBC CLAUSE 4.1.6.2
   MINIMUM SPECIFIED SNOW LOAD, $S=I_sS_s(C_bC_wC_sC_a)+S_r$]= 1.56 kN/m²

IMPORTANCE FACTOR =	I_s =	ULS = 1.15 SLS = 0.9	
GROUND SNOW LOAD (1-IN-50 YEAR)	S_s =	1.20 kN/m²	
BASIC ROOF FACTOR =	C_b =	0.80	
WIND EXPOSURE FACTOR =	C_w =	1.0	
SLOPE FACTOR =	C_s =	1.0(MIN)	SEE PLAN FOR ROOF SPECIFIC LOADS
ACCUMULATION FACTOR =	C_a =	1.0(MIN)	SEE PLAN FOR ROOF SPECIFIC LOADS
ASSOCIATED RAIN LOAD =	S_r =	0.4 kN/m²	

RAIN (S) AND ROOF DRAINAGE– OBC CLAUSE 4.1.6.4
24 HOUR RAINFALL = 108 mm
ROOFS HAVE BEEN DESIGNED FOR THE WORST CASE OF SNOW OR RAIN, AS PER OBC 4.1.6.4(3) & (4).
ROOF DRAINAGE SYSTEM:
1) RAIN LOAD IS DETERMINED FOR FLOW CONTROL ROOF DRAINS THAT MEET THE REQUIREMENTS OBC 4.1.6.4 (4) AND OBC 7.4.10.4. INCLUDING EMERGENCY SCUPPERS AT 150mm ABOVE THE HIGH POINT OF THE ROOF. THE ROOF STRUCTURE MUST INCLUDE THE LOAD ACCUMULATED BY A 24H RAINFALL, BUT RAIN AND SNOW LOADS DO NOT NEED TO BE CONSIDERED TO ACT SIMULTANEOUSLY TO A MAXIMUM OF 180mm ACCUMULATED WATER.

12. **LATERAL LOADS**
   LATERAL FORCE RESISTING SYSTEM:
   - CONVENTIONAL CONSTRUCTION CONCRETE SHEAR WALLS WITH HORIZONTAL CONCRETE SLAB DIAPHRAGM
   - CONVENTIONAL CONSTRUCTION STEEL BRACED FRAME WITH HORIZONTAL CONCRETE ON STEEL DECK
   - CONVENTIONAL CONSTRUCTION STEEL BRACED FRAME WITH HORIZONTAL CONCRETE ON STEEL DECK AND HORIZONTAL BRACING

WIND (W) – OBC 4.1.7.1- STATIC PROCEDURE
MINIMUM SPECIFIED BUILDING WIND LOAD AT GRADE, $P=I_wq_hC_eC_gC_pC_a$ = 0.94 kPa
INTERNAL PRESSURE: CATEGORY 2 (OBC 4.1.7.7)

IMPORTANCE FACTOR =	I_w =	ULS = 1.15 SLS = 0.75	OBC 4.1.7.3.
REFERENCE VELOCITY PRESSURE (1-IN-50 YEAR)	q =	0.44 kPa	OBC 4.1.7.3.
EXPOSURE FACTOR (TOP OF BUILDING) =	C_e =	1.04 (MAX)	OBC 4.1.7.3. OR ***4.1.7.8***
TOPOGRAPHIC FACTOR =	C_t =	1.0	OBC 4.1.7.4.
GUST EFFECT FACTOR =	C_g =	2.0	OBC 4.1.7.3. OR ***4.1.7.8***
EXTERNAL PRESSURE COEFFICIENT (WINDWARD)	C_p =	0.621	OBC 4.1.7.5.
EXTERNAL PRESSURE COEFFICIENT (LEEWARD)	C_p =	0.319	OBC 4.1.7.5.
FACTORED SHEAR TO LATERAL LOAD RESISTING SYSTEM(NORTH-SOUTH)	V_{W_NS} =	400 kN	
FACTORED SHEAR TO LATERAL LOAD RESISTING SYSTEM (EAST-WEST)	V_{W_EW} =	425 kN	

EARTHQUAKE LOADS AND EFFECTS (E) - OBC 4.1.8
SEISMIC PARAMETERS

IMPORTANCE FACTOR =	I_E =	1.30	OBC 4.1.8.5
PROBABILITY OF EXCEEDANCE (IN 50 YEARS)		2%	4.1.8.4 (1)

SITE PROPERTIE

AVERAGE SHEAR WAVE VELOCITY =	V_{s30} =	453 m/s	OBC 4.1.8.4 (2)
SITE CLASS	S =	C	OBC 4.1.8.4 (3)
SITE DESIGNATION	X	X_C	
PEAK GROUND ACCELERATION	PGA(X)	0.154	OBC 4.1.8.4 (3)
PEAK GROUND VELOCITY	PGV(X)	0.109	OBC 4.1.8.4 (3)

2% PROBABILITY OF EXCEEDANCE (5% DAMPED) SPECTRAL ACCELERATION VALUES

PERIOD, T (SECONDS)	S(T, X)	DESIGN SPECTRAL ACCELERATION, S(T)
<=0.2	Sa(0.2, X) =	0.310
0.5	Sa(0.5, X) =	0.166
1.0	Sa(1.0, X) =	0.0863
2.0	Sa(2.0, X) =	0.0396
5.0	Sa(5.0, X) =	0.0103
10.0	Sa(10.0, X) =	0.00357

10% PROBABILITY OF EXCEEDANCE (5% DAMPED) SPECTRAL ACCELERATION VALUES

PERIOD, T (SECONDS)	S(T, X)	DESIGN SPECTRAL ACCELERATION, S(T)
<=0.2	Sa(0.2, X) =	0.102
0.5	Sa(0.5, X) =	0.0586
1.0	Sa(1.0, X) =	0.0299
2.0	Sa(2.0, X) =	0.0131
5.0	Sa(5.0, X) =	0.003
10.0	Sa(10.0, X) =	0.00107

SEISMIC LOAD AND PERFORMANCE

$I_ES(0.2)$ =	0.403		OBC 4.1.8.5
$I_ES(1.0)$ =	0.1122		OBC 4.1.8.5
SEISMIC CATEGORY	SC3		OBC 4.1.8.5
IRREGULARITIES	NOT APPLICABLE		OBC 4.1.8.9
METHOD OF ANALYSIS	STATIC	STRUCTURE HEIGHT < 60m AND Ta < 2s	OBC 4.1.8.7

		BUILDING	
DIRECTION		**NORTH-SOUTH**	**EAST-WEST**
SFRS DUCTILITY RELATED FORCE MODIFICATION FACTORS	R_d =	1.5	1.5
SFRS OVERSTRENGTH RELATED FORCE MODIFICATION FACTORS	R_O =	1.3	1.3
FUNDAMENTAL LATERAL PERIOD	Ta =	0.31 s	0.31 s
MAX. HIGHER MODE FACTOR	M_V =	1.0	1.0
TORSIONAL SENSITIVITY	B =	<=1.7	<=1.7
DESIGN BASE SHEAR	V = MAX BETWEEN 1. (2/3) x S(0.2) x I_E x W / (R_d x R_o) 2. S(0.5) x I_E x W / (R_d x R_o)	2700 kN	2700 kN
	V = $S(T_a)$ x M_v x I_E x W / (R_d x R_o)	2960 kN	2960 kN
	V_{MIN} = S(4.0) x M_v x I_E x W / (R_d x R_o)	185 kN	185 kN

MAX. INTERSTOREY DRIFT RATIO: 0.005xHs = 60mm USING Ie=1.0 AND RdRo=1.0
FOR 10% PROBABILITY OF EXCEEDANCE SPECTRAL VALUES

CONCRETE

GN-008
December 22, 2021

1. CONFORM TO CSA A23.1: "CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION".
2. CONCRETE IS SPECIFIED PER ALTERNATIVE 1 - PERFORMANCE SPECIFICATION AS OUTLINED IN CAN/CSA A23.1. THE CONTRACTOR AND THE CONCRETE SUPPLIER TO MEET ALL CERTIFICATION, DOCUMENTATION, AND QUALITY CONTROL REQUIREMENTS.
3. THE CONCRETE SUPPLIER TO BE CERTIFIED BY "CONCRETE ONTARIO" (FORMERLY RMCAO CERTIFICATION).
4. THE CONTRACTOR AND THE CONCRETE SUPPLIER TO ENSURE THAT THE PLASTIC AND HARDENED MIX PROPERTIES MEET SITE REQUIREMENTS FOR PLACING, FINISHING, AND THE SPECIFIED PERFORMANCE REQUIREMENTS.
5. CEMENT TO BE PORTLAND CEMENT TYPE GU UNLESS NOTED OTHERWISE OR REQUIRED BY EXPOSURE CLASS.
6. CONCRETE TO BE NORMAL DENSITY, UNLESS NOTED OTHERWISE.
7. NOMINAL SIZE OF COURSE AGGREGATE TO BE 20 (3/4"), UNLESS NOTED OTHERWISE.
8. PLACE CONCRETE AS CLOSE AS POSSIBLE TO FINAL LOCATION TO AVOID SEGREGATION. VIBRATE ALL CONCRETE.
9. PROTECT CONCRETE FROM FREEZING. DO NOT PLACE CONCRETE AGAINST FROZEN GROUND. USE COLD WEATHER CONCRETING METHODS IN ACCORDANCE WITH CAN/CSA-A23.1.
10. PROTECT CONCRETE FROM EXCESSIVE HEAT AND DRYING. USE HOT WEATHER CONCRETING METHODS IN ACCORDANC

CONCRETE COVER TO REINFORCING STEEL

GN-009
December 22, 2021

CONCRETE COVER TABLE WITH 2 HOUR FIRE RATING, NOT PRESTRESSED				
STRUCTURAL ELEMENT	BAR SIZE (M)	BUILDING INTERIOR (WITHIN VAPOUR BARRIER) BELOW GRADE (EXCLUDE PARKING STRUCTURE) COVER (mm)		
		TOP	BOTTOM	SIDE
FOOTING (WITHOUT FORMWORK)	≤ 35	50	75	75
FOOTING (WITH FORMWORK)	≤ 35	50	75	50
GRADE BEAM AND PIER (WITH FORMWORK / LINER)	≤ 35	50	50	50
COLUMN (TO TIE)	≤ 35			40
SHEAR WALL	≤ 25			25
	≤ 30			30
	≤ 35			35
BEAM (TO STIRRUP)	≤ 35	30	30	30
PERIMETER FOUNDATION WALL (EXTERIOR FACE REINFORCEMENT - CAST AGAINST EARTH)	≤ 35			75
PERIMETER FOUNDATION WALL (EXTERIOR FACE NOT CAST AGAINST EARTH + INTERIOR FACE)	≤ 25			40
	≤ 30			45
	≤ 35			55
BELOW GRADE SLAB	≤ 25	25	25	
	≤ 30	30	30	
	≤ 35	35	35	

STRUCTURAL ELEMENT	BAR SIZE (M)	INTERIOR (WITHIN VAPOUR BARRIER) ABOVE GRADE (EXCLUDE PARKING STRUCTURE) COVER (mm)		
		TOP	BOTTOM	SIDE
SHEAR WALL EXPOSED TO FIRE ON BOTH SIDES (SEE PLAN OR ELEVATION)	≤ 35			50
GROUND FLOOR SLAB (NON VEHICLE ACCESSIBLE)	≤ 25	25	25	
	≤ 30	30	30	
	≤ 35	35	35	
FLOOR SLAB (EXCLUDING EXPOSED BALCONY SLAB)	≤ 25	25	25	
	≤ 30	30	30	
	≤ 35	35	35	
STRUCTURAL ELEMENT	BAR SIZE (M)	EXTERIOR (OUTSIDE VAPOUR BARRIER) ABOVE GRADE COVER (mm)		
		TOP	BOTTOM	SIDE
COLUMN (TO TIE)	≤ 35			60
COLUMN (TO TIE)	≤ 30			40
	≤ 35			45
SHEAR WALL	≤ 30			60
	≤ 35			70
SHEAR WALL	≤ 25			40
	≤ 30			45
	≤ 35			55
RETAINING WALL	≤ 30			60
	≤ 35			70
RETAINING WALL	≤ 25			40
	≤ 30			45
	≤ 35			55
SLAB	≤ 30	60	60	
	≤ 35	70	70	
SLAB / EXPOSED BALCONY SLAB	≤ 25	40	40	
	≤ 30	45	45	
	≤ 35	55	55	
BEAM (TO STIRRUP)	≤ 35	60	60	60
BEAM (TO STIRRUP)	≤ 25	30	30	30
	≤ 30	35	35	35
	≤ 35	45	45	45

* REFER TO PLAN FOR LOCATION WHERE DIFFERENT CONCRETE COVER IS REQUIRED.

CONCRETE REINFORCEMENT

GN-009A
December 22, 2021

1. REINFORCEMENT IS TO CONFORM TO THE FOLLOWING STANDARDS:
   A. DEFORMED BARS: CAN/CSA G30.18, GRADE 400R.
   B. WELDED DEFORMED BARS: CAN/CSA G30.18, GRADE 400W.
   C. WELDED WIRE FABRIC: ASTM A185/A185M.
   D. EPOXY COATING: ASTM D3963
2. PROVIDE MIN. 24 HOURS NOTICE PRIOR TO CLOSING FORMS OR PLACING CONCRETE TO ALLOW FOR REINFORCING STEEL REVIEW.
3. BARS MARKED CONTINUOUS TO BE TERMINATED IN STANDARD HOOKS AT ENDS AND SPLICED USING CLASS B LAPS. DO NOT SHORTEN HOOKS TO ACCOMMODATE MEMBER DIMENSION.
4. ALL REBAR HOOKS TO BE STANDARD LENGTH 90° OR 180° HOOKS. REBAR LENGTHS LISTED ON DRAWINGS DO NOT INCLUDE THE HOOK LENGTH.
5. WHERE TWO BARS OF DIFFERENT SIZE ARE LAPPED IN TENSION, SPLICE LENGTH TO BE EQUAL TO THE SMALLER BAR'S TENSION LAP SPLICE, OR TO THE LARGER BAR'S TENSION DEVELOPMENT LENGTH, WHICHEVER IS LONGER.
6. WHERE TWO BARS OF DIFFERENT SIZE ARE LAPPED IN COMPRESSION, SPLICE LENGTH TO BE EQUAL TO THE SMALLER BAR'S COMPRESSION LAP SPLICE, OR TO THE LARGER BAR'S COMPRESSION DEVELOPMENT LENGTH, WHICHEVER IS LONGER.
7. PROVIDE ADDITIONAL SUPPORT BARS AS REQUIRED TO ADEQUATELY SUPPORT AND SECURE ALL REINFORCEMENT AND PREVENT MOVEMENT WHEN PLACING CONCRETE.
8. PROVIDE SUFFICIENT CHAIRS TO REINFORCING TO MAINTAIN SPECIFIED CONCRETE COVER.
9. ALL REINFORCING TO BE CLEAN, FREE OF LOOSE SCALE, OIL, DIRT, RUST, AND ANY OTHER FOREIGN COATING THAT AFFECT BONDING CAPACITY.
10. MINIMUM CLEAR CONCRETE COVER TO REINFORCEMENT CLOSEST TO THE SURFACE TO BE AS FOLLOWS UNLESS NOTED OTHERWISE ON THE DRAWINGS.
11. WALLS
    A. UNLESS OTHERWISE NOTED ON DRAWINGS, SCHEDULES OR NOTES, MINIMUM REINFORCEMENT FOR CONCRETE WALLS TO BE AS FOLLOWS:
       a. 150 MAXIMUM WALL: 10 @ 300 (1*2") H + 10 @ 400 (16") V IN CENTRE
       b. 200 MAXIMUM WALL: 10 @ 250 (10") H + 10 @ 300 (12") V IN CENTRE
       c. 250 MAXIMUM WALL: 10 @ 400 (16") HEF + 10 @ 500 (20") VEF
       d. 300 MAXIMUM WALL: 10 @ 330 (13") HEF + 10 @ 440 (17") VEF
       e. VERTICAL REINFORCING TO BE OUTSIDE LAYER.
    B. REFER TO TYPICAL DETAILS FOR REINFORCING REQUIRED AT WALL CORNERS, INTERSECTIONS AND OPENINGS, AND FOR MINIMUM DOWELING REQUIREMENTS BETWEEN WALLS AND SLABS.
12. SLABS
    A. UNLESS OTHERWISE SPECIFIED ON PLANS, PROVIDE BOTTOM TEMPERATURE REINFORCEMENT IN BOTH DIRECTIONS FOR FRAMED SLABS IN ACCORDANCE WITH TYPICAL DETAILS.
    B. DO NOT ELIMINATE, CUT, OR DISPLACE REINFORCING TO ACCOMMODATE MECHANICAL AND ELECTRICAL SLEEVES, OPENINGS OR HARDWARE. SPREAD REINFORCING AROUND SLEEVES. PROVIDE ADDITIONAL REINFORCING AS SHOWN IN TYPICAL DETAILS AND DRAWINGS FOROPENINGS LARGER THAN 450 x 450 (18"X18"). IF OPENINGS, SLEEVES, OR HARDWARE CANNOT BE LOCATED AS SPECIFIED, OBTAIN APPROVAL OF MODIFICATIONS FROM CONSULTANT PRIOR TO PLACING CONCRETE.

CONDUITS AND EMBEDS

GN-010
December 22, 2021

1. GENERAL:
   A. ALL CONCRETE EMBEDS (PIPES, CONDUITS, SLEEVES, ETC. INCLUDING ANY EMBEDDED FITTINGS OR COUPLERS) AND THEIR INSTALLATION TO CONFORM TO CSA A23.1/A23.2 AND THE FOLLOWING GUIDELINES. THE MORE STRINGENT GUIDELINES GOVERN.
   B. ALL CONCRETE EMBEDS SHALL NOT IMPAIR THE STRENGTH OF STRUCTURAL ELEMENTS AND SHALL BE MOVED OR REMOVED IF DIRECTED BY THE CONSULTANT. NO CONCRETE EMBEDS ARE PERMITTED WITHIN "PROTECTED ZONES" INDICATED ON THE STRUCTURAL DRAWINGS. IF REQUIRED, REMEDIAL REINFORCING AT ZONES OF CONGESTED EMBEDS SHALL BE PROVIDED AT NO EXTRA COST TO THE CONTRACT.
   C. DO NOT CUT, ELIMINATE OR DISPLACE REINFORCING TO INSTALL EMBEDS OR HARDWARE UNLESS APPROVED IN WRITING BY THE CONSULTANT. OBTAIN APPROVAL BEFORE PLACING CONCRETE.
   D. WHERE APPROVED BY THE CONSULTANT, SET SLEEVES, TIES, PIPE HANGERS AND OTHER INSERTS AND OPENINGS AS INDICATED OR SPECIFIED ELSEWHERE. SLEEVES AND OPENINGS GREATER THAN 100 X 100 mm (4" X 4") NOT INDICATED MUST BE REVIEWED BY THE CONSULTANT.
   E. CONCRETE EMBEDS SHALL NOT HAVE AN OUTSIDE DIAMETER LARGER THAN ¼ OF THE THICKNESS OF CONCRETE ELEMENT WHICH THEY ARE EMBEDDED IN AND SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS ON CENTER. THE LARGER EMBED DIAMETER WILL GOVERN FOR SPACING.
   F. EMBEDS WITHIN A CONCRETE ELEMENT WILL BE PLACED WITHIN THE MIDDLE THIRD OF THE ELEMENT THICKNESS. PROVIDE CHAIRS/SUPPORTS FOR THE EMBEDS, DO NOT TIE TO REINFORCING STEEL. PROVIDE MINIMUM 50 CLEAR SPACE BETWEEN ALL EMBEDS AND REINFORCING BARS.
   G. SUBMIT A COMBINED LAYOUT DRAWING SET SHOWING SLEEVES AND OPENINGS REQUIRED BY ALL TRADES FOR THE CONSULTANT'S REVIEW PRIOR TO CONSTRUCTION. SLEEVE COORDINATION TO BE DONE BY OTHERS. CONFIRM LOCATIONS AND SIZES OF SLEEVES AND OPENINGS SHOWN ON DRAWINGS.
   H. METAL EMBEDS OTHER THAN CARBON STEEL MUST BE ELECTRICALLY ISOLATED WITH A DIELECTRIC COATING.
2. CONCRETE VERTICAL ELEMENTS:
   A. DO NOT PLACE EMBEDS INCLUDING CONDUITS, SLEEVES AND RECESSED BOXES PERMITTED IN ANY COLUMN WITHOUT PRIOR APPROVAL FROM THE CONSULTANT.
   B. CONCRETE WALL EMBEDS TO BE SPACED MINIMUM AT 4 DIAMETERS ON CENTER. VERTICAL EMBEDS TO BE LOCATED WITHIN MIDDLE THIRD OF WALL AND TO HAVE MINIMUM 75 CLEAR COVER. NO HORIZONTAL EMBED LENGTHS PERMITTED ALONG THE LONG DIRECTION WITHIN ANY WALL. ONLY VERTICAL EMBEDS AND THROUGH WALLS SLEEVES ARE PERMITTED.
   C. NO EMBEDS PERMITTED WITHIN WALL CONCENTRATED ZONE STEEL AREAS OR WITHIN 1000 OF WALL END.
3. CONCRETE HORIZONTAL ELEMENTS:
   A. MAXIMUM 2 LAYERS OF CONDUITS PERMITTED WITHIN A CONCRETE SLAB, INCLUDING CROSSOVERS.
   B. PENETRATIONS TO BE SET A MINIMUM OF 3X SLAB THICKNESS AWAY FROM THE FACE OF ANY COLUMN. DO NOT LOCATE PENETRATIONS WITHIN COLUMN CAPITALS OR DROP PANELS. PENETRATIONS TO BE SET A MINIMUM OF 3X SLAB THICKNESS AWAY FROM THE EDGES OF DROP PANELS OR STUD RAILS. SLEEVES TO NOT BE LOCATED WITHIN COLUMN ZONES WITHOUT WRITTEN APPROVAL FROM THE CONSULTANT.
   C. MAXIMUM OUTSIDE DIAMETER OF ANY SLEEVE, CONDUIT OR EMBED IN OR THROUGH THE SLAB NOT TO EXCEED ONE QUARTER OF THE SLAB THICKNESS AND MINIMUM SPACING TO BE 3 DIAMETERS ON CENTRE ALL AROUND.
   D. NO SLEEVES THROUGH ANY BEAMS OR SLAB BANDS UNLESS APPROVED IN WRITING BY THE CONSULTANT.
   E. SLEEVES AND EMBEDS ARE NOT PERMITTED IN BONDED TOPPINGS UNLESS APPROVED BY THE CONSULTANT.
   F. FOR SLABS-ON-GRADE OR UNBONDED CONCRETE TOPPINGS, LOCATE ALL CONDUITS, PIPES, OR HEATING CABLES EMBEDDED IN CONCRETE CLEAR OF THE TOP ONE THIRD OF THE THICKNESS TO AVOID DAMAGE DURING SAWCUTTING.
   G. PT SLABS TO FOLLOW SAME RULES AS NORMAL SLABS. IN ADDITION, NO EMBEDS SHALL BE PLACED NEAR POST-TENSIONING ANCHORAGES UNLESS APPROVED BY THE PT CONTRACTOR.

POST-INSTALLED ANCHORS AND DOWELS

GN-011
December 22, 2021

1. UNLESS OTHERWISE NOTED, ANCHORAGE TO CONCRETE TO BE:
   A. WHERE DRILLED CONCRETE ANCHORS (DCA) ARE NOTED ON DRAWINGS, PROVIDE HILTI KWIK BOLT – TZ2 EXPANSION ANCHORS.
   B. WHERE ADHESIVE ANCHORS ARE NOTED ON DRAWINGS, PROVIDE HILTI HIT-HY 200 ADHESIVE ANCHORING SYSTEM WITH HILTI HIT-Z ANCHOR RODS USING HILTI SAFESET HOLLOW DRILL BIT TECHNOLOGY.
   C. WHERE DRILLED REINFORCING STEEL DOWELS ARE NOTED ON DRAWINGS, PROVIDE HILTI HIT-HY 200 ADHESIVE ANCHORING SYSTEM INSTALLED USING HILTI SAFESET HOLLOW DRILL BIT TECHNOLOGY.
2. UNLESS OTHERWISE NOTED ON DRAWINGS, ANCHORAGE TO MASONRY TO BE:
   A. FOR SOLID OR GROUTED MASONRY, WHERE DRILLED MASONRY ANCHORS (DMA) ARE NOTED ON DRAWINGS, PROVIDE HILTI KB-3 EXPANSION ANCHORS.
   B. FOR SOLID OR GROUTED MASONRY, WHERE ADHESIVE ANCHORS ARE NOTED ON DRAWINGS, PROVIDE HILTI HIT-HY 270 ADHESIVE ANCHORING SYSTEM WITH HAS-E THREADED RODS.
   C. FOR HOLLOW MASONRY, PROVIDE HILTI HIT-HY 270 ADHESIVE ANCHORING SYSTEM WITH HIT-SC MESH SLEEVE AND HAS-E THREADED RODS.
3. WHERE GALVANIZED ANCHORS ARE SPECIFIED, PROVIDE HOT DIPPED GALVANIZED (NOT ZINC PLATED) ANCHORS. *Check if galvanized is available for the anchors specified. Hilti KB-TZ are currently not available galvanized.*
4. CONCRETE TO BE MINIMUM 28 DAYS OLD AT THE TIME OF ANCHOR INSTALLATION.
5. USE DRILLING AND INSTALLATION TOOLS AND PROCEDURES PER MANUFACTURERS' RECOMMENDATIONS. HOLE DIAMETERS NOT TO EXCEED THOSE REQUIRED BY MANUFACTURER.
6. ARRANGE FOR A MANUFACTURER'S TECHNICAL REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL THE PRODUCTS SPECIFIED, AND CONDITIONS ENCOUNTERED (E.G. HORIZONTAL, INCLINED, OVERHEAD). SUBMIT RECORD OF TRAINING LISTING ALL TRAINED PERSONNEL. IN ADDITION, PERSONNEL INSTALLING ADHESIVE ANCHORS IN A HORIZONTALLY OR UPWARDLY INCLINED DIRECTION THAT ARE REQUIRED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE CERTIFIED IN ACCORDANCE WITH ACI/CRSI ADHESIVE INSTALLER CERTIFICATION PROGRAM AS PER CSA A23.3 ANNEX D.
7. ARRANGE FOR A MANUFACTURER'S TECHNICAL REPRESENTATIVE TO BE PRESENT DURING INSTALLATION OF FIRST FEW ANCHORS OF EACH SIZE AND TYPE. SUBMIT RECORDS BY MANUFACTURER TO CONSULTANT WITHIN ONE WEEK OF EACH VISIT. REPORTS TO INDICATE ANCHOR TYPES AND SIZES INSTALLED, LOCATIONS AND INSTALLERS' NAMES.
8. DO NOT CUT REINFORCEMENT TO ACCOMMODATE DRILLED ANCHORS AND DOWELS. SCAN THE STRUCTURE TO LOCATE REINFORCEMENT PRIOR TO FABRICATING STRUCTURAL STEEL FASTENED BY DRILLED ANCHORS.
9. WHEN OBSTRUCTIONS PREVENT DRILLING HOLES IN SPECIFIED LOCATIONS TO THE REQUIRED DEPTH, RELOCATE AT NO EXTRA COST TO THE CONTRACT. OBTAIN CONSULTANT'S APPROVAL OF NEW LOCATIONS BEFORE DRILLING HOLES. FILL ALL ABANDONED HOLES WITH MIN. 30MPa GROUT. DO NOT TIGHTEN ANCHORS UNTIL GROUT IN ADJACENT ABANDONED HOLES REACHES 75% f'c.
10. DO NOT BEND POST INSTALLED DOWELS AND RODS AFTER INSTALLATION.
11. STANDARD EMBEDMENT LENGTHS FOR POST-INSTALLED ANCHORS:

ANCHOR TYPE	HILTI KB-TZ2		ADHESIVE HY-200V3		ADHESIVE HY-270 +HIT-S SCREEN	
BASE MATERIAL	CONCRETE	GROUTED MASONRY	CONCRETE	GROUTED MASONRY	GROUTED MASONRY	HOLLOW MASONRY
12 (1/2")	64 (2 1/2")	64 (2 1/2")	114 (4 1/2")	114 (4 1/2")	86 (3 3/8")	51 (2")
16 (5/8")	83 (3 1/4")	83 (3 1/4")	143 (5 5/8")	143 (5 5/8")	114 (4 1/2")	51 (2")
19 (3/4")	121 (4 3/4")	102 (4")	171 (6 3/4")	171 (6 3/4")	143 (5 5/8")	51 (2")
25 (1")	N/A	N/A	229 (9")	N/A	171 (6 3/4")	51 (2")

NOTES:

1. ALL ANCHORS TO HAVE MINIMUM EFFECTIVE EMBEDMENT LENGTHS NOTED IN THE TABLE ABOVE, UNLESS OTHERWISE SHOWN ON DRAWINGS.
2. FOLLOW INSTALLATION PROCEDURE AND USE TOOLS SPECIFIED IN THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII).
3. GROUTED MASONRY :
   A. EMBEDMENTS SHOWN ARE FOR ANCHORS INTO THE SIDE OF A CONCRETE MASONRY UNIT.
   B. HIT-HY 200 ANCHORS MAY BE INSTALLED IN THE SIDE OF MASONRY AT ANY LOCATIONS LIMITED TO ONE ANCHOR PER CELL.
   C. ONLY INSTALL 5/8" AND 3/4" USING HIT HY-200V3 ANCHORS INTO GROUTED WALLS GREATER OR EQUAL TO 190 THICK
   D. HILTI HIT KB-TZ2 ANCHORS MAY ONLY BE INSTALLED ONLY THROUGH THE FACE SHELL OF THE BLOCK, A MINIMUM OF 35 (1-3/8") FROM ANY VERTICAL MORTAR JOINT.
4. RELOCATE ANCHORS AS REQUIRED TO CLEAR REINFORCING BARS. FILL ABANDONED ANCHOR HOLES WITH MIN. 30 MPa GROUT. DO NOT TIGHTEN ADJACENT ANCHORS UNTIL GROUT IN ABANDONED HOLES REACHES 75% fc'. DO NOT CUT ANY REINFORCING STEEL.

STRUCTURAL STEEL

GN-012
December 22, 2021

1. CONFORM TO CSA S16: "LIMIT STATES DESIGN OF STEEL STRUCTURES"
2. FABRICATOR SHALL BE CERTIFIED BY CANADIAN WELDING BUREAU UNDER REQUIREMENTS OF CSA W47.1, DIVISION 1 OR 2
3. MATERIALS: TO CSA G40.21 UNLESS OTHERWISE NOTED, WITH THE FOLLOWING GRADES:
   A. WIDE FLANGE SECTIONS - 350W
   B. CHANNELS, ANGLES - 350W
   C. PLATES, BARS - 300W
   D. HOLLOW STRUCTURAL SECTIONS (HSS) - 350W (CLASS 'C') OR ASTM A500, 345MPa (GRADE 'C')
   E. GALVANIZED HSS 350W (CLASS 'H') OR STRESS-RELIEVED PRIOR TO GALVANIZNG.
   F. STEEL PIPE - ASTM A53, 240W
   G. STRUCTURAL STEEL BOLTS - ASTM A325M, UNLESS NOTED
   H. ANCHOR RODS - 300W
   I. SPECIAL ANCHOR RODS - ASTM F1554 GRADE 105 (WHERE SHOWN ON DRAWINGS)
4. DETAILS ON STRUCTURAL DRAWINGS SHOW DESIGN INTENT. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN, DETAILING, FABRICATION, AND ERECTION REQUIREMENTS.
5. DO NOT CUT HOLES OR OTHERWISE MODIFY STRUCTURAL MEMBERS ON SITE.
6. CLEAN SURFACES DOWN TO BARE METAL AND APPLY TWO COATS OF ZINC-RICH TOUCH-UP PAINT TO ANY GALVANIZED SURFACE THAT HAS BEEN DAMAGED OR FIELD WELDED.
7. PROVIDE ALL ERECTION BRACING REQUIRED TO KEEP THE STRUCTURE STABLE AND IN ALIGNMENT DURING CONSTRUCTION.
8. WHERE STRUCTURAL DRAWINGS INDICATE PROTECTED ZONES, DO NOT FASTEN OR OTHERWISE CONNECT ANY STRUCTURAL OR NON-STRUCTURAL ELEMENT TO THOSE ZONES, WITH EXCEPTION OF STEEL DECK CONNECTED WITH 19 (3/4") PUDDLE WELDS. FAILURE TO ADHERE WILL REQUIRE COMPLETE REPLACEMENT OF THE AFFECTED MEMBER.
9. WHERE MOMENT CONNECTIONS ARE SHOWN ON PLAN PROVIDE CONNECTION FOR FULL MEMBER CAPACITY (UNLESS NOTED OTHERWISE.
10. DISTRIBUTE HANGER LOADS FROM MECHANICAL AND HEAVY ELECTRICAL SERVICES SUSPENDED FROM STEELWORK UNIFORMLY ALONG MEMBERS. DO NOT CAUSE TWISTING OF STEEL MEMBERS OR EXCESSIVE BENDING OF MEMBER FLANGES. ALTERNATE HANGER POSITION ON EITHER SIDE OF MEMBERS.
11. WHERE SEISMIC RESTRAINTS ARE REQUIRED FOR NON-STRUCTURAL ELEMENTS, APPLY LOADS IN THE PLANE OF PRIMARY STRUCTURAL ELEMENTS, AND DO NOT EXCEED 1.5kN.
12. DO NOT APPLY LATERAL LOADS TO MEMBERS UNLESS APPROVED BY THE CONSULTANT.
13. AESS MEMBERS: ERECT USING SOFTENED SLINGS OR OTHER METHODS SUCH THAT THEY ARE NOT DAMAGED. PROVIDE PADDING AS REQUIRED TO PROTECT WHILE RIGGING AND ALIGNING. WELD TABS FOR TEMPORARY BRACINGS AND SAFETY CABLING ONLY AT POINTS CONCEALED FROM VIEW IN THE COMPLETE STRUCTURE OR WHERE APPROVED BY THE CONSULTANT.
14. AESS MEMBERS: REMOVE ALL FIELD CONNECTION AIDS ADDED TO ALLOW ALIGNMENT, FIT UP AND WELDING. REMOVE WELDS AT RUN-OUT TABS TO MATCH ADJACENT SURFACE, AND GRIND SMOOTH. PLUG WELD AND GRIND SMOOTH HOLES FOR ERECTION BOLTS.
15. GROUT UNDER BASE PLATES AND BEARING PLATES USING PROCEDURES IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. PROVIDE 100% CONTACT OVER GROUTED AREA.
16. GROUT COLUMN BASE PLATES AND BEAM BEARING PLATES AS SOON AS STEELWORK IS COMPLETED. DO NOT ADD LOAD ON STEELWORK UNTIL GROUTING IS COMPLETED AND GROUT STRENGTH HAS REACHED AT LEAST 20 MPA.

STEEL ROOF DECK & COMPOSITE FLOOR SLABS

GN-014
December 22, 2021

1. CONFORM TO S136 "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" FOR STEEL DECKING, AND TO CONCRETE AND CONCRETE REINFORCEMENT NOTES.
2. STEEL DECK MATERIAL: TO ASTM A653/653M OR ASTM A792/792M, GRADE 230.
3. DESIGN STEEL DECK AND CONNECTING FASTENERS FOR THE LOADS DESCRIBED ON THE STRUCTURAL DRAWINGS. LIMIT TOTAL DEFLECTIONS TO L/240 AND DEFLECTIONS UNDER LIVE OR SNOW LOADS TO L/360. REFER TO PLANS AND SPECIFICATIONS FOR LOADING AND PERFORMANCE REQUIREMENTS.
4. DECK THICKNESS AND DECK FASTENERS SHOWN ON THE DRAWINGS ARE MINIMUM REQUIREMENTS. ALTERNATIVE SOLUTIONS MAY BE PROPOSED BY DECK SUPPLIER'S ENGINEER.
5. MINIMUM DECK FASTENING REQUIREMENTS (IF DIAPHRAGM SHEAR AND / OR DECK FASTENERS ARE NOTED ON PLAN) ARE AS FOLLOWS:
   DIAPHRAGM SHEAR: 5 kN/m (MINIMUM)
   TRANSVERSE FASTENERS
   19 (3/4") DIAMETER ARC SPOT WELDS, OR HILTI DIRECT FASTENERS, PLACED IN EVERY SECOND FLUTE. IN ADDITION, PLACE TWO FASTENERS IN FLUTES WHERE SIDE LAPS ARE MADE, ONE EACH SIDE OF LAP.
   LONGITUDINAL FASTENERS
   TYPE TO MATCH TRANSVERSE FASTENERS, SPACED AT 450 (18") ON CENTRE
   SIDELAPS
   MECHANICALLY CLINCHED (BUTTON PUNCHED) OR FASTENED WITH #10 SCREWS, SPACED AT 230 (9") ON CENTRE
6. STEEL ROOF DECK IS DESIGNED TO SUPPORT UNIFORMLY DISTRIBUTED LOADS INDICATED ON DRAWINGS AND MAY NOT BE ABLE TO RESIST CONCENTRATED LOADS. DO NOT HANG ANY CONCENTRATED LOADS FROM STEEL ROOF DECK, AND ATTACH TO STRUCTURAL STEEL FRAMING INSTEAD.
7. COMPOSITE FLOOR SLABS:
   A. STEEL DECK SHOWN WITH CONCRETE TOPPING SHALL BE A COMPOSITE PROFILE HAVING EMBOSSMENTS THAT ENSURE 100% COMPOSITE ACTION BETWEEN THE CONCRETE AND STEEL DECK, UNLESS OTHERWISE NOTED.
   B. REFER TO PLANS FOR SLAB THICKNESS, WHICH IS MEASURED FROM THE TOP OF DECK FLUTES TO THE TOP SURFACE OF CONCRETE. MAINTAIN SPECIFIED THICKNESS WHILE PLACING CONCRETE.
   C. PROVIDE SHORING FOR DECK AND COMPOSITE STEEL BEAMS WHERE SHOWN ON DRAWINGS. DO NOT REMOVE UNTIL CONCRETE HAS ATTAINED 75% OF ITS SPECIFIED 28 DAY COMPRESSION STRENGTH.
   D. UNLESS OTHERWISE SHOWN ON DRAWINGS, REINFORCE SLAB ON DECK SUPPORTED BY NON-COMPOSITE BEAMS OR JOISTS AS PER TYPICAL DETAILS.
   E. UNLESS OTHERWISE SHOWN ON DRAWINGS, REFER TO TYPICAL DETAIL SD-300 FOR REINFORCING OF SLAB ON DECK SUPPORTED BY COMPOSITE BEAMS.
   F. SPLICE ALL REBAR WITH CLASS 'B' LAPS AND PROVIDE STANDARD HOOKS ALONG SLAB EDGES.
   G. LAP WELDED WIRE FABRIC A MINIMUM OF 200 (8").
   H. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN WIRE FABRIC OR REBAR 25 (1") BELOW TOP OF SLAB.
   I. PROVIDE CONTINUOUS POUR STOP FOR THE FULL DECK + CONCRETE DEPTH, UNLESS STEEL ANGLES OR BENT PLATES ARE SHOWN ON DRAWINGS.
   J. CONDUITS AND PIPES ARE NOT PERMITTED IN THE CONCRETE TOPPING UNLESS APPROVED BY THE STRUCTURAL ENGINEER.
   K. PRIOR TO CONCRETE PLACEMENT, STEEL DECK TO BE FREE OF SOIL, DEBRIS, STANDING WATER, LOOSE MIL SCALE, AND OTHER FOREIGN MATTER.

LOAD BEARING MASONRY

GN-017
December 22, 2021

1. CONFORM TO CSA A371 "MASONRY CONSTRUCTION FOR BUILDINGS" AND CSA S304.1 "DESIGN OF MASONRY STRUCTURE
2. MATERIALS:
   A. HOLLOW BLOCK: CSA A165.1 - H/15/A/M
   B. SOLID BLOCK: CSA A165.1 - S/F/15/A/M
   C. MORTAR: CSA A179 - TYPE S
   D. MASONRY GROUT: CSA A179M - COARSE GROUT, 1:3:2 - CEMENT: SAND: PEA-STONE BY VOLUME, WITH SLUMP, OR APPROVED PRE-BAGGED MATERIAL
3. MASONRY CONTRACTOR IS TO BE A MEMBER OF THE CANADIAN MASONRY CONTRACTORS ASSOCIATION.
4. NON LOAD-BEARING MASONRY WALLS ARE NOT NECESSARILY SHOWN ON STRUCTURAL DRAWINGS, SEE ARCHITECTURAL DRAWINGS.
5. LAY UNITS IN RUNNING BOND. ALL FACE SHELLS TO BE FULLY BEDDED, UNLESS NOTED OTHERWISE.
6. DO NOT USE MORTAR WHERE GROUT IS SPECIFIED.
7. PROVIDE LADDER TYPE HORIZONTAL JOINT REINFORCING PER ARCHITECTURAL SPECIFICATIONS, UNLESS OTHERWISE NOTED. USE CORNER TYPE LADDER REINFORCING AT ALL MASONRY WALL CORNERS AND INTERSECTIONS.
8. PROVIDE LATERAL SUPPORT AT TOPS OF ALL WALLS, TO REFER TO ARCHITECTURAL PROJECT DETAILS AND ARCHITECTURAL SPECIFICATIONS.
9. UNLESS OTHERWISE NOTED, PROVIDE MINIMUM 25 (1") DEFLECTION GAP AT TOP OF ALL NON LOAD BEARING MASONRY WALLS.
10. WHERE MASONRY WALLS ABUT COLUMNS OR CONCRETE WALLS, PROVIDE 25 WIDE MOVEMENT JOINTS BETWEEN THEM AND FILL WITH COMPRESSIBLE MATERIAL.
11. PROVIDE VERTICAL MOVEMENT JOINTS (MJ) IN LOAD BEARING WALLS AT LOCATIONS INDICATED ON PLANS, FILL WITH COMPRESSIBLE MATERIAL AS PER ARCHITECTURAL SPECIFICATIONS. EXTEND MOVEMENT JOINTS HEIGHT OF WALL, TERMINATING AT THE UNDERSIDE OF CONTINUOUS BOND BEAMS AT THE FLOOR OR ROOF (WHERE APPLICABLE).
12. PROVIDE VERTICAL MOVEMENT JOINTS (MJ) IN NON-LOAD BEARING WALLS AS INDICATED ON ARCHITECTURAL DRAWINGS AND SPECIFICATIONS.
13. PROVIDE STANDARD LINTELS OVER ALL OPENINGS IN NON-LOAD BEARING MASONRY WALLS AS SHOWN ON TYPICAL DETAILS. THE OPENINGS ARE NOT NECESSARILY SHOWN ON STRUCTURAL DRAWINGS, REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS.
14. PROVIDE MIN. 190 (7-1/2") DEEP BOND BEAMS AT TOP OF WALLS. REINFORCE WITH MIN. 1-15 TOP AND BOTTOM CONTINUOUS AND GROUT SOLID. CARRY ALL VERTICAL REINFORCING BARS CONTINUOUSLY THROUGH BOND BEAM.
15. REINFORCED MASONRY:
    A. SEE PLANS AND DETAILS FOR STRUCTURAL MASONRY REINFORCING.
    B. FOR NON-LOAD BEARING MASONRY, SEE TYPICAL DETAILS FOR MINIMUM SEISMIC MASONRY REINFORCING.
    C. MASONRY CELLS TO BE REINFORCED ARE TO BE CLEAR OF CONDUITS AND OVER-HANGING MORTAR OR DEBRIS.
    D. SPLICES FOR MASONRY WALL REINFORCING TO BE:
       WIRE REINFORCING 200 (8")
       10M BARS 450 (18")
       15M BARS 650 (26")
       20M BARS 900 (36")
       25M BARS 1300 (51")
    E. LOCATE VERTICAL BARS ACCURATELY WITHIN CELLS AS INDICATED ON DRAWINGS. HOLD IN POSITION AND BOTTOM. USE REBAR POSITIONERS AS REQUIRED.
    F. PROVIDE VERTICAL DOWELS AT BASE OF WALLS TO MATCH VERTICAL REINFORCING. EMBED INTO SLAB WITH STANDARD HOOKS OR PROVIDE CLASS B LAP INTO FOUNDATION WALLS BELOW (AS APPLICABLE).
    G. REINFORCE ALL SIDES OF WALL OPENINGS EXCEEDING 1000 (3'-6") IN WIDTH OR HEIGHT WITH ADDITIONAL 1-15 HORIZONTAL AND VERTICAL AS APPROPRIATE. EXTEND 600 (2'-0") BEYOND OPENING. PLACE HORIZONTAL BARS IN BLOCKS WITH PREFORMED CELLS WHICH ALLOW PASSING OF VERTICAL BARS. WHERE STEEL LINTELS ARE USED, OFFSET VERTICALS TO CLEAR LINTEL BEARING.
    H. ADD 1-15 VERTICAL AT WALL ENDS AND ON EACH SIDE OF MOVEMENT JOINTS.
16. GROUTING MASONRY:
    A. GROUT CELLS CONTAINING VERTICAL OR HORIZONTAL REINFORCEMENT ONLY. IF MASONRY WALLS ARE AS "GROUTED SOLID" ON PLAN, THEN GROUT ALL CELLS FULLY.
    B. USE LOW LIFT GROUTING PROCEDURE UNLESS OTHERWISE APPROVED IN WRITING BY THE STRUCTURAL CONSULTANT. PLACE GROUT IN LIFTS NOT EXCEEDING 1500 (5'-0") HEIGHT. TERMINATE EACH LIFT 40 (1 ½") BELOW TOP OF MASONRY UNIT.
    C. UNLESS OTHERWISE NOTED, PREFILL VOIDS IN MASONRY UNITS BELOW BEAM AND JOIST SUPPORTS WITH GROUT FOR A MINIMUM VERTICAL DEPTH OF 600 (2'-0") AND A LENGTH EXTEN

Key to Detail Location

NO. — Detail Number
NO. — Drawing Number

If this sheet is not 46 7/8" x 33 1/8" (1189 x 841 mm) It is a reduced print - Read dwg. accordingly.

Contractors must check and verify all dimensions on the job and report any discrepancies to the Architect before proceeding with the work.

All prints and specifications are the property of the Architect and must be returned at the completion of the work.

Drawings should not be scaled.

No	Date	Revision/Issued
1	2025-12-15	ISSUED FOR 30% CONSTRUCTION DOCUMENTS
2	2026-03-13	ISSUED FOR BUILDING PERMIT
3	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

DRAWING ABBREVIATIONS

AR	ANCHOR ROD
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
Af	FACTORED AXIAL LOAD IN kN
	(+ INDICATES TENSION,
	- INDICATES COMPRESSION)
ALT	ALTERNATE
ARCH	ARCHITECTURAL
B, BOT	BOTTOM
BCP	BORED CONCRETE PILE
BEW	BOTTOM EACH WAY
BLL	BOTTOM LOWER LAYER
BM	BEAM
BOF	BOTTOM OF FOOTING ELEVATION
BOP	BOTTOM OF PILE ELEVATION
BP	BEARING/BASE PLATE
BUL	BOTTOM UPPER LAYER
BUP	BOTTOM OF UNDERPINNING
CA	COLUMN ABOVE ONLY (NO COLUMN BELOW)
CAM	CAMBER
CANT	CANTILEVER
CB	COLUMN BELOW
C/C	CENTRE TO CENTRE
CEL	CUT OFF ELEVATION FOR PILES
CJ	CONTROL JOINT (CONCRETE)
CLR	CLEAR
℄	CENTRELINE
CNT	STEEL DECK CORE NOMINAL THICKNESS
CONSTR JT	CONSTRUCTION JOINT
COL	COLUMN
CONC	CONCRETE
CONT	CONTINUOUS
CP	CONNECTION PLATE
CWS	SEE GENERAL NOTES (WHERE INDICATED)
CLS	
CSS	
C	STRUCTURAL STEEL CHANNEL
DCA	DRILLED CONCRETE ANCHOR
DET	DETAIL
DFL	DOUGLAS FIR-LARCH
DIA	DIAMETER
DIM	DIMENSION
DL	DEAD LOAD IN kN/m ²
DMA	DRILLED MASONRY ANCHOR
DO	DITTO
DP	DEEP
DWG	DRAWING
DWL	DOWEL
EA	EACH
ECR	EPOXY COATED REINFORCEMENT
EE	EACH END
EF	EACH FACE
EJ,	EXP JT EXPANSION JOINT
EL,	ELEV ELEVATION
EMBED	EMBEDMENT
EQ	EQUAL
EX. EXIST	EXISTING
FD	FLOOR DRAIN
FF	FAR FACE
FL	FLOOR
FMC	FULL MOMENT CONNECTION
FTG	FOOTING
f'c	COMPRESSIVE STRENGTH OF CONC IN MPa
fy	YIELD STRENGTH IN MPa
GA	GAUGE
GALV	GALVANIZED STEEL
GB	GRADE BEAM
GL	GRIDLINE
h	TOTAL THICKNESS
HOR	HORIZONTAL
HEF	HORIZONTAL EACH FACE
HH	HOOK-HOOK (HOOK EACH END)
HIC	HORIZONTAL IN CENTRE
H	HOOK
HP	HIGH POINT
HSS	HOLLOW STRUCTURAL SECTION
IBA	INTEGRITY BARS ADDED
IBI	INTEGRITY BARS INTERIOR
IBE	INTEGRITY BARS EXTERIOR
ld	TENSION DEVELOPMENT LENGTH OF REBAR
ldc	COMPRESSION DEVELOPMENT LENGTH OF REBAR
L	SINGLE ANGLE
⅃L ,2Ls	DOUBLE ANGLES
LE	LEFT END
LG	LONG/LENGTH 2
LL	LIVE LOAD IN kN/m / LOWER LEVEL
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LSV	LONG SIDE VERTICAL
LSH	LONG SIDE HORIZONTAL
LP	LOW POINT
L-1, L-2, etc.	SPECIAL LINTEL AS SHOWN IN SCHEDULE
MAX	MAXIMUM
Mf	FACTORED MOMENT IN kN.m
MJ	MOVEMENT JOINT (MASONRY)
MIN	MINIMUM
NF	NEAR FACE
NIC	NOT IN CONTRACT
NTS	NOT TO SCALE
O/C	ON CENTRE
O/O	OUT TO OUT
P	POINT LOAD IN kN
Pf	FACTORED POINT LOAD IN kN
PL	PLATE
RD	ROOF DRAIN
REINF	REINFORCEMENT
RE	RIGHT END
Rf	FACTORED VERTICAL REACTION IN kN
Rhf	FACTORED HORIZONTAL REACTION IN kN
SDL	SUPERIMPOSED DL (EXCLUDING SELF-WEIGHT) IN kN/m
SECT	SECTION
SIM	SIMILAR
SLS	SERVICEABILITY LIMIT STATE
SL1, SL2, etc.	SHELF ANGLE 1, 2, ETC
SOG	SLAB ON GRADE
SPEC	SPECIFICATION
SPF	SPRUCE PINE FIR
STIR	STIRRUP
STIFF	STIFFENER
t	THICKNESS
T	TOP
T/O	TOP OF
TEW	TOP EACH WAY
T&B EW	TOP AND BOTTOM EACH WAY
TJ	TIE JOIST
TLL	TOP LOWER LAYER
Tmf	FACTORED TORSION IN kN.m
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOP	TOP OF PILE
TOS	TOP OF STEEL
TPC	TOP OF PILE CAP
TUL	TOP UPPER LAYER
TYP	TYPICAL
UL	UPPER LEVEL
ULS	ULTIMATE LIMIT STATE
U/S	UNDERSIDE
U/N	UNLESS NOTED
UPT	UPTURNED
VB	VERTICAL BRACING
VEF	VERTICAL EACH FACE
Vf	FACTORED SHEAR IN kN
VIC	VERTICAL IN CENTRE
VERT	VERTICAL
VSC	VERTICALLY SLOTTED CONNECTION TO ALLOW FOR DEFLECTION
VXB	VERTICAL CROSS BRACING ('X' SHAPE)
W	STRUCTURAL STEEL WIDE FLANGE BEAM
WC	WIND COLUMN
WWF	STRUCTURAL STEEL WELDED WIDE FLANGE BEAM
W.W.F	WELDED WIRE FABRIC
W /	WITH
ZRP	ZINC RICH PAINT

DRAWING LEGEND

- HOLE THROUGH CONCRETE BEAM
- HOLE THROUGH STEEL BEAM
- UL / LL — UPPER LEVEL BM/JOIST; LOWER LEVEL BM/JOIST
- MOMENT CONNECTION
- SDF — STEP DOWN FOOTING IN DIRECTION OF ARROW
- SIM. X Y — X: SECTION NUMBER; Y: DRAWING REFERENCE
- EXISTING FRAMING
- CONCRETE MASONRY WALL
- FULLY GROUTED CONCRETE MASONRY WALL
- STRUCTURAL PRECAST CONCRETE
- CAST-IN PLACE CONCRETE
- EXISTING CONCRETE
- LOAD BEARING WALL OR COLUMN BELOW
- G/L — NEW GRID LINE
- X — EXISTING GRID LINE REFERENCE
- ELEVATION / LEVEL NAME — ELEVATION SYMBOL
- STEP IN SLAB ON GRADE
- X — DRAWING REVISION NUMBER
- CURRENT REVISION CLOUD
- CONCRETE MASONRY LINTEL OR BOND BEAM BLOCK
- SOIL (SEE GEOTECH REPORT)
- GRANULAR SUBBASE (SEE GEOTECH REPORT)

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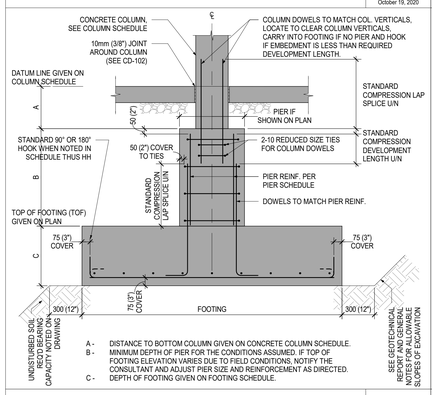
GENERAL NOTES

scale:	1 : 1
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S102
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

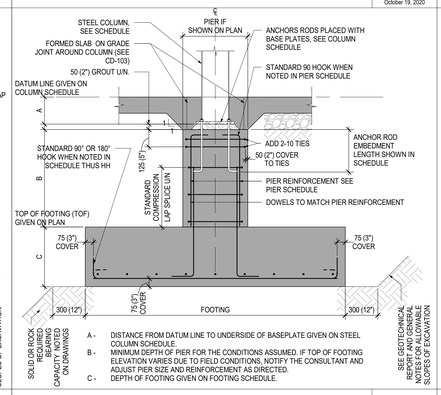
DRAWING LAST EDITED: 2026-07-03 3:04:50 PM

FOOTING AT CONCRETE COLUMN — CD-001
October 19, 2020



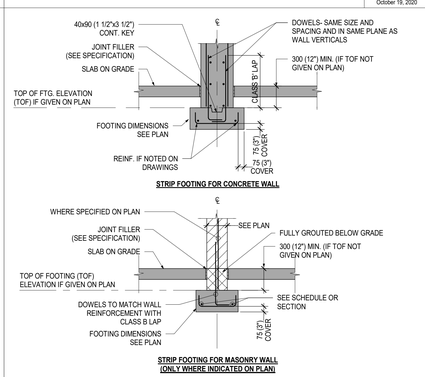
A - DISTANCE TO BOTTOM COLUMN GIVEN ON CONCRETE COLUMN SCHEDULE.
B - MINIMUM DEPTH OF PIER FOR THE CONDITIONS ASSUMED. IF TOP OF FOOTING ELEVATION VARIES DUE TO FIELD CONDITIONS, NOTIFY THE CONSULTANT AND ADJUST PIER SIZE AND REINFORCEMENT AS DIRECTED.
C - DEPTH OF FOOTING GIVEN ON FOOTING SCHEDULE.

FOOTING AT STEEL COLUMN — CD-002
October 19, 2020

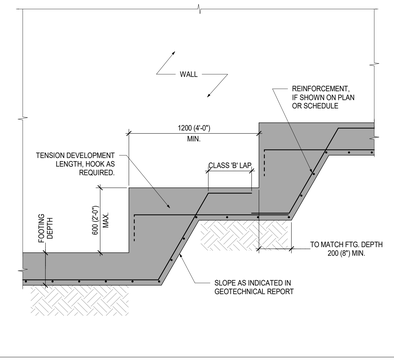


A - DISTANCE FROM DATUM LINE TO UNDERSIDE OF BASEPLATE GIVEN ON STEEL COLUMN SCHEDULE.
B - MINIMUM DEPTH OF PIER FOR THE CONDITIONS ASSUMED. IF TOP OF FOOTING ELEVATION VARIES DUE TO FIELD CONDITIONS, NOTIFY THE CONSULTANT AND ADJUST PIER SIZE AND REINFORCEMENT AS DIRECTED.
C - DEPTH OF FOOTING GIVEN ON FOOTING SCHEDULE.

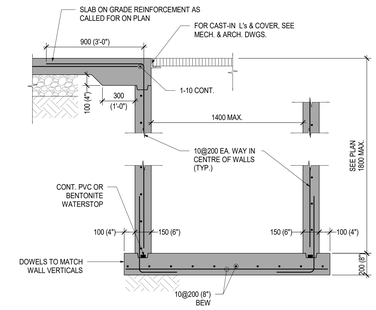
STRIP FOOTING FOR WALLS — CD-007
October 19, 2020



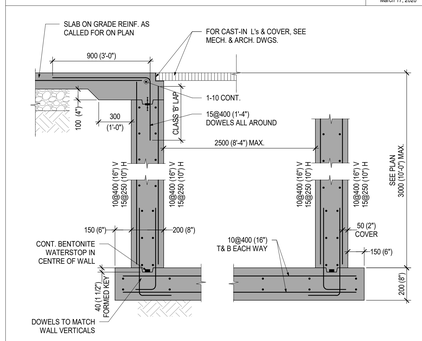
STEPPING OF STRIP FOOTING — CD-008
October 19, 2020



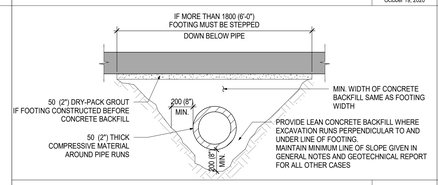
SUMP PIT DETAIL FOR PITS MAX. 1800 (6'-0") DEEP — CD-009
March 17, 2020



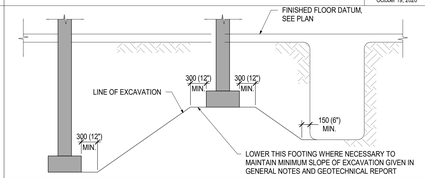
SUMP PIT DETAIL FOR PITS OVER 1800(6'-0") DEEP — CD-010
March 17, 2020



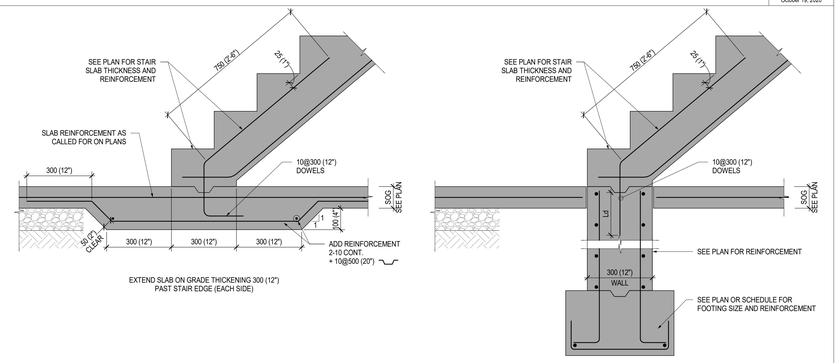
BACKFILL AROUND PIPE UNDER STRIP FOOTING — CD-011
October 19, 2020



SLOPE BETWEEN ADJACENT EXCAVATIONS — CD-012
October 19, 2020



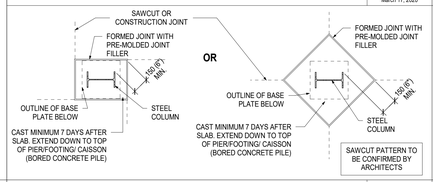
SLAB ON GRADE THICKENING UNDER CONCRETE STAIRS — CD-106
October 19, 2020



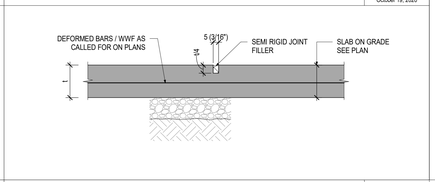
A) FOR STAIR FLIGHT LESS THAN 2.0m

A) FOR STAIR FLIGHT GREATER THAN 2.0m

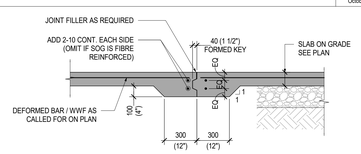
TYPICAL SLAB ON GRADE JOINTS AROUND STEEL COLUMN — CD-103
March 17, 2020



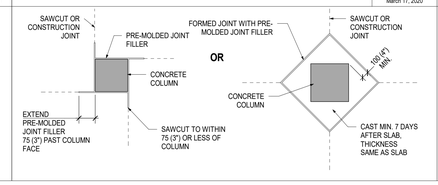
SAWCUT CONTROL JOINT IN SLAB ON GRADE — CD-100
October 19, 2020



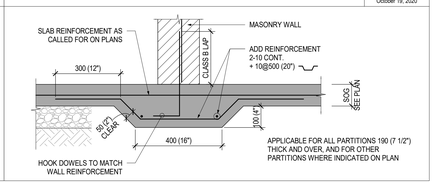
CONSTRUCTION JOINT IN SLAB ON GRADE — CD-101
October 19, 2020



SLAB ON GRADE JOINTS AROUND CONCRETE COLUMN — CD-102
March 17, 2020



SLAB ON GRADE THICKENING UNDER MASONRY PARTITIONS — CD-105
October 19, 2020



Key to Detail Location

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5	2026-07-08	ISSUED FOR TENDER

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TYPICAL DETAILS

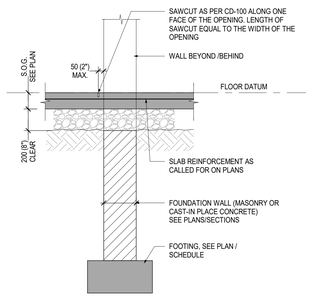
scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S103
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-03 3:04:51 PM

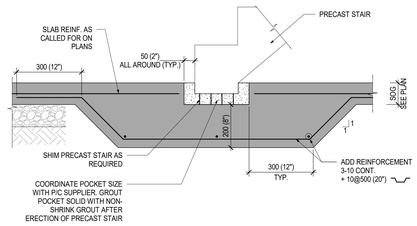
SLAB ON GRADE CONTROL JOINT AT INTERIOR DOORWAY

CD-104
March 17, 2020



SLAB ON GRADE THICKENING UNDER PRECAST STAIRS

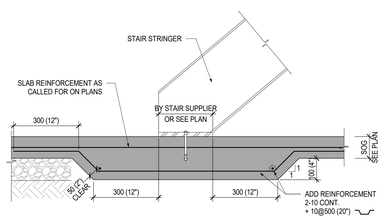
CD-107
October 19, 2020



EXTEND SLAB ON GRADE THICKENING 300 (12") PAST STAIR EDGE (EACH SIDE)

SLAB ON GRADE THICKENING UNDER STEEL STAIRS

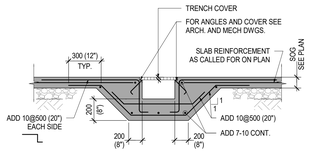
CD-108
October 19, 2020



EXTEND SLAB ON GRADE THICKENING 300 (12") PAST STAIR EDGE (EACH SIDE)

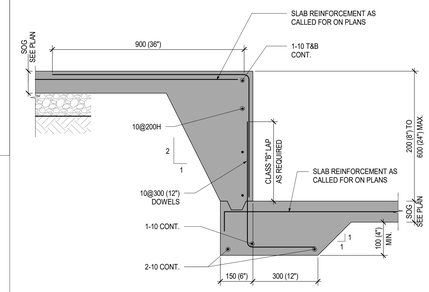
TRENCH DRAIN IN SLAB ON GRADE

CD-109
October 19, 2020



CHANGE IN SLAB ON GRADE LEVEL OF 200 (8") UP TO 600 (24")

CD-110
October 19, 2020



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	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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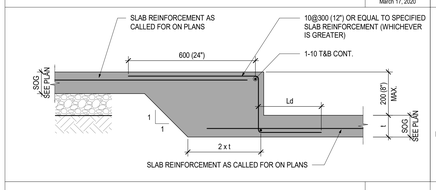
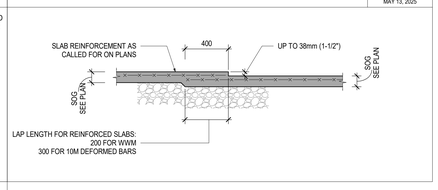
CHANGE IN SLAB ON GRADE LEVEL OF UP TO 200 (8")

CD-111
March 17, 2020



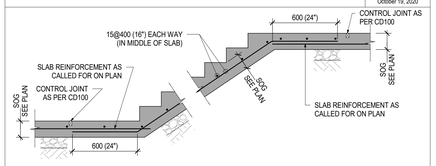
DEPRESSION IN SLAB-ON-GRADE UP TO 38mm (1-1/2")

CD-113
MAY 13, 2025



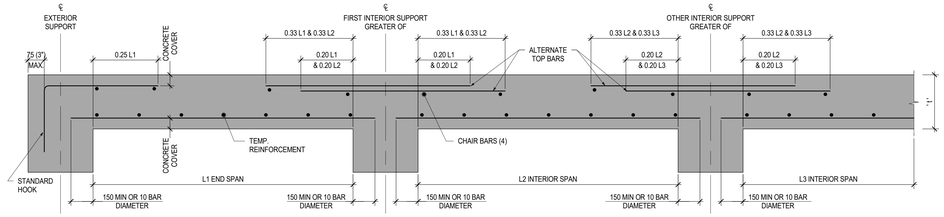
STAIR ON SLAB ON GRADE

CD-112
October 19, 2020

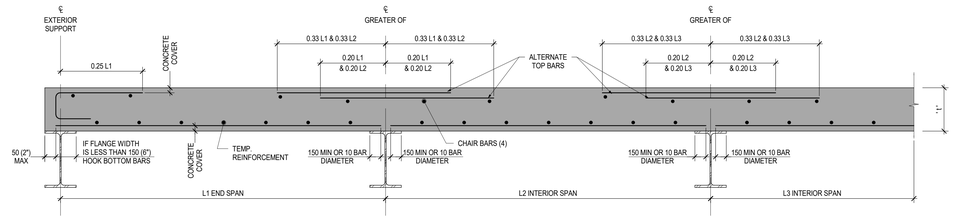


ONE WAY SLAB

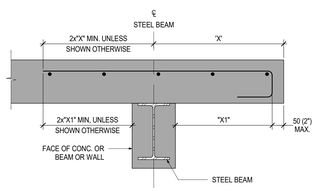
CD-200
October 19, 2020



SLAB SUPPORTED ON CONCRETE BEAMS OR WALLS



SLAB SUPPORTED ON STEEL BEAMS



CANTILEVERED SLAB TOP REINFORCEMENT

SLAB NOTES:

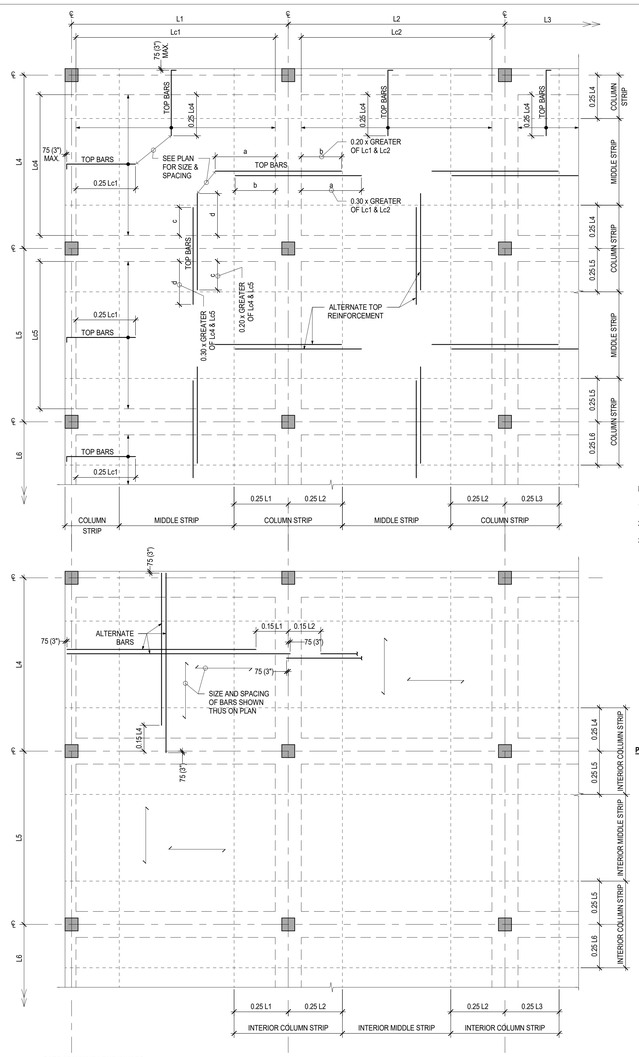
1. UNLESS OTHERWISE SHOWN OR NOTED ON PLAN, REINFORCEMENT IN ONE WAY SLABS SHALL BE PLACED AS SHOWN ABOVE.
2. MAXIMUM BAR SPACING IS 3 X SLAB THICKNESS BUT NOT GREATER THAN 500 (20").
3. PROVIDE 1-15 X 1800 (6'-0") LONG TOP BARS ACROSS RE-ENTRANT CORNERS UNLESS OTHERWISE SHOWN.
4. PROVIDE BAR SUPPORTS AND ACCESSORIES TO THE REQUIREMENTS OF R.S.I.C. MANUAL.

TEMPERATURE REINFORCEMENT FOR ONE WAY SLABS:
(As = 0.002 x b x t MIN.)

THICKNESS	REINFORCING	NOTES
100 (4")	10@500 (20")	A. BARS TO BE LAPPED USING CLASS 'B' LAP SPLICES.
120 (5")	10@400 (16")	B. MAXIMUM SPACING FOR TEMPERATURE BARS IS 5X SLAB THICKNESS BUT NOT GREATER THAN 500 (20") C/C.
140 (5.5")	10@350 (14")	
150 (6")	10@330 (13")	
160 (6.5")	10@310 (12")	
180 (7")	10@270 (11")	
200 (8")	15@500 (20")	
220 (9")	15@450 (18")	
240 (9.5")	15@410 (16")	
250 (10")	15@400 (16")	
280 (11")	15@350 (14")	
300 (12")	15@330 (13")	

TWO WAY SLAB WITH BEAMS

CD-201
October 19, 2020

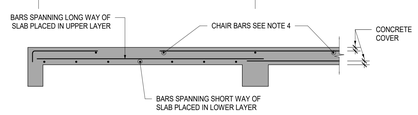


PLAN SHOWING REINFORCEMENT IN TOP OF SLAB
(SEE PLAN FOR ACTUAL REINFORCEMENT)

NOTES:

1. PLACE BOTTOM LOWER LAYERS AND TOP UPPER LAYER BARS PARALLEL TO SHORTER SPAN.
2. MAXIMUM SPACING TO BE 2x SLAB THICKNESS, BUT NOT MORE THAN 500(20").
3. PROVIDE BAR SUPPORTS AND ACCESSORIES TO THE REQUIREMENTS OF R.S.I.C. MANUAL.

PLAN SHOWING REINFORCEMENT IN BOTTOM OF SLAB
(SEE PLAN FOR ACTUAL REINFORCEMENT)



SECTION THROUGH TYPICAL EDGE, SLAB AND BEAM

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drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S105
Revision:	5

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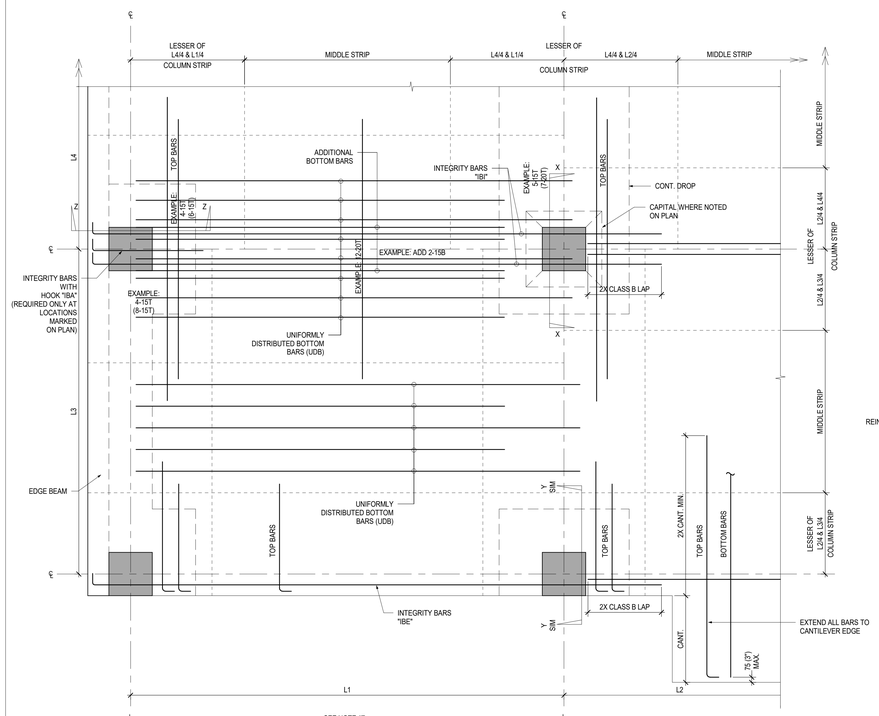
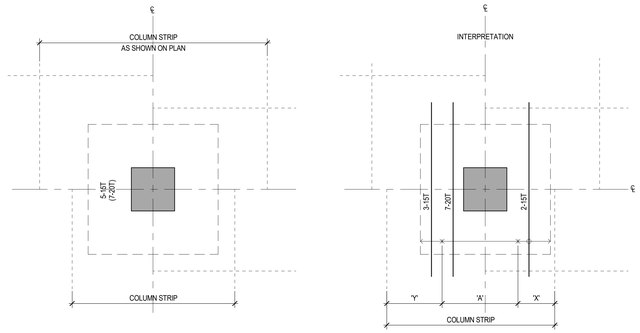
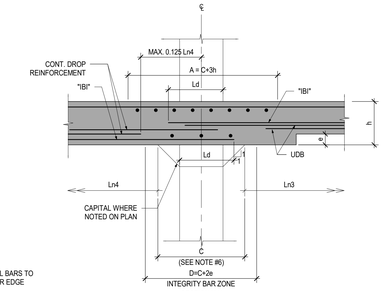
TYPICAL FLAT SLAB DETAILS

CD-202
March 11, 2021

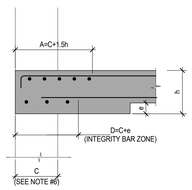




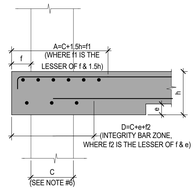
NOTE:
5-15T DISTRIBUTED IN RATIO OF DISTANCE "X" & "Y" (IN THIS CASE X/Y = 2/3)



SECTION X-X

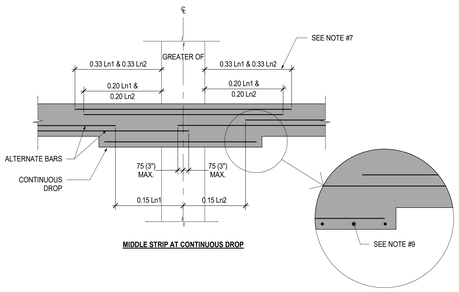


SECTION Y-Y

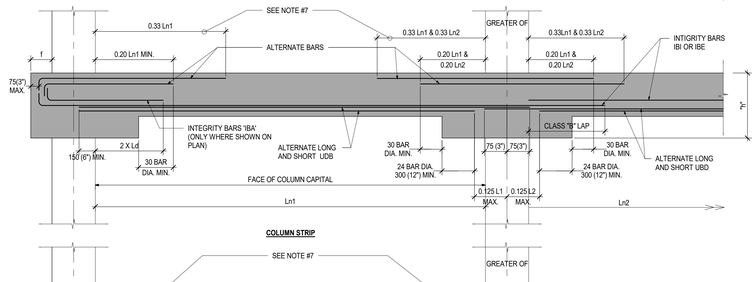


SECTION Z-Z

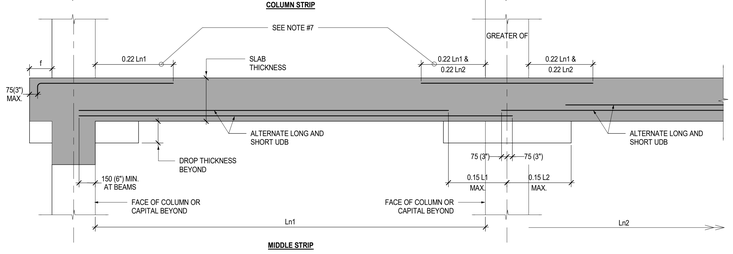
SEE PLAN FOR SLAB, DROP AND CAPITAL SIZES. REFER TO PLAN FOR SLAB REINFORCEMENT.



MIDDLE STRIP AT CONTINUOUS DROP



COLUMN STRIP



MIDDLE STRIP

NOTES:
1. BAR LENGTHS ARE MINIMUM CUTTING LENGTHS UNLESS OTHERWISE SHOWN ON PLAN.
2. UNLESS NOTED OTHERWISE, MAXIMUM BAR SPACING IS 3 x SLAB THICKNESS BUT NOT GREATER THAN 500 (20"). ADD BARS WHERE NECESSARY IN ORDER NOT TO EXCEED MAXIMUM SPACING. PLACE FIRST BAR HALF SPACING FROM EDGE OF STRIP.
3. BARS SHOWN ON THIS TYPICAL PLAN ARE DRAWN IN ONE DIRECTION ONLY FOR CLARITY AND NOT ALL OF THE BARS ARE DRAWN.
4. SEE PLANS FOR SLAB, DROP AND CAPITAL SIZE, FOR UNIFORMLY DISTRIBUTED BOTTOM REINFORCEMENT "UDB", FOR ORDER OF BAR PLACING AND FOR SIZE AND NUMBER OF INTEGRITY BARS "IBI, "IBE" AND "IBA"(SEE EXAMPLE).
5. PROVIDE SLAB BOLSTERS, CHAIRS AND SUPPORT BARS IN ACCORDANCE WITH R.S.I.C. MANUAL.
6. IF THERE ARE COLUMN CAPITALS, "C" IS THE WIDTH OF CAPITAL.
7. IF THERE ARE COLUMN CAPITALS, CUT LENGTHS OF ALL BARS AND CLEAR SPANS "Ln" SHALL BE MEASURED FROM FACE OF CAPITAL RATHER THAN FROM FACE OF COLUMN.
8. UNLESS OTHERWISE SHOWN ON PLAN, PLACE UDB REINFORCEMENT IN TWO ORTHOGONAL DIRECTIONS OVER THE ENTIRE SLAB, EXCEPT AT CONTINUOUS DROPS.
9. PLACE CONTINUOUS DROP REINFORCEMENT AT BOTTOM OF DROPS (BLL), AND ADD 10@490 BUL OVER DROP WIDTH.
10. ADD INTEGRITY BOTTOM BARS BETWEEN ALL COLUMNS OVER WIDTH "D". IBI INDICATES INTERIOR INTEGRITY BARS, TO BE PROVIDED BETWEEN ALL COLUMNS, EXCEPT BETWEEN COLUMNS ALONG SLAB EDGES, WHERE IBE (EXTERIOR INTEGRITY BARS) SHALL BE PROVIDED. ADDITIONAL INTEGRITY BARS IBA (IF APPLICABLE) SHALL ONLY BE PROVIDED AT LOCATIONS MARKED ON PLANS. OMIT INTEGRITY BARS AT BAYS WITH BEAMS.
11. PLACE ADDITIONAL COLUMN STRIP BOTTOM REINFORCEMENT WHERE SHOWN ON PLAN. LOCATE BETWEEN UDB (SPACING SAME AS FOR UDB), OUTSIDE THE INTEGRITY BARS' ZONE. CENTRE AT COLUMNS (WHERE POSSIBLE).
12. PLACE COLUMN STRIP TOP BARS SHOWN IN BRACKETS OVER WIDTH "A", AND PLACE THE OTHER TOP BARS EVENLY OVER THE REMAINDER OF COLUMN STRIP. SPACING OF THE BARS WITHIN WIDTH "A" NEVER TO EXCEED THE LESSER OF 250 (10") OR 1.5 x SLAB THICKNESS; LOCALLY ADJUST TOP BAR SPACING WHERE REQUIRED TO MEET THIS REQUIREMENT.
13. IN CASE OF VARYING WIDTHS OF COLUMN STRIPS, PLACE ALL COLUMN STRIP TOP BARS WITHIN THE NARROWER COLUMN STRIP, AND PLACE ALL MIDDLE STRIP TOP BARS UNIFORMLY IN BETWEEN.
14. ADD EXTRA BARS ON ALL SIDES OF OPENINGS EQUIVALENT TO INTERRUPTED BARS, SEE TYPICAL DETAIL CD-211.
15. SEE TYPICAL DETAIL CD-204 FOR LENGTHS OF MIDDLE STRIP REINFORCEMENT WHERE SPANS OF ADJACENT COLUMN STRIPS VARY.

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THE STERLING HALL SCHOOL

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TYPICAL DETAILS

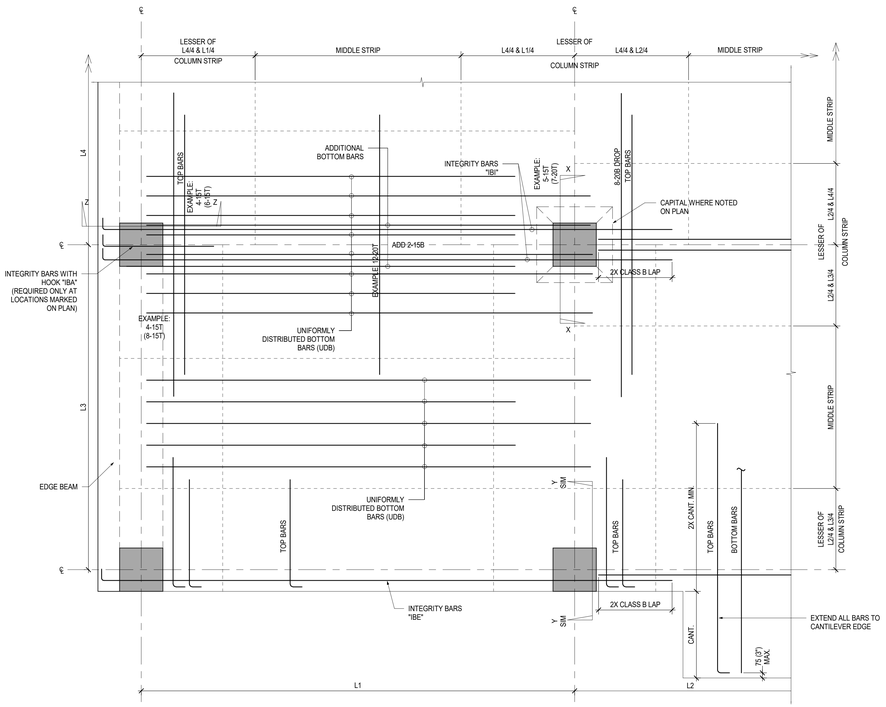
scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S106
	Revision: 5

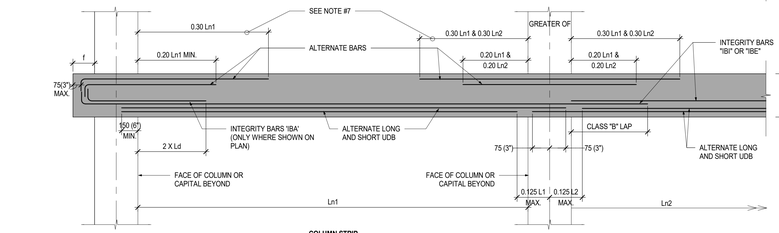
A0 FILENAME: Autodesk Docs://2032 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-03 3:04:52 PM

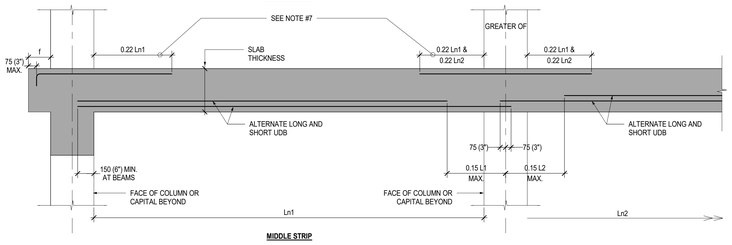
TYPICAL FLAT PLATE

CD-203
March 11, 2021

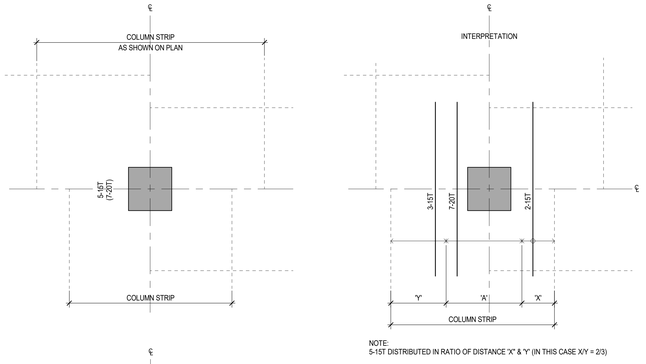




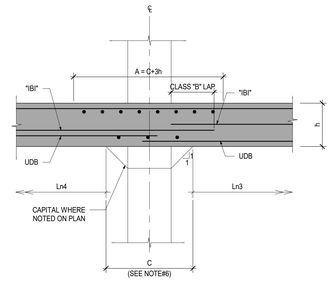
COLUMN STRIP



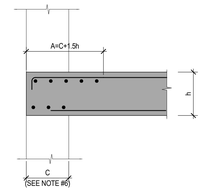
MIDDLE STRIP



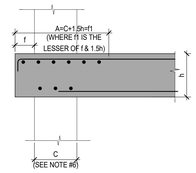
NOTE:
5-15T DISTRIBUTED IN RATIO OF DISTANCE "X" & "Y" (IN THIS CASE X/Y = 2/3)



SECTION X-X



SECTION Y-Y



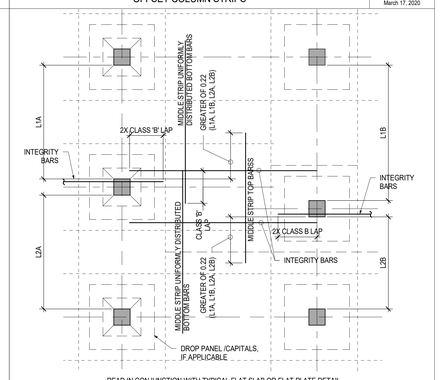
SECTION Z-Z

REFER TO PLAN FOR SLAB AND CAPITAL SIZES. REFER TO PLAN FOR SLAB REINFORCEMENT.

NOTES:
1. BAR LENGTHS ARE MINIMUM CUTTING LENGTHS UNLESS OTHERWISE SHOWN ON PLAN.
2. UNLESS NOTED OTHERWISE, MAXIMUM BAR SPACING IS 3 x SLAB THICKNESS BUT NOT GREATER THAN 500 (20"). ADD BARS WHERE NECESSARY IN ORDER NOT TO EXCEED MAXIMUM SPACING. PLACE FIRST BAR HALF SPACING FROM EDGE OF STRIP.
3. BARS SHOWN ON THIS TYPICAL PLAN ARE DRAWN IN ONE DIRECTION ONLY FOR CLARITY AND NOT ALL OF THE BARS ARE DRAWN.
4. SEE PLANS FOR SLAB AND CAPITAL SIZE, FOR UNIFORMLY DISTRIBUTED BOTTOM REINFORCEMENT "UDB", FOR ORDER OF BAR PLACING AND FOR SIZE AND NUMBER OF INTEGRITY BARS "IBI, "IBE" AND "IBA"(SEE EXAMPLE)
5. PROVIDE SLAB BOLSTERS, CHAIRS AND SUPPORT BARS IN ACCORDANCE WITH R.S.I.C. MANUAL.
6. IF THERE ARE COLUMN CAPITALS, "C" IS THE WIDTH OF CAPITAL.
7. IF THERE ARE COLUMN CAPITALS, CUT LENGTHS OF ALL BARS AND CLEAR SPANS "Ln" SHALL BE MEASURED FROM FACE OF CAPITAL RATHER THAN FROM FACE OF COLUMN.
8. UNLESS OTHERWISE SHOWN ON PLAN, PLACE UDB BOTTOM REINFORCEMENT IN TWO ORTHOGONAL DIRECTIONS OVER THE ENTIRE SLAB
9. ADD INTEGRITY BOTTOM BARS BETWEEN ALL COLUMNS OVER WIDTH "C". IBI INDICATES INTERIOR INTEGRITY BARS. TO BE PROVIDED BETWEEN ALL COLUMNS, EXCEPT BETWEEN COLUMNS ALONG SLAB EDGES, WHERE IBE (EXTERIOR INTEGRITY BARS) SHALL BE PROVIDED. ADDITIONAL INTEGRITY BARS IBA (IF APPLICABLE) SHALL ONLY BE PROVIDED AT LOCATIONS MARKED ON PLANS. OMIT INTEGRITY BARS AT BAYS WITH BEAMS.
10. PLACE ADDITIONAL COLUMN STRIP BOTTOM REINFORCEMENT WHERE SHOWN ON PLAN. LOCATE BETWEEN UDB (SPACING SAME AS FOR UDB), OUTSIDE THE INTEGRITY BARS' ZONE. CENTRE AT COLUMNS (WHERE POSSIBLE).
11. PLACE COLUMN STRIP TOP BARS SHOWN IN BRACKETS OVER WIDTH "A", AND PLACE THE OTHER TOP BARS EVENLY OVER THE REMAINDER OF COLUMN STRIP. SPACING OF THE BARS WITHIN WIDTH "A" NEVER TO EXCEED THE LESSER OF 250 (10") OR 1.5 x SLAB THICKNESS. LOCALLY ADJUST TOP BAR SPACING WHERE REQUIRED TO MEET THIS REQUIREMENT.
12. IN CASE OF VARYING WIDTHS OF COLUMN STRIPS, PLACE ALL COLUMN STRIP TOP BARS WITHIN THE NARROWER COLUMN STRIP, AND PLACE ALL MIDDLE STRIP TOP BARS UNIFORMLY IN BETWEEN.
13. ADD EXTRA BARS ON ALL SIDES OF OPENINGS EQUIVALENT TO INTERRUPTED BARS, SEE TYPICAL DETAIL CD-211.
14. SEE TYPICAL DETAIL CD-204 FOR LENGTHS OF MIDDLE STRIP REINFORCEMENT WHERE SPANS OF ADJACENT COLUMN STRIPS VARY.

TWO-WAY SLAB REINFORCEMENT DETAIL AT OFFSET COLUMN STRIPS

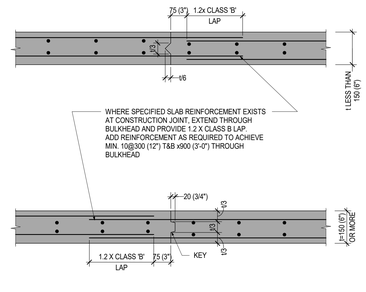
CD-204
March 17, 2020



READ IN CONJUNCTION WITH TYPICAL FLAT SLAB OR FLAT PLATE DETAIL

CONSTRUCTION JOINT IN FRAMED SLAB

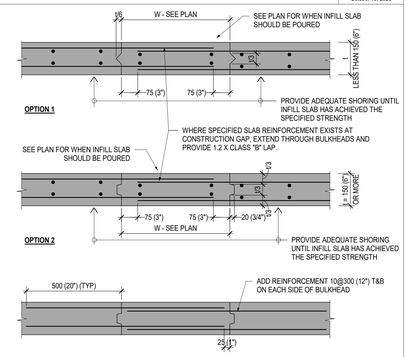
CD-205
March 17, 2020



NOTE:
CONSTRUCTION JOINT TO BE WITHIN MIDDLE THIRD OF SPAN. LOCATIONS TO BE APPROVED BY THE STRUCTURAL CONSULTANT.

CONSTRUCTION GAP IN FRAMED SLAB

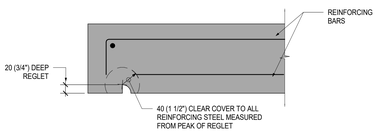
CD-206
October 19, 2020



OPTION 1 OR 2 WHEN NO REINFORCEMENT CALLED ON PLAN
(REFER TO OPTION 1 OR 2 FOR REMAINING DETAIL)

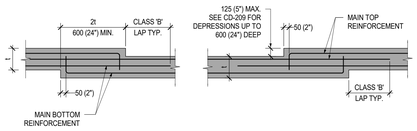
EXPOSED CONCRETE OVERHANG

CD-207
March 17, 2020



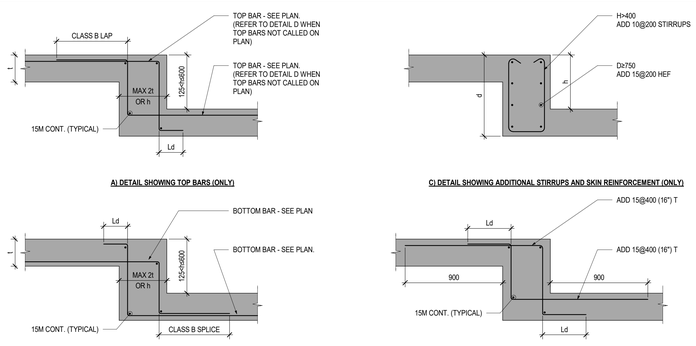
SPLICING OF REINFORCEMENT AT DEPRESSION IN FRAMED SLAB

CD-208
October 19, 2020



SPLICING OF REINFORCEMENT AT STEP IN FRAMED SLAB

CD-209
August 18, 2020



A) DETAIL SHOWING TOP BARS (ONLY)

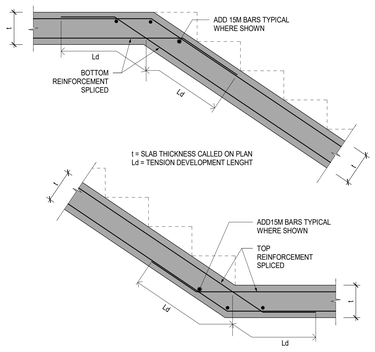
C) DETAIL SHOWING ADDITIONAL STIRRUPS AND SKIN REINFORCEMENT (ONLY)

B) DETAIL SHOWING BOTTOM BARS (ONLY)

D) ADDITIONAL TOP REINFORCEMENT WHEN NO TOP REINFORCEMENT IS CALLED ON PLAN

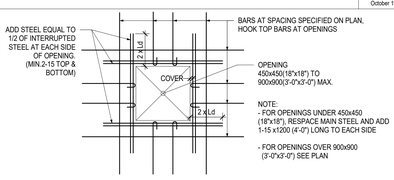
SPLICING OF REINFORCEMENT IN CRANKED SLAB

CD-210
October 19, 2020



ADDED REINFORCEMENT AROUND OPENING IN SLAB

CD-211
October 19, 2020



Key to Detail Location

NO. — Detail Number
NO. — Drawing Number

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2	2026-03-13	ISSUED FOR BUILDING PERMIT
3	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

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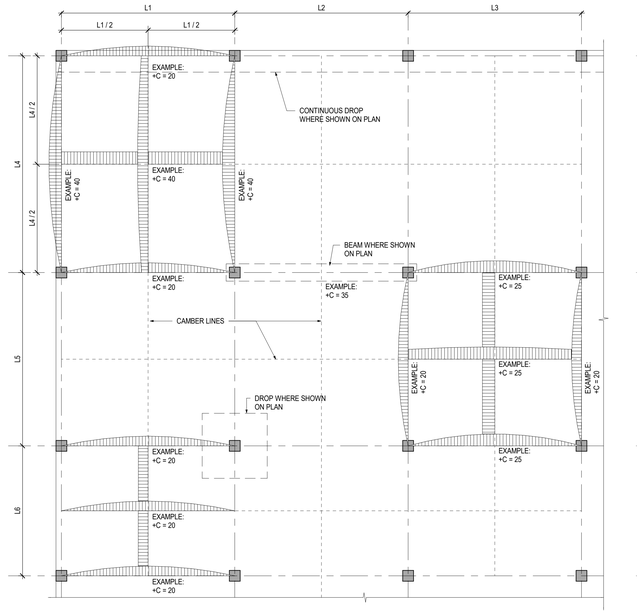
TYPICAL DETAILS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S107
	Revision: 5

FRAMED SLAB CAMBERS

CD-212
October 19, 2020

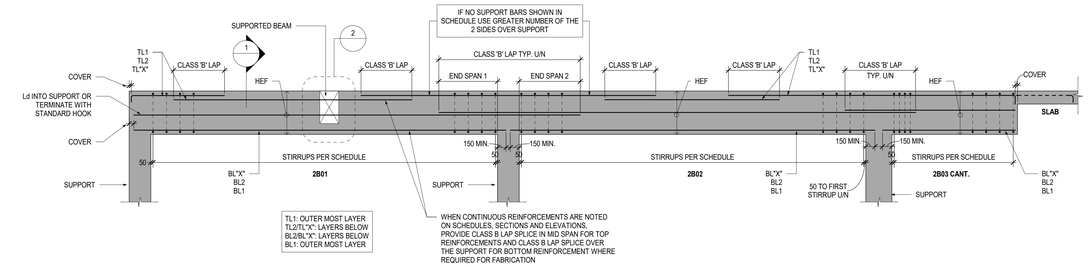


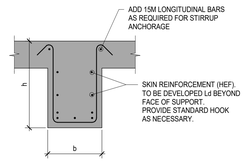
CAMBER NOTES:

1. PROVIDE UPWARD SLAB AND BEAM CAMBER AS INDICATED "+C = XX" ON PLAN WHERE "XX" DENOTES AMOUNT OF CAMBER IN MILLIMETER FOR METRIC PROJECT AND INCH FOR IMPERIAL PROJECT.
2. CAMBER BOTH THE UNDERSIDE AND THE TOP OF CONCRETE ALONG CAMBER LINES IN A PARABOLIC PROFILE SO AS TO MAINTAIN THE STRUCTURAL THICKNESS SPECIFIED ON DRAWINGS.
3. BEFORE PLACING CONCRETE, SURVEY TOP OF FORMWORK AT SUPPORTS, AT MIDSPANS BETWEEN SUPPORTS AND AT CENTERS OF BAYS.

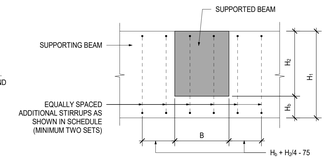
CONCRETE BEAMS

CD-300
March 11, 2021





SECTION '1'

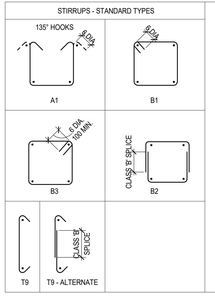


SECTION '2'
TYPICAL ADDED STIRRUPS IN SUPPORTING BEAMS AT SUPPORTED BEAM

SAMPLE BEAM SCHEDULE

MARK SIZE	BAR LOCATION	END 1	SUPPORT	MAIN SPAN + # OF BARS	SUPPORT	END 2	STIRRUPS (#-SIZE@SPACING / LOCATION - TYPE) LOCATION: L - @ LEFT SUPPORT; R - @ RIGHT SUPPORT; REM - REMAINDER THR - THROUGHOUT
*SUPPORT TO THE LEFT OF BEAM MARK ON PLAN DENOTES LEFT SUPPORT WHEREAS SUPPORT TO THE RIGHT OF THE BEAM MARK ON PLAN DENOTES RIGHT SUPPORT							
2B01 1000x1100 DP.	TL1			5-20		INTO 2B02	10-15 @ 175 / L - 2B1+T9 10-15 @ 175 / R - 2B1+T9 15@400 / REM B1
	HEF			15@200			
	BL2			4-30			
	BL1			4-30			
2B02 1000x1100 DP.	TL1	FROM 2B01		5-20 5-25 CLASS 'B' LAP @ MID BAY		INTO 2B03	10-15 @ 150 / L A1 15@250 / REM B3
	HEF			15@200			
	BL3		150	5-25	150		
	BL2			5-30			
	BL1			5-35			
2B03 CANT. 1000x1100 DP.	TL1	FROM 2B02		5-25 2500	3-20	2500 INTO SLAB	15@200 / THR - 2B1
	HEF			15@200			
	BL1			5-15			

STIRRUPS - STANDARD TYPES



STIRRUP NOTES:

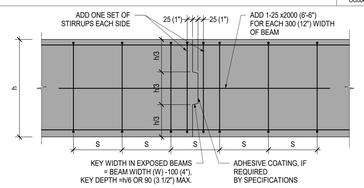
1. TYPICAL STIRRUP MARKS A1, B1, ETC. SHOWN IN BEAM SCHEDULE.
2. FIRST SET OF STIRRUP TO BE PLACED AT HALF OF THE TYPICAL SPACING (50) FROM FACE OF SUPPORT.
3. MULTIPLE STIRRUPS ARE SHOWN AS "2 B1", "B1+T9" IN THE BEAM SCHEDULE OR BEAM ELEVATIONS.

4. TYPE B1 MUST BE THE OUTERMOST STIRRUP ENCLOSING THE ENTIRE BEAM WHEN COMBINED WITH STIRRUPS TYPE A1 OR T9.

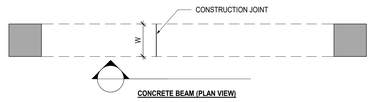
MIN. CLEAR DISTANCE BETWEEN DEFORMED BARS FOR 20mm AGGREGATE	
BAR SIZE	CLEARANCE, mm
15 TO 20	30
25	35
30	42
35	49
45	63
55	77

CONSTRUCTION JOINT IN BEAM

CD-301
October 19, 2020



CONCRETE BEAM (ELEVATION VIEW)



CONCRETE BEAM (PLAN VIEW)

NOTE:
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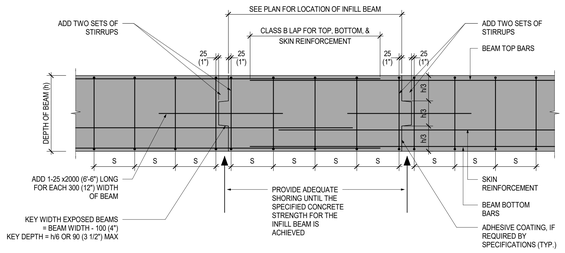
S - SPACING OF STIRRUPS AS SHOWN ON BEAM SCHEDULE

CONSTRUCTION GAP IN BEAM

CD-302
August 18, 2020



NOTE:
1. REFER TO BEAM SCHEDULES / ELEVATIONS / SECTIONS FOR BEAM REINFORCING.
2. "S" DENOTE STIRRUPS SPACING.

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2026-07-08
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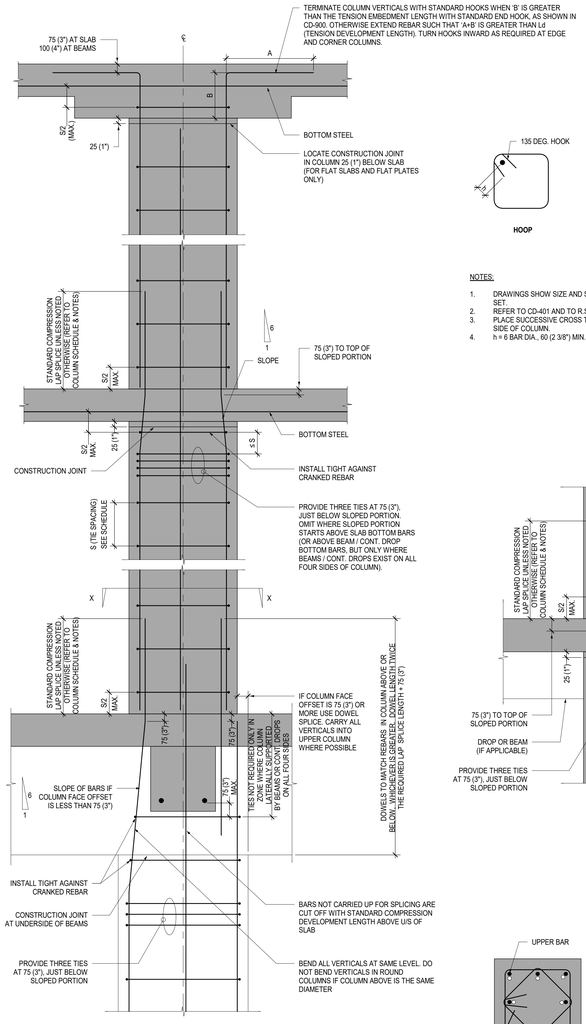
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-03 3:04:53 PM

CONCRETE COLUMNS AND REINFORCED PIERS

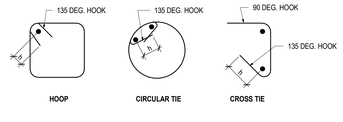
CD-400
October 19, 2020



INTERIOR COLUMN

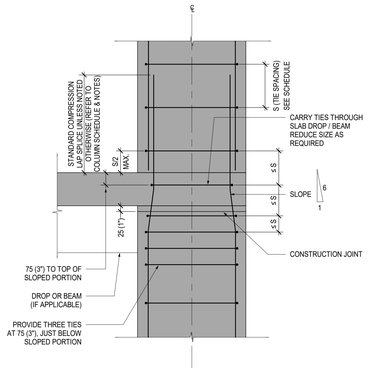
A. IF COLUMNS ARE CAST INTEGRALLY WITH SHEAR WALLS, REFER TO SHEAR WALL DETAILS
B. IF COLUMNS ARE CAST INTEGRALLY WITH SHEAR WALLS IN WHICH BENT CONCENTRATED REINFORCEMENT IS PROHIBITED, PROVIDE DOWELS BETWEEN COLUMN LIFTS INSTEAD OF USING OFFSET BENT VERTICALS.

TYPICAL TIE SHAPES

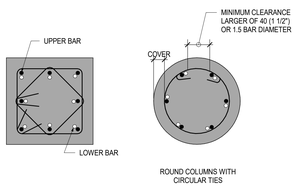


NOTES:

1. DRAWINGS SHOW SIZE AND SPACING OF TIE SETS. "TIE" ON DRAWINGS MEANS TIE SET.
2. REFER TO CD-401 AND TO R.S.I.C. MANUAL FOR TIE ARRANGEMENT .
3. PLACE SUCCESSIVE CROSS TIES WITH 135° HOOKS ALTERNATING FROM SIDE TO SIDE OF COLUMN.
4. h = 6 BAR DIA., 60 (2 3/8") MIN.



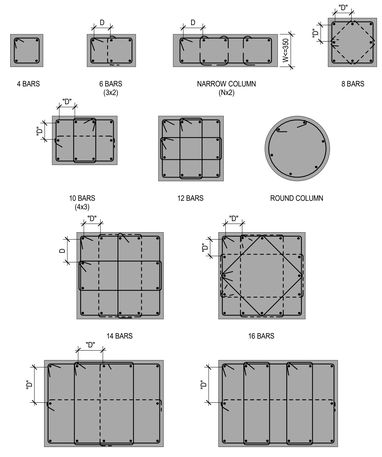
PERIMETER / CORNER COLUMN



SECTION X-X **SECTION X-X**

TIE ARRANGEMENTS FOR COLUMNS, BORED CONCRETE PILES AND REINFORCED PIERS

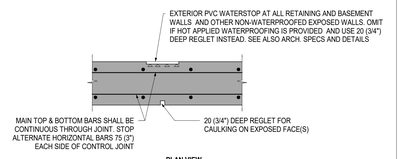
CD-401
October 19, 2020



- FOR RECTANGULAR COLUMN, VERTICALS ARRANGEMENT IS GIVEN IN (YxZ) WITH Y BEING THE # OF VERTICALS PARALLEL TO THE LONG SIDE OF THE COLUMN AND Z BEING THE # OF VERTICALS PARALLEL TO THE SHORT SIDE OF THE COLUMN.
- FOR SQUARE COLUMNS, VERTICALS TO BE PLACED EQUALLY ON EACH SIDE OF COLUMN.
- IF DIMENSION "D" GREATER THAN 150(6") OR IF ADDITIONAL TIES ARE SPECIFIED, ADD TIES SHOWN DASHED.
- SEE CD-400 FOR "TYPICAL TIE SHAPES".
- FOR COLUMNS LESS THAN OR EQUAL TO 350 WIDE, REFER TO "NARROW COLUMN" DETAIL.

CONTROL JOINT IN CONCRETE WALL

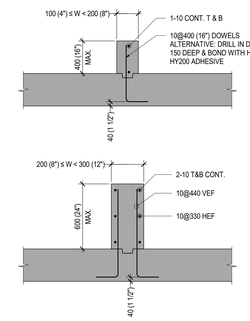
CD-501
October 19, 2020



PLAN VIEW

CURB DETAIL

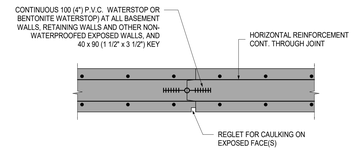
CD-905
October 19, 2020



NOTE:
1. FOR CURB SIZES AND LOCATIONS, REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. UNLESS OTHERWISE NOTED, THIS DETAIL DOES NOT APPLY FOR CURBS SUPPORTING BALUSTRADE, RAILING, FALL ARREST ANCHORS OR WALL WITHOUT LATERAL SUPPORT AT TOP.
3. ALTERNATIVE: DRILL IN DOWELS 150 (6") DEEP & BOND WITH HILTI HY200 ADHESIVE.

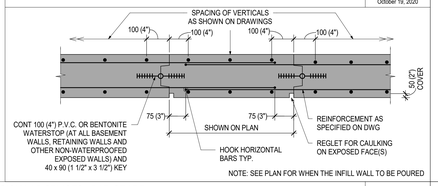
CONSTRUCTION JOINT IN WALL

CD-502
March 17, 2020



CONSTRUCTION GAP IN WALL

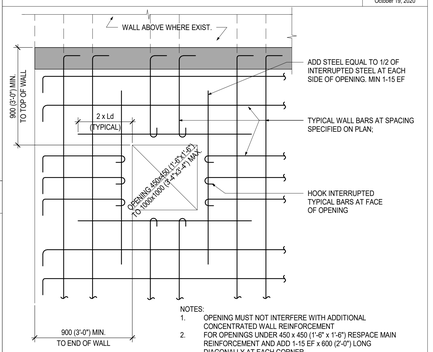
CD-504
October 19, 2020



NOTE: SEE PLAN FOR WHEN THE INFILL WALL TO BE POURED

ADDED REINFORCEMENT AROUND OPENING IN WALL

CD-505
October 19, 2020



NOTES:
1. OPENING MUST NOT INTERFERE WITH ADDITIONAL CONCENTRATED WALL REINFORCEMENT
2. FOR OPENINGS UNDER 450 x 450 (1'-8" x 1'-6") RESPACE MAIN REINFORCEMENT AND ADD 1-15 EF x 600 (2'-0") LONG DIAGONALLY AT EACH CORNER
3. FOR OPENINGS OVER 1000 x 1000 (3'-4" x 3'-4") SEE PLANS OR ELEVATIONS.

TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPLICES - BAR GRADE 400 MPa

CD-900
March 17, 2020

FOR CONCRETE STRENGTHS NOT SHOWN BETWEEN 25 TO 50, VALUES ARE TO BE INTERPOLATED FROM NEAREST VALUES PROVIDED IN TABLE BELOW.

TENSION DEVELOPMENT LENGTH FOR BAR GRADE 400 MPa, Ld, IN mm (in")

- BLACK BARS (IN MEMBERS CONTAINING MINIMUM TIES OR STIRRUPS, A MINIMUM SPACING OF 2.0db, AND CLEAR COVER OF A MINIMUM OF 1.0db; OR ALL SLABS AND WALLS)
- FOR EPOXY BARS MULTIPLY VALUES IN TABLE BY 1.5 EXCEPT THAT A MULTIPLIER OF 1.2 CAN BE USED WHEN CLEAR COVER IS MORE THAN 3x BAR DIAMETER AND CLEAR SPACING BETWEEN BARS IS MORE THAN 6x BAR DIAMETER.
- FOR SEMI LOW DENSITY CONCRETE MULTIPLY VALUES IN TABLE BY 1.2.
- FOR BUNDLED BARS, MULTIPLY VALUES IN TABLE BY 10% FOR A TWO BAR BUNDLE, 20% FOR A THREE BAR BUNDLE AND 33% FOR A FOUR BAR BUNDLE
- "TOP" MEANS THAT THERE IS MORE THAN 300 (12") OF CONCRETE BELOW, AND LESS THAN 300 (12") OF CONCRETE ABOVE THE HORIZONTAL BAR WITHIN THE INDIVIDUAL CONCRETE POUR.
- ALL VERTICAL BARS ARE CONSIDERED "BOTTOM" WITH MINIMUM STIRRUPS OR TRANSVERSE REINFORCEMENT AND A MIN SPACING OF 2.0Db

BAR SIZE	f'c = 25		f'c = 30		f'c = 35		f'c = 40		f'c = 50	
	BOTTOM	TOP	BOTTOM	TOP	BOTTOM	TOP	BOTTOM	TOP	BOTTOM	TOP
10	300 (12")	380 (15")	300 (12")	350 (14")	300 (12")	320 (13")	300 (12")	300 (12")	300 (12")	300 (12")
15	440 (17")	570 (23")	400 (16")	520 (21")	370 (14")	480 (19")	350 (14")	450 (18")	310 (12")	400 (16")
20	580 (23")	750 (30")	530 (21")	690 (27")	490 (19")	640 (25")	460 (18")	600 (24")	410 (16")	530 (21")
25	900 (36")	1170 (46")	830 (32")	1070 (42")	770 (30")	990 (39")	720 (28")	930 (37")	640 (25")	830 (33")
30	1080 (43")	1410 (55")	990 (39")	1290 (51")	920 (36")	1190 (47")	860 (34")	1110 (44")	770 (30")	1000 (39")
35	1260 (50")	1640 (65")	1160 (46")	1500 (60")	1070 (42")	1390 (55")	1000 (40")	1300 (52")	900 (35")	1160 (46")

CLASS B TENSION LAP SPLICES LENGTH FOR BAR GRADE 400 MPa IN mm (in")

- BLACK BARS (IN MEMBERS CONTAINING MINIMUM TIES OR STIRRUPS, A MINIMUM SPACING OF 2.0db, AND CLEAR COVER OF A MINIMUM OF 1.0db; OR ALL SLABS AND WALLS)
- FOR EPOXY BARS MULTIPLY VALUES IN TABLE BY 1.5 EXCEPT THAT A MULTIPLIER OF 1.2 CAN BE USED WHEN CLEAR COVER IS MORE THAN 3x BAR DIAMETER AND CLEAR SPACING BETWEEN BARS IS MORE THAN 6x BAR DIAMETER
- FOR SEMI LOW DENSITY CONCRETE MULTIPLY VALUES IN TABLE BY 1.2
- FOR BUNDLED BARS, MULTIPLY VALUES IN TABLE BY 10% FOR A TWO BAR BUNDLE, 20% FOR A THREE BAR BUNDLE AND 33% FOR A FOUR BAR BUNDLE
- "TOP" MEANS THAT THERE IS MORE THAN 300 (12") OF CONCRETE BELOW, AND LESS THAN 300 (12") OF CONCRETE ABOVE THE HORIZONTAL BAR WITHIN THE INDIVIDUAL CONCRETE POUR.
- ALL VERTICAL BARS ARE CONSIDERED "BOTTOM"

BAR SIZE	f'c = 25		f'c = 30		f'c = 35		f'c = 40		f'c = 50	
	BOTTOM	TOP	BOTTOM	TOP	BOTTOM	TOP	BOTTOM	TOP	BOTTOM	TOP
10	380 (15")	490 (19")	350 (14")	450 (18")	320 (13")	420 (17")	300 (12")	390 (15")	300 (12")	350 (14")
15	570 (23")	740 (29")	520 (21")	670 (27")	480 (19")	620 (25")	450 (18")	580 (23")	400 (16")	520 (20")
20	750 (29")	980 (39")	690 (27")	890 (35")	640 (25")	830 (33")	600 (24")	770 (30")	530 (21")	690 (27")
25	1170 (46")	1530 (61")	1070 (42")	1390 (55")	990 (39")	1290 (51")	930 (37")	1210 (48")	830 (33")	1080 (43")
30	1410 (56")	1830 (72")	1290 (51")	1670 (66")	1190 (47")	1550 (61")	1110 (44")	1450 (57")	1000 (39")	1300 (51")
35	1640 (65")	2130 (84")	1500 (60")	1950 (77")	1390 (55")	1800 (72")	1300 (52")	1690 (67")	1160 (46")	1510 (59")

MINIMUM TENSION EMBEDMENT LENGTH WITH STANDARD END HOOKS FOR BAR GRADE 400 MPa IN mm (in")

- BLACK BARS
- FOR EPOXY BARS MULTIPLY VALUES IN TABLE BY 1.2
- FOR SEMI LOW DENSITY CONCRETE MULTIPLY VALUES IN TABLE BY 1.2
- IF SIDE COVER (PERPENDICULAR TO THE PLANE OF THE HOOK) IS < 60 (2.5") DIVIDE VALUES IN TABLE BY 0.70.

BAR SIZE	f'c = 25	f'c = 30	f'c = 35	f'c = 40	f'c = 50
10	150 (6")	150 (6")	150 (6")	150 (6")	150 (6")
15	210 (8")	200 (8")	180 (7")	170 (7")	150 (6")
20	280 (11")	260 (10")	240 (10")	230 (9")	200 (8")
25	350 (14")	320 (13")	300 (12")	280 (11")	250 (9")
30	420 (17")	390 (16")	360 (14")	340 (14")	300 (11")
35	490 (20")	450 (18")	420 (17")	390 (16")	350 (13")

COMPRESSION DEVELOPMENT LENGTHS AND LAP SPLICES LENGTHS - BAR GRADE 400 MPa

CD-901
March 17, 2020

COMPRESSION DEVELOPMENT LENGTH FOR BAR GRADE 400 MPa, Ldc IN mm (in")

BAR SIZE	f'c = 25	f'c = 30	f'c = 35	f'c = 40	f'c = 50
10	220 (9")	200 (8")	200 (8")	200 (8")	200 (8")
15	310 (12")	280 (11")	280 (11")	280 (11")	280 (11")
20	380 (15")	340 (13")	340 (13")	340 (13")	340 (13")
25	490 (19")	440 (17")	440 (17")	440 (17")	440 (17")
30	580 (23")	530 (21")	530 (21")	530 (21")	530 (21")
35	690 (27")	630 (25")	630 (25")	630 (25")	630 (25")

COMPRESSION LAP SPLICE LENGTHS IN mm (in")

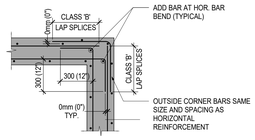
BAR SIZE	STANDARD LAP
10	300 (12")
15	440 (17")
20	590 (23")
25	730 (29")
30	880 (35")
35	1030 (41")

- IF VALUE NOT PROVIDED IN TABLE FOR ANY CONCRETE STRENGTH BETWEEN 25 TO 50, VALUES ARE TO BE INTERPOLATED FROM NEAREST VALUES PROVIDED.

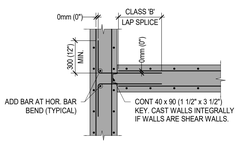
- FOR BUNDLED BARS, MULTIPLY VALUES IN TABLE BY 10% FOR A TWO BAR BUNDLE, 20% FOR A THREE BAR BUNDLE AND 33% FOR A FOUR BAR BUNDLE

INTERSECTING CONCRETE WALL REINFORCEMENT DETAILS

CD-500
March 17, 2020



LAPPING AT CORNER WALL



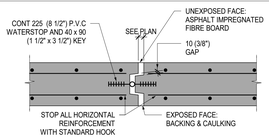
DOWELS SAME SIZE AND SPACING AS HORIZONTAL REINFORCEMENT. DOWELS NOT REQUIRED IF WALLS CAST TOGETHER AND HORIZONTAL REINFORCEMENT EXTENDED INTO INTERSECTING WALL AND HOOKED AS SHOWN.

DOWELING OF INTERSECTING WALLS

N.B. ALL WALL REINFORCEMENT TO BE SPLICED WITH CLASS 'B' LAPS

KEYED EXPANSION JOINT

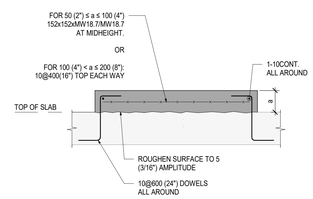
CD-503
October 19, 2020



IN BASEMENT WALLS, RETAINING WALLS AND OTHER NON-WATERPROOFED EXPOSED WALLS

HOUSEKEEPING PADS

CD-904
March 17, 2020



ALTERNATIVE: DRILL IN DOWELS 150 (6") DEEP & BOND WITH HILTI HY200 ADHESIVE

NOTES:
1. FOR PAD SIZES AND LOCATIONS, REFER TO MECHANICAL AND ELECTRICAL DRAWINGS
2. FOR PADS MORE THAN 200 (8") THICK OBTAIN CONSULTANT'S APPROVAL PRIOR TO POURING.

Key to Detail Location

NO. — Detail Number
NO. — Drawing Number

If this sheet is not 46 7/8" x 33 1/8" (1189 x 841 mm) It is a reduced print - Read dwg. accordingly.

Contractors must check and verify all dimensions on the job and report any discrepancies to the Architect before proceeding with the work.

All prints and specifications are the property of the Architect and must be returned at the completion of the work.

Drawings should not be scaled.

No	Date	Revision/Issued
1	2025-12-15	ISSUED FOR 30% CONSTRUCTION DOCUMENTS
2	2026-03-13	ISSUED FOR BUILDING PERMIT
3	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

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structural engineer — LEA Consulting Ltd. 40 University Ave, Suite 503 Toronto, ON, M5J 1T1 Phone.905.470.0015

mechanical engineer — Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151

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landscape architect — Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

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2026-07-08
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THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

TYPICAL DETAILS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S109
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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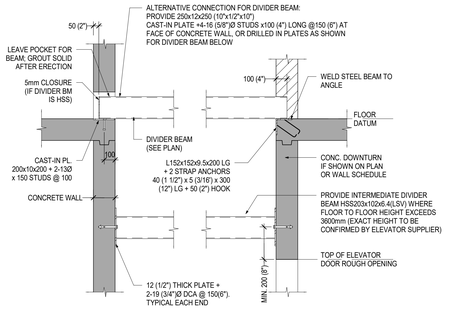
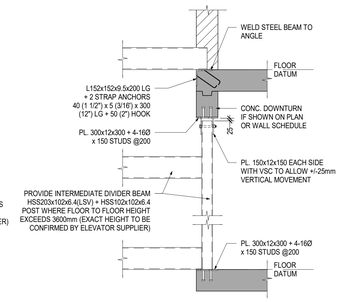
ELEVATOR DIVIDER BEAM

GD-001
October 29, 2020



CONDITION A

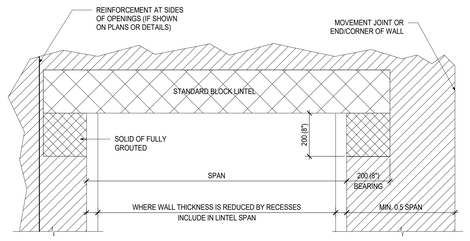


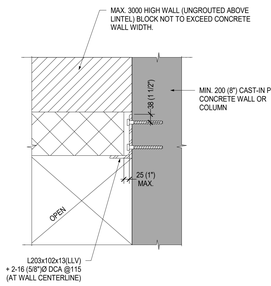
CONDITION B

TYPICAL MASONRY LINTELS IN NON-LOAD BEARING WALLS

MD-300
October 29, 2020

SEE SD-312 FOR STANDARD STEEL LINTELS

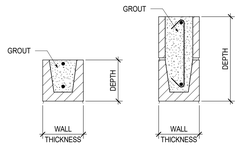




WALL THICKNESS	SPAN	BLOCK LINTEL DEPTH	REINFORCEMENT TOP & BOTTOM
90 (4")	UP TO 1200 (4'-0")	190	1-10
	1200 (4'-0") TO 1800 (6'-0")	N/A	N/A
	1800 (6'-0") TO 2400 (8'-0")	N/A	N/A
	2400 (8'-0") TO 3000 (10'-0")	N/A	N/A
140 (6")	UP TO 1200 (4'-0")	190	1-10
	1200 (4'-0") TO 1800 (6'-0")	190	1-10
	1800 (6'-0") TO 2400 (8'-0")	390	1-10 +1-10@200 (8") STIRRUP
	2400 (8'-0") TO 3000 (10'-0")	N/A	N/A
190 (8")	UP TO 1200 (4'-0")	190	1-15
	1200 (4'-0") TO 1800 (6'-0")	190	1-15
	1800 (6'-0") TO 2400 (8'-0")	390	1-10 +1-10@200 (8") STIRRUP
	2400 (8'-0") TO 3000 (10'-0")	390	1-10 +1-10@200 (8") STIRRUP
240 (10")	UP TO 1200 (4'-0")	190	1-15
	1200 (4'-0") TO 1800 (6'-0")	190	1-15
	1800 (6'-0") TO 2400 (8'-0")	390	1-10 +1-10@200 (8") STIRRUP
	2400 (8'-0") TO 3000 (10'-0")	390	1-10 +1-10@200 (8") STIRRUP
290 (12")	UP TO 1200 (4'-0")	190	1-15
	1200 (4'-0") TO 1800 (6'-0")	190	1-15
	1800 (6'-0") TO 2400 (8'-0")	390	1-10 +1-10@200 (8") STIRRUP
	2400 (8'-0") TO 3000 (10'-0")	390	1-10 +1-10@200 (8") STIRRUP

NOTES:

1. DO NOT USE MASONRY LINTELS FOR OPENINGS WHERE MOVEMENT JOINT OR WALL END OR CORNER IS LOCATED LESS THAN 0.5 OF THE LINTEL SPAN AWAY FROM THE FACE OF THE OPENING.
2. STANDARD LINTELS ARE NOT NECESSARILY SHOWN ON STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR OPENING LOCATIONS.
3. SEE PLANS FOR SPECIAL LINTELS.
4. FILL LINTEL BLOCKS WITH 20MPa GROUT. IT IS NOT ACCEPTABLE TO FILL LINTELS WITH MORTAR.



MASONRY LINTEL

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4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

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THE STERLING HALL SCHOOL

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TYPICAL DETAILS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S110
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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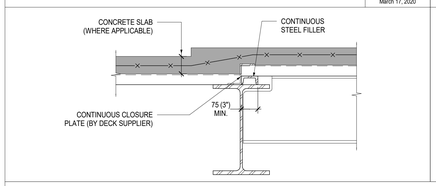
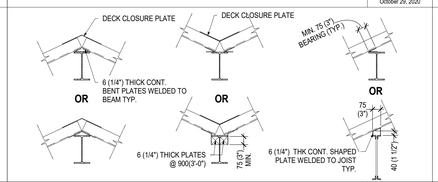
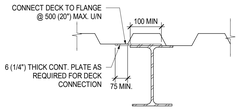
STEEL DECK AT CHANGE IN LEVEL — SD-201
March 17, 2020



SUPPORT OF SLOPED ROOF DECK — SD-202
October 29, 2020



STEEL BEAM PARALLEL OR ANGLED TO STEEL DECK — SD-203
October 29, 2020

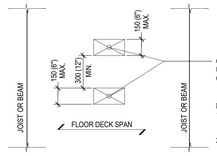


REINFORCEMENT FOR SLAB ON COMPOSITE DECK — SD-204
October 29, 2020

SLAB THICKNESS (t)	LESS THAN 89 (3 1/2")	89 (3 1/2") OR MORE
REINFORCEMENT	W/F 152x152 MW18.7/MW18.7	10@400 (16") E.W.

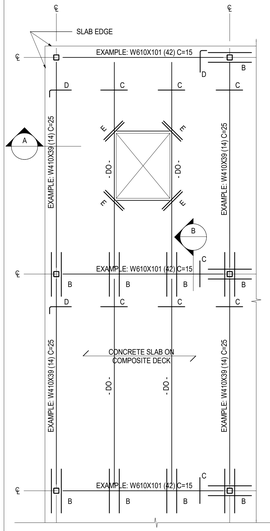
NOTES:
1. LAP WELDED WIRE FABRIC 200 (8").
2. PROVIDE "CLASS "B" LAP FOR ALL REBARS.
3. PROVIDE STANDARD HOOKS ALONG SLAB EDGES.
4. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN WIRE FABRIC OR REBARS 25 (1") BELOW TOP OF SLAB.
5. FOR ADDITIONAL REINFORCEMENT AT COMPOSITE BEAMS, SEE DETAIL FOR TYPICAL COMPOSITE STEEL BEAM CONSTRUCTION.

FLOOR DECK OPENINGS — SD-207
October 29, 2020



NOTES:
1. ALL LARGER OPENINGS AND OPENINGS SPACED CLOSER THAN SPECIFIED IN DIAGRAM TO BE FRAMED BY STRUCTURAL STEEL, SEE DRAWING NOTES.
2. CUT STEEL DECK FOR THE OPENINGS AFTER CONCRETE REACHES MINIMUM 75% OF SPECIFIED CONCRETE STRENGTH

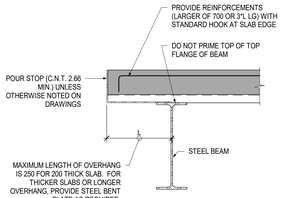
COMPOSITE STEEL BEAM CONSTRUCTION — SD-300
October 29, 2020



FRAMING PLAN

			SLAB THICKNESS (t)	
			LESS THAN 89	89 OR MORE
REINFORCEMENT	A	MINIMUM SLAB REINFORCEMENT	W/F 152x152 MW18.7/MW18.7 TOP	10@400 TOP EA.WAY
	B	ADDED LONGITUDINAL REINFORCEMENT AT ENDS OF COMPOSITE BEAMS	2-15T x 2000 LG.	2-15T x 2000 LG.
	C	ADDED TRANSVERSE REINFORCEMENT OVER COMPOSITE BEAMS	10@500T x1200 LG.	NOT REQUIRED
	D	ADDED TRANSVERSE REINFORCEMENT OVER COMPOSITE BEAMS	15@500 T REFER TO SECTION 'A'	NOT REQUIRED
	E	ADDED REINFORCEMENT AT OPENINGS	2-10x1200 LG. IN MIDDLE OF SLAB	2-10x1200 LG. IN MIDDLE OF SLAB

NOTES:
1. COMPOSITE MEMBERS ARE IDENTIFIED BY "(XX)" FOLLOWING THE STEEL SECTION SIZE. WHERE "(XX)" DENOTES NUMBER OF STUDS REQUIRED FOR MEMBER.
2. "C=YY" IS THE AMOUNT OF POSITIVE CAMBER AT THE MIDSPAN OF THE COMPOSITE BEAM.
3. STUD DIAMETER IS 19 (3/4") UNLESS OTHERWISE SPECIFIED.
4. STUD LENGTH AFTER WELDING SHALL BE THE OVERALL DEPTH OF STEEL DECK PLUS SLAB THICKNESS MINUS 25 (1").
5. SPACING OF STUDS SHALL NOT EXCEED 600 (2'-0") INCREASE THE NUMBER OF STUDS SHOWN ON PLANS AS REQUIRED TO MEET THIS CRITERION.
6. DESIGN CONNECTIONS FOR COMPOSITE BEAMS FOR ONE AND A HALF TIMES THE END REACTION DUE TO THE UDL CAPACITY OF THE STEEL SECTION UNLESS A LARGER REACTION IS GIVEN ON PLAN.
7. LAP WELDED WIRE FABRIC 200 (8"). FOR CONTINUOUS DEFORMED BARS PROVIDE CLASS "B" LAP AND HOOK AT PERIMETER. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN WIRE FABRIC OR REBARS 25 (1") BELOW TOP OF SLAB.



SECTION 'A'

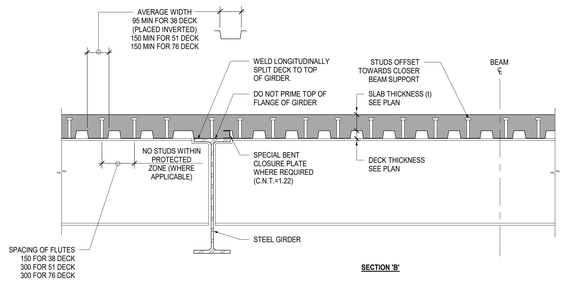
USE THE FOLLOWING CRITERIA TO LAY OUT STUDS:

1. DO NOT WELD ANY STUDS TO BEAMS OR GIRDERS UNTIL ALL STUDS FOR THAT MEMBER HAVE BEEN LAID OUT ON SITE.
2. DO NOT PLACE STUDS ON THE CANTILEVERED PORTION OF A COMPOSITE BEAM UNLESS OTHERWISE NOTED ON THE DRAWINGS.
3. STEEL GIRDERS: LAY OUT SINGLE ROW OF STUDS ON CENTRELINE OF BEAM UNIFORMLY SPACED AT MINIMUM 150 (6") AND MAXIMUM 600 (24"). IF THE NUMBER OF STUDS EXCEEDS THE NUMBER THAT CAN BE ACCOMMODATED WITH MINIMUM SPACING IN ONE ROW, LAY OUT BALANCE IN DOUBLE ROW AS SHOWN IN THE PLAN "D" BELOW.
4. STEEL BEAM: THE STUD CANNOT BE PLACED IN CENTRE OF FLUITE, OFFSET THE STUD TOWARDS THE CLOSER BEAM SUPPORT. THE MINIMUM DISTANCE REQUIRED TO MISS UPSTAND IN FLUTE. SEE SECTION 'B' BELOW

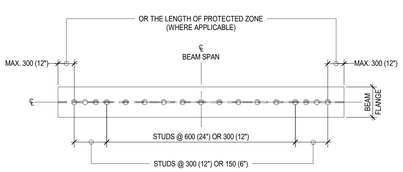
STEP 1: LAY OUT SINGLE ROW OF STUDS ON CENTERLINE OF BEAM UNIFORMLY SPACED @ 600 (2'-0").

STEP 2: LAY OUT THE REMAINING STUDS HALF WAY BETWEEN THE ALREADY PLACED STUDS, STARTING FROM EACH END. REPEAT THE PROCESS AS REQUIRED. SEE PLAN "C" BELOW.

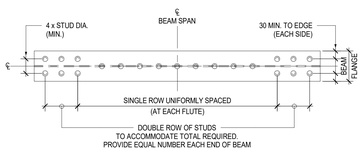
STEP 3: IF THE NUMBER OF STUDS EXCEEDS THE NUMBER THAT CAN BE ACCOMMODATED BY FILLING EACH FLUTE IN ONE ROW, LAY OUT BALANCE IN DOUBLE ROW FROM BOTH ENDS AS SHOWN IN THE PLAN "D" BELOW.



SECTION 'B'

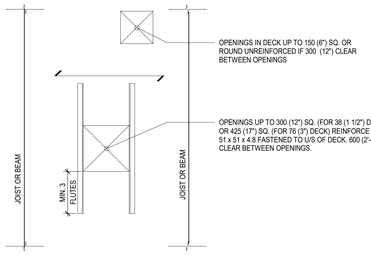


PLAN 'C'



PLAN 'D'

ROOF DECK OPENINGS — SD-206
March 17, 2020



- ALL LARGER OPENINGS AND OPENINGS SPACED CLOSER THAN SPECIFIED ABOVE TO BE FRAMED BY STRUCTURAL STEEL, SEE DRAWING NOTES.

Key to Detail Location

NO. — Detail Number
NO. — Drawing Number

If this sheet is not 46 7/8" x 33 1/8" (1189 x 841 mm) It is a reduced print - Read dwg. accordingly.

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No	Date	Revision/Issued
1	2025-12-15	ISSUED FOR 30% CONSTRUCTION DOCUMENTS
2	2026-03-13	ISSUED FOR BUILDING PERMIT
3	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

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TYPICAL DETAILS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S111
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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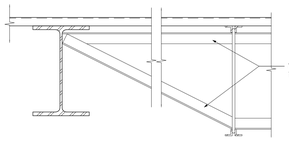
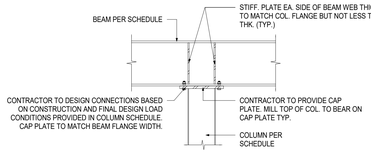
ANCHORAGE OF BRIDGING TO STEEL

SD-309
March 17, 2020



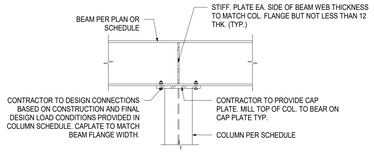
TYPICAL STEEL BEAM BEARING ON COLUMN (PARALLEL WEB)

SD-311
October 29, 2020



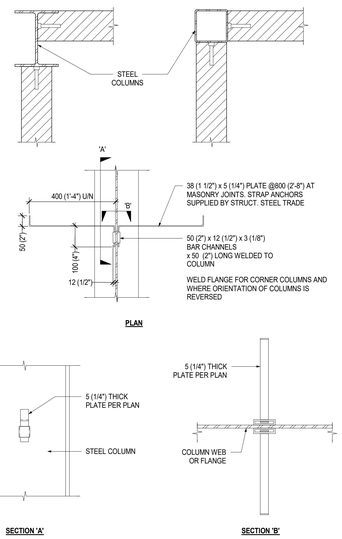
TYPICAL STEEL BEAM BEARING ON COLUMN (PERPENDICULAR WEB)

SD-312
October 29, 2020



ANCHORAGE OF ABUTTING LOAD BEARING MASONRY WALLS TO STEEL COLUMNS

SD-400
October 29, 2020



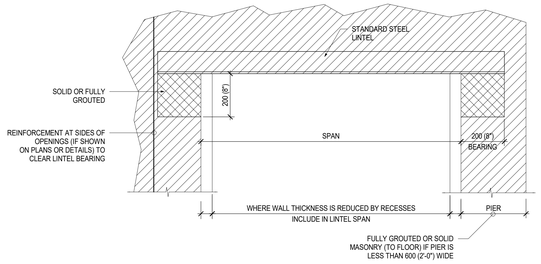
** NOTE FOR ENGINEERS:
USE ONLY FOR MASONRY SHEAR WALL BUILDING

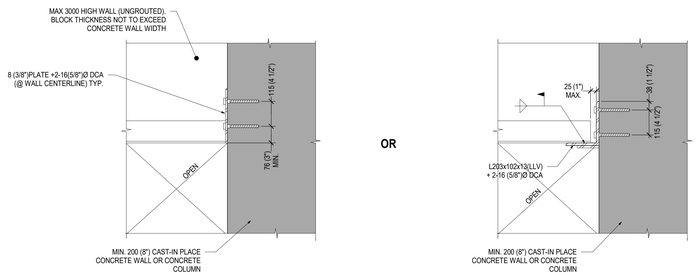
TYPICAL STEEL LINTELS IN NON LOAD BEARING MASONRY WALLS

SD-310
March 17, 2020

NOTES:

1. CONNECT BACK TO BACK DOUBLE ANGLES USING MIN. 16 (5/8")Ø BOLTS @ 450 (1'-6") MAX., OR BY WELDING @ TOP & BOTTOM USING 6mm (1/4") WELDS x 50 (2") LG. @ 450 (1'-6") MAX. LOCATE FIRST CONNECTION MAX. 75 (3") FROM END OF LINTEL.
2. PROVIDE STEEL PACKING AS REQUIRED TO ENSURE EVEN BEARING OF STEEL LINTELS.
3. STANDARD LINTELS ARE NOT NECESSARILY SHOWN ON STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR OPENING LOCATIONS.
4. SEE PLANS FOR SPECIAL LINTELS.



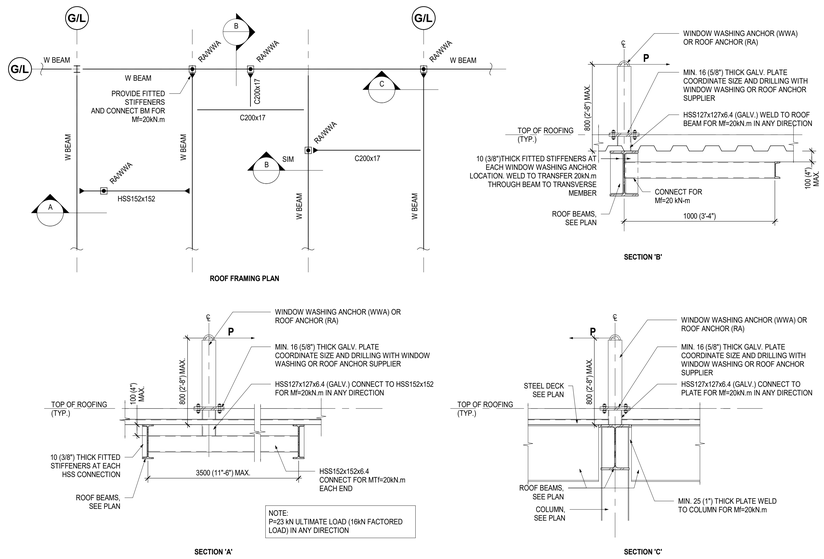


LINTEL SUPPORTED BY POURED CONCRETE WALL

WALL THICKNESS	SPAN	ANGLE SIZE	DETAIL
90 (4") VENEER	UP TO 1200 (4'-0")	L89x89x6.4	89
	1200 (4'-0") TO 1800 (6'-0")	L127x89x6.4	
	1800 (6'-0") TO 2400 (8'-0")	L127x89x6.4	
	2400 (8'-0") TO 3000 (10'-0")	L152x89x9.5	
140 (6")	UP TO 1200 (4'-0")	2- L64x64x6.4	64 64 100x10 (4"x3/8") STEEL PLATE
	1200 (4'-0") TO 1800 (6'-0")	2 - L89x64x6.4	
	1800 (6'-0") TO 2400 (8'-0")	2 - L89x64x7.9 W/ 100x10 PLATE	
	2400 (8'-0") TO 3000 (10'-0")	N/A	
190 (8")	UP TO 1200 (4'-0")	2 - L76x89x6.4	89 89
	1200 (4'-0") TO 1800 (6'-0")	2 - L89x89x6.4	
	1800 (6'-0") TO 2400 (8'-0")	2 - L102x89x7.9	
	2400 (8'-0") TO 3000 (10'-0")	2 - L152x89x7.9	
240 (10")	UP TO 1200 (4'-0")	2 - L76x102x6.4	102 102
	1200 (4'-0") TO 1800 (6'-0")	2 - L102x102x6.4	
	1800 (6'-0") TO 2400 (8'-0")	2 - L102x102x9.5	
	2400 (8'-0") TO 3000 (10'-0")	2 - L152x102x7.9	
290 (12")	UP TO 1200 (4'-0")	3 - L76x89x6.4	89 89 89
	1200 (4'-0") TO 1800 (6'-0")	3 - L89x89x6.4	
	1800 (6'-0") TO 2400 (8'-0")	3 - L102x89x7.9	
	2400 (8'-0") TO 3000 (10'-0")	3 - L127x89x7.9	

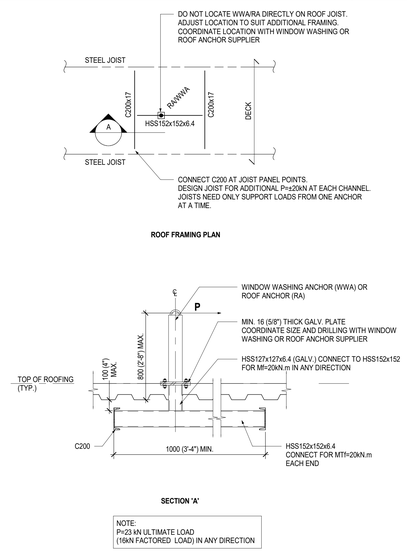
RA / WWA ON ROOFS FRAMED WITH STEEL BEAMS

SD-900
October 29, 2020



RA / WWA ON ROOFS FRAMED WITH STEEL JOISTS

SD-901
October 29, 2020



NOTE:
P=±23 kN ULTIMATE LOAD
(16kN FACTORED LOAD) IN ANY DIRECTION

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4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

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TYPICAL DETAILS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S112
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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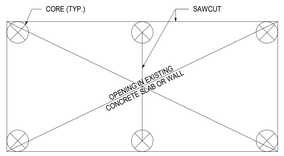
SAW CUTTING OF EXISTING SLAB OR WALL

ED-100
March 17, 2020



SAW CUTTING PROCEDURE:

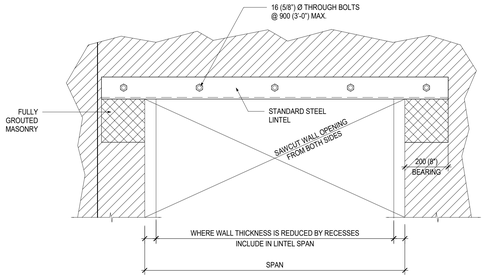
1. OBTAIN ENGINEER'S APPROVAL BEFORE CUTTING ANY OPENINGS. IF REQUESTED, LOCATE REINFORCEMENT AND OTHER NON-STRUCTURAL EMBEDDED ITEMS (SUCH AS ELECTRICAL CONDUITS) USING A NON-DESTRUCTIVE METHOD.
2. CORE HOLES AT EACH CORNER AND AT ENDS OF SAWCUTS.
3. SAWCUT AND DO NOT OVERCUT.
4. CHIP CORNERS SQUARE.

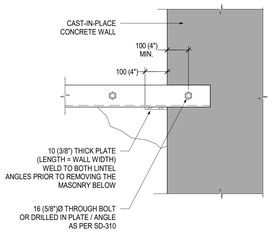
TYPICAL STEEL LINTELS IN EXISTING NON-LOAD BEARING MASONRY WALL

ED-200
October 29, 2020

NOTES:

1. PROVIDE SHORING AND BRACING AS REQUIRED.
2. GROUT MASONRY AT SUPPORTS PRIOR TO SAWCUTTING.
3. PROVIDE STEEL PACKING AS REQUIRED TO PROVIDE EVEN BEARING OF STEEL LINTELS.
4. STANDARD LINTELS ARE NOT NECESSARILY SHOWN ON STRUCTURAL DRAWINGS, REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR OPENING LOCATIONS.
5. SEE PLANS FOR SPECIAL LINTELS.



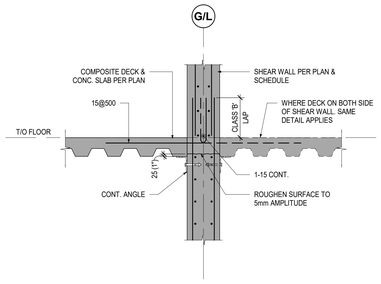


LINTEL SUPPORTED BY POURED CONCRETE WALL

WALL THICKNESS	SPAN	ANGLE SIZE	DETAIL
140 (6")	UP TO 1200 (4'-0")	2- L64x64x6.4	64 64
	1200 (4'-0") TO 1800 (6'-0")	2 - L89x64x6.4	
	1800 (6'-0") TO 2400 (8'-0")	N/A	
	2400 (8'-0") TO 3000 (10'-0")	N/A	
190 (8")	UP TO 1200 (4'-0")	2 - L76x89x6.4	89 89
	1200 (4'-0") TO 1800 (6'-0")	2 - L89x89x6.4	
	1800 (6'-0") TO 2400 (8'-0")	2 - L102x89x7.9	
	2400 (8'-0") TO 3000 (10'-0")	2 - L152x89x7.9	
240 (10")	UP TO 1200 (4'-0")	2 - L76x102x6.4	102 102
	1200 (4'-0") TO 1800 (6'-0")	2 - L102x102x6.4	
	1800 (6'-0") TO 2400 (8'-0")	2 - L102x102x9.5	
	2400 (8'-0") TO 3000 (10'-0")	2 - L152x102x7.9	
290 (12")	UP TO 1200 (4'-0")	2 - L89x152x7.9	152 152
	1200 (4'-0") TO 1800 (6'-0")	2 - L102x152x7.9	
	1800 (6'-0") TO 2400 (8'-0")	2 - L102x152x9.5	
	2400 (8'-0") TO 3000 (10'-0")	2 - L152x152x9.5	

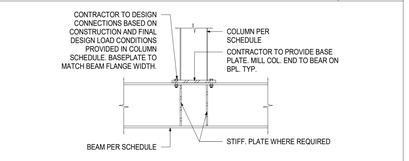
TYPICAL COMPOSITE STEEL DECK SUPPORT DETAIL AT SHEAR WALLS

PD-CS-001
October 29, 2020

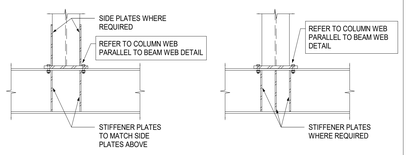


TYPICAL COLUMN BEARING ON STEEL BEAM

PD-S-004
October 29, 2020



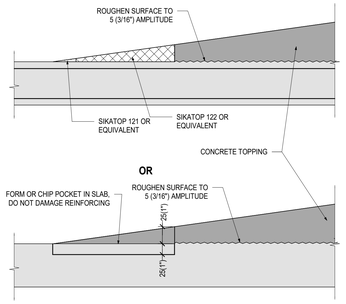
A) COLUMN WEB PARALLEL TO BEAM WEB



B) COLUMN WEB PERPENDICULAR TO BEAM WEB

EDGE OF BONDED CONCRETE TOPPING

PD-C-002
October 21, 2020

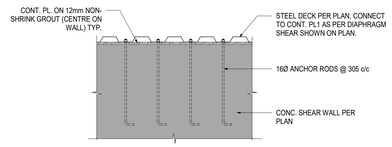


NOTES:

1. THIS DETAIL ALSO APPLICABLE FOR LEVELING OF EXISTING SLAB DEPRESSIONS, AND THIN BUILT-UP SLABS.
2. OPTION FOR NEW SLABS: POUR TOPPING MONOLITHICALLY WITH SLAB

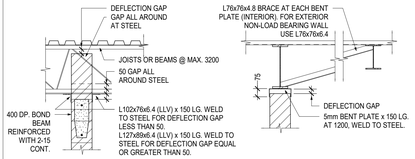
STEEL DECK AT TOP OF WALL

PD-CS-002
October 29, 2020



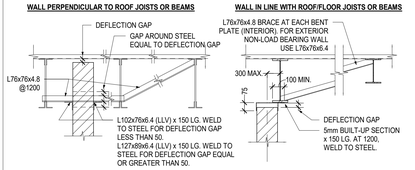
LATERAL SUPPORT OF NON-BEARING INTERIOR MASONRY PARTITIONS

PD-M-001
April 29, 2021



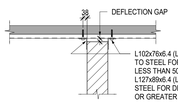
WALL PERPENDICULAR TO ROOF JOISTS OR BEAMS

WALL IN LINE WITH ROOF/FLOOR JOISTS OR BEAMS

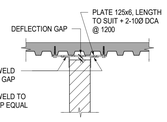


WALL PARALLEL TO ROOF JOISTS OR BEAMS

WALL OFFSET FROM ROOF/FLOOR JOISTS OR BEAMS



WALL PERPENDICULAR TO FLOOR DECK FLUTES

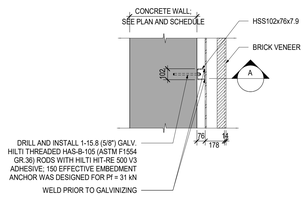


WALL PARALLEL TO FLOOR DECK FLUTES

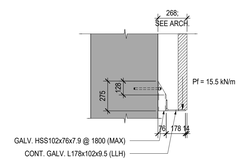
1. DCA TO BE HILTI KB-3 WITH 64 EMBEDMENT.
2. PROVIDE 30mm DEFLECTION GAP TYPICAL U/N.
3. MINIMUM WALL THICKNESS IS 140 FOR INTERIOR WALLS AND 190 FOR EXTERIOR WALLS. WALL HEIGHT SHALL NOT EXCEED 36 TIMES WALL THICKNESS FOR INTERIOR WALLS AND 30 TIMES WALL THICKNESS FOR EXTERIOR WALLS. REFER TO DETAIL FOR EXTERIOR NON-LOAD BEARING WALL REINFORCEMENT, TYPICAL U/N.
4. FOR EXTERIOR WALL LATERAL SUPPORT INCREASE 150 LG. ANGLE/BENT PLATE SHOWN IN THE DETAILS TO 250 LG. AND REPLACE 2-10Ø DCA WITH 2-12Ø DCA + INCREASE ALL BENT PLATES TO 6mm THICK.
5. FOR CONCEALED LATERAL SUPPORT REFER TO ARCH SPEC. AND DRAWINGS.

BRICK SHELF ANGLE ASSEMBLY DETAIL AT CONCRETE WALL

PD-001
May 11, 2026



PLAN VIEW



SECTION 'A'

NOTES:

1. ALL STEEL SHALL BE GALVANIZED
2. ALL FASTENERS SHALL BE GALVANIZED
3. ELIMINATE ALL FIELD WELDING. USE ONLY FIELD BOLTED CONNECTIONS.
4. JOINTS IN ANGLES SHALL MATCH MASONRY CONTROL JOINTS.
5. DECREASE SPACING OF HSS SPACERS AND ANCHORS TO SUIT CONTROL JOINTS TURNS AND DISCONTINUED ENDS.

Key to Detail Location

NO. — Detail Number
NO. — Drawing Number

If this sheet is not 46 7/8" x 33 1/8" (1189 x 841 mm) It is a reduced print - Read dwg. accordingly.

Contractors must check and verify all dimensions on the job and report any discrepancies to the Architect before proceeding with the work.

All prints and specifications are the property of the Architect and must be returned at the completion of the work.

Drawings should not be scaled.

No	Date	Revision/Issued
1	2025-12-15	ISSUED FOR 30% CONSTRUCTION DOCUMENTS
2	2026-03-13	ISSUED FOR BUILDING PERMIT
3	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

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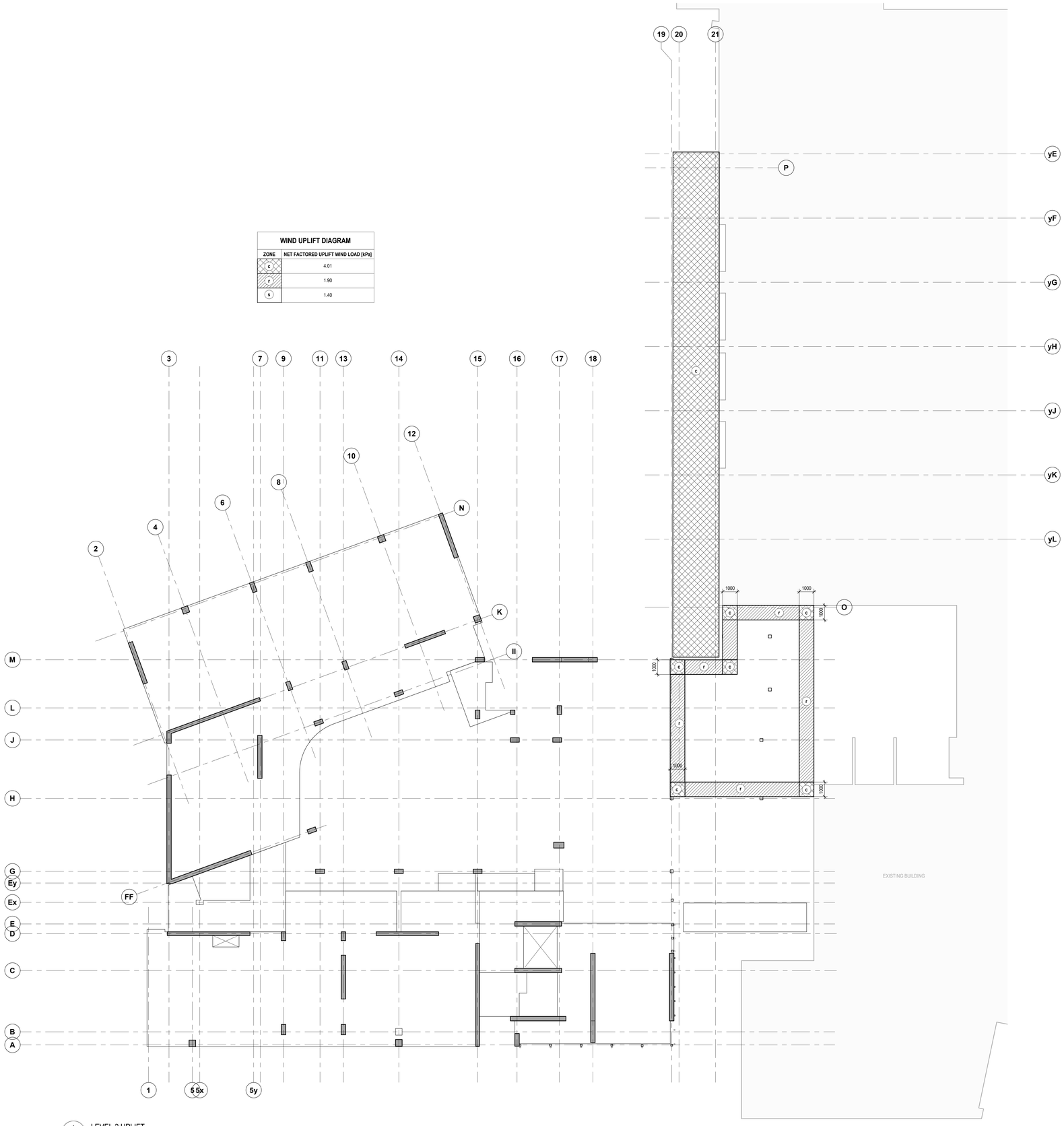
TYPICAL DETAILS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S113
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-03 3:04:58 PM

WIND UPLIFT DIAGRAM	
ZONE	NET FACTORED UPLIFT WIND LOAD [kPa]
c	4.01
r	1.90
e	1.40



1 / S120 LEVEL 2 UPLIFT
1 : 100

Key to Detail Location

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NO. — Drawing Number

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No	Date	Revision/Issued
1	2026-03-13	ISSUED FOR BUILDING PERMIT
2	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
3	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
4	2026-07-08	ISSUED FOR TENDER

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MEZZANINE AND SECOND FLOOR ROOF UPLIFT DIAGRAM

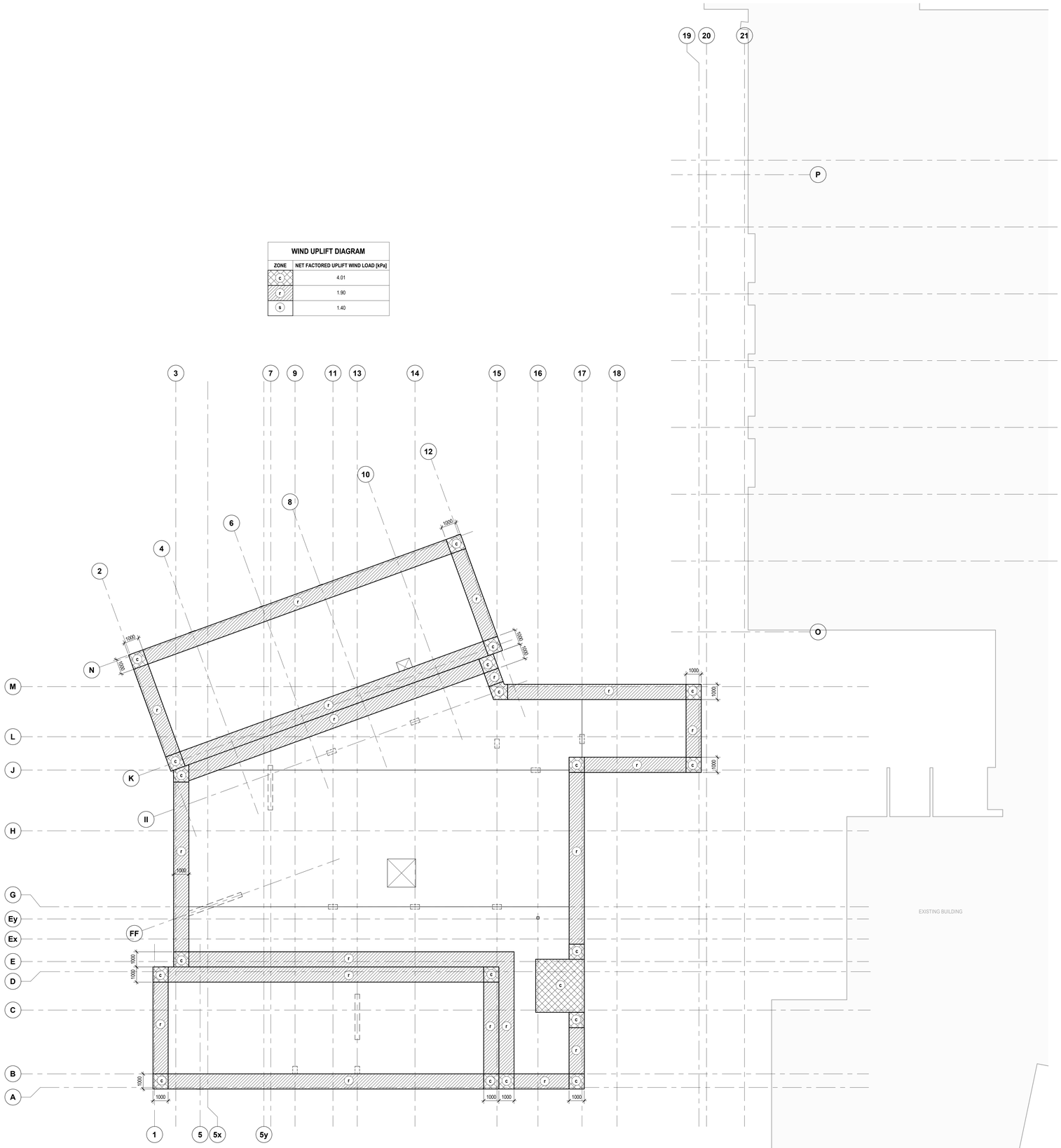
scale:	As indicated
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S120
	Revision: 4

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-03 3:04:58 PM

WIND UPLIFT DIAGRAM

ZONE	NET FACTORED UPLIFT WIND LOAD [kPa]
c	4.01
r	1.90
s	1.40



1 S121 ROOF UPLIFT DIAGRAM
1 : 100

Key to Detail Location

NO. — Detail Number
NO. — Drawing Number

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No	Date	Revision/Issued
1	2026-03-13	ISSUED FOR BUILDING PERMIT
2	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
3	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
4	2026-07-08	ISSUED FOR TENDER

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ROOF UPLIFT DIAGRAM

scale:	As indicated
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S121
	Revision: 4

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

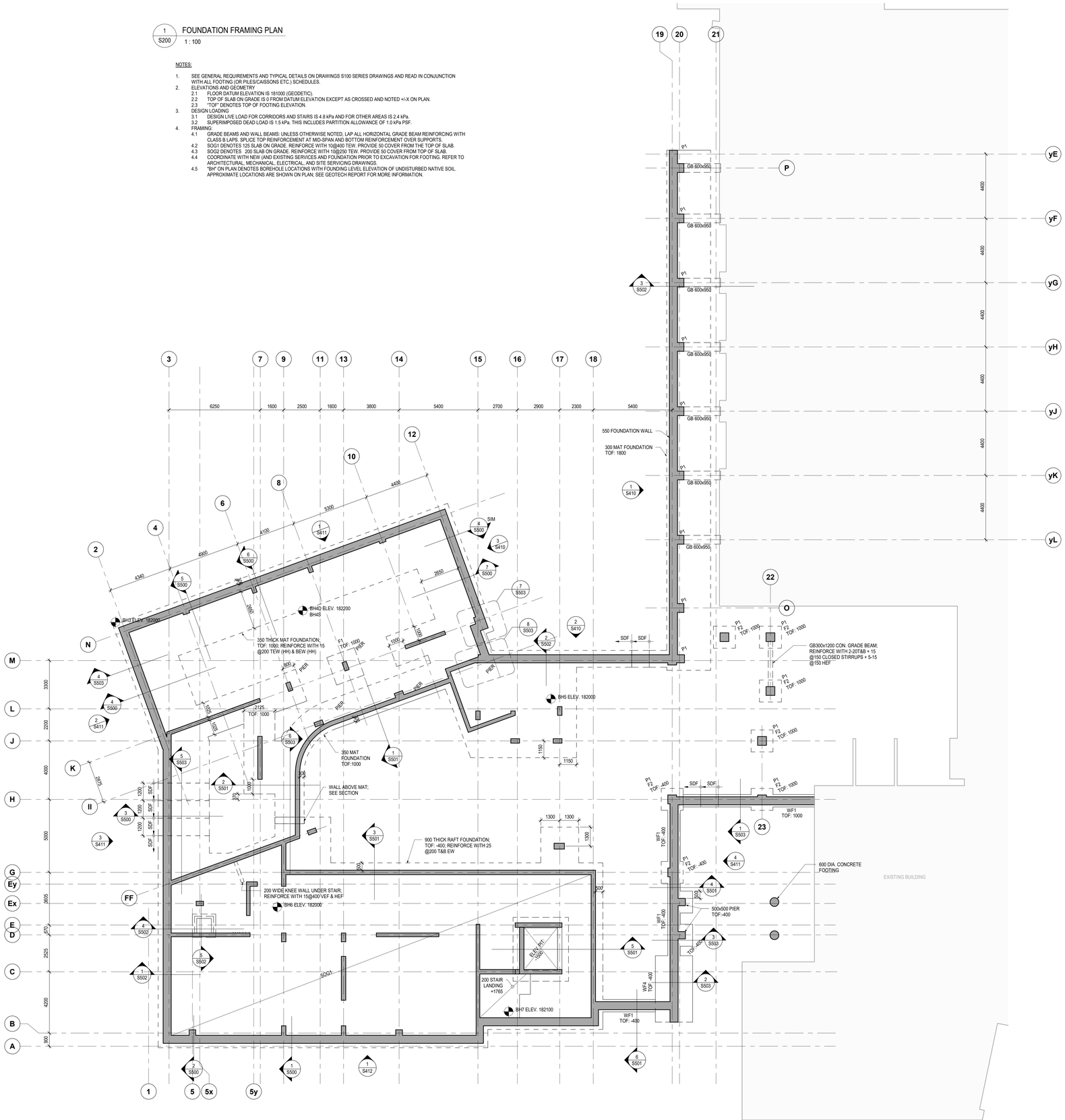
DRAWING LAST EDITED: 2026-07-03 3:04:59 PM

1 / S200 FOUNDATION FRAMING PLAN

1 : 100

NOTES:

1. SEE GENERAL REQUIREMENTS AND TYPICAL DETAILS ON DRAWINGS S100 SERIES DRAWINGS AND READ IN CONJUNCTION WITH ALL FOOTING (OR PILES/CAISSONS ETC.) SCHEDULES.
2. ELEVATIONS AND GEOMETRY
   - 2.1 FLOOR DATUM ELEVATION IS 181000 (GEODETIC).
   - 2.2 TOP OF SLAB ON GRADE IS 0 FROM DATUM ELEVATION EXCEPT AS CROSSED AND NOTED +/-X ON PLAN.
   - 2.3 "TOF" DENOTES TOP OF FOOTING ELEVATION.
3. DESIGN LOADING
   - 3.1 DESIGN LIVE LOAD FOR CORRIDORS AND STAIRS IS 4.8 kPa AND FOR OTHER AREAS IS 2.4 kPa.
   - 3.2 SUPERIMPOSED DEAD LOAD IS 1.5 kPa. THIS INCLUDES PARTITION ALLOWANCE OF 1.0 kPa PSF.
4. FRAMING:
   - 4.1 GRADE BEAMS AND WALL BEAMS: UNLESS OTHERWISE NOTED, LAP ALL HORIZONTAL GRADE BEAM REINFORCING WITH CLASS B LAPS. SPLICE TOP REINFORCEMENT AT MID-SPAN AND BOTTOM REINFORCEMENT OVER SUPPORTS.
   - 4.2 SOG1 DENOTES 125 SLAB ON GRADE. REINFORCE WITH 10@400 TEW. PROVIDE 50 COVER FROM THE TOP OF SLAB.
   - 4.3 SOG2 DENOTES 200 SLAB ON GRADE. REINFORCE WITH 10@250 TEW. PROVIDE 50 COVER FROM TOP OF SLAB.
   - 4.4 COORDINATE WITH NEW (AND EXISTING SERVICES AND FOUNDATION PRIOR TO EXCAVATION FOR FOOTING. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND SITE SERVICING DRAWINGS.
   - 4.5 "BH" ON PLAN DENOTES BOREHOLE LOCATIONS WITH FOUNDING LEVEL ELEVATION OF UNDISTURBED NATIVE SOIL. APPROXIMATE LOCATIONS ARE SHOWN ON PLAN; SEE GEOTECH REPORT FOR MORE INFORMATION.



CONCRETE PIER SCHEDULE

MARK	SIZE (mm)	REINFORCING		REMARKS
		VERTICAL	TIES	
P1	600x600	8-20V	10@300	

SPREAD FOOTING SCHEDULE

MARK	SIZE (mm)	REINFORCING	BEARING CAPACITY (kN)		REMARKS
			SLS	ULS	
F1	2000x2000x350	9-20 BEW (HH)	550	700	
F2	1500x1500x300	7-15 BEW (HH)	300	400	

STRIP FOOTING SCHEDULE

MARK	SIZE (mm)		REINFORCING	REMARKS
	WIDTH	DEPTH		
WF1	850	300	SEE SECTION	
WF4	2550	350	SEE SECTION	

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4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

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PROJECT NORTH

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THE STERLING HALL SCHOOL

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FOUNDATION AND LOWER LEVEL FRAMING PLAN

scale:	As indicated
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S200
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

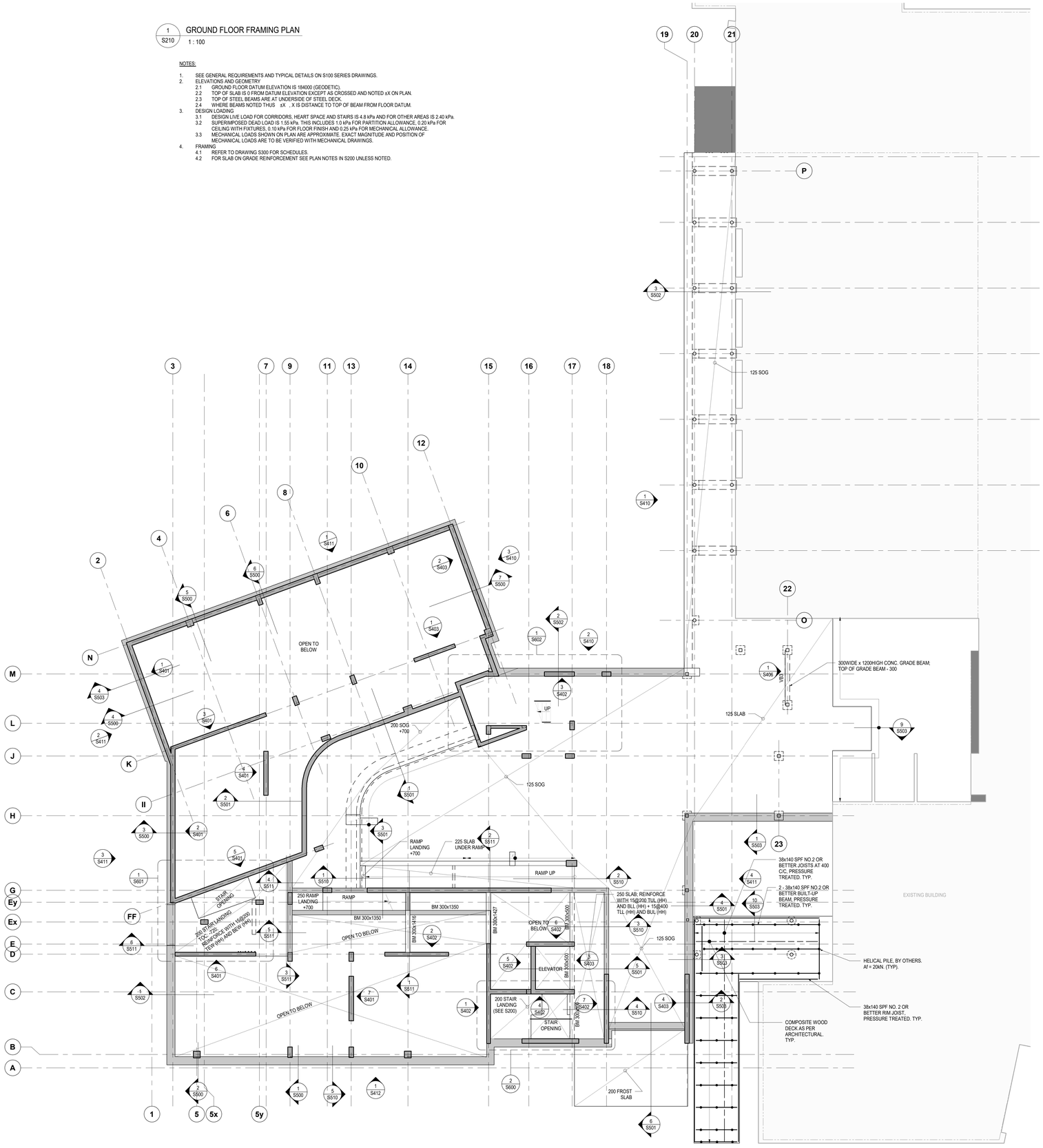
DRAWING LAST EDITED: 2026-07-03 3:05:00 PM

1 / S210 GROUND FLOOR FRAMING PLAN

1 : 100

NOTES:

1. SEE GENERAL REQUIREMENTS AND TYPICAL DETAILS ON S100 SERIES DRAWINGS.
2. ELEVATIONS AND GEOMETRY
   - 2.1 GROUND FLOOR DATUM ELEVATION IS 184000 (GEODETIC).
   - 2.2 TOP OF SLAB IS 0 FROM DATUM ELEVATION EXCEPT AS CROSSED AND NOTED ±X ON PLAN.
   - 2.3 TOP OF STEEL BEAMS ARE AT UNDERSIDE OF STEEL DECK.
   - 2.4 WHERE BEAMS NOTED THUS ±X , X IS DISTANCE TO TOP OF BEAM FROM FLOOR DATUM.
3. DESIGN LOADING
   - 3.1 DESIGN LIVE LOAD FOR CORRIDORS, HEART SPACE AND STAIRS IS 4.8 kPa AND FOR OTHER AREAS IS 2.40 kPa.
   - 3.2 SUPERIMPOSED DEAD LOAD IS 1.55 kPa. THIS INCLUDES 1.0 kPa FOR PARTITION ALLOWANCE, 0.20 kPa FOR CEILING WITH FIXTURES, 0.10 kPa FOR FLOOR FINISH AND 0.25 kPa FOR MECHANICAL ALLOWANCE.
   - 3.3 MECHANICAL LOADS SHOWN ON PLAN ARE APPROXIMATE. EXACT MAGNITUDE AND POSITION OF MECHANICAL LOADS ARE TO BE VERIFIED WITH MECHANICAL DRAWINGS.
4. FRAMING
   - 4.1 REFER TO DRAWING S300 FOR SCHEDULES.
   - 4.2 FOR SLAB ON GRADE REINFORCEMENT SEE PLAN NOTES IN S200 UNLESS NOTED.



Key to Detail Location

NO. / NO. — Detail Number / Drawing Number

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4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

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THE STERLING HALL SCHOOL

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GROUND FLOOR FRAMING PLAN

scale:	As indicated
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S210
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

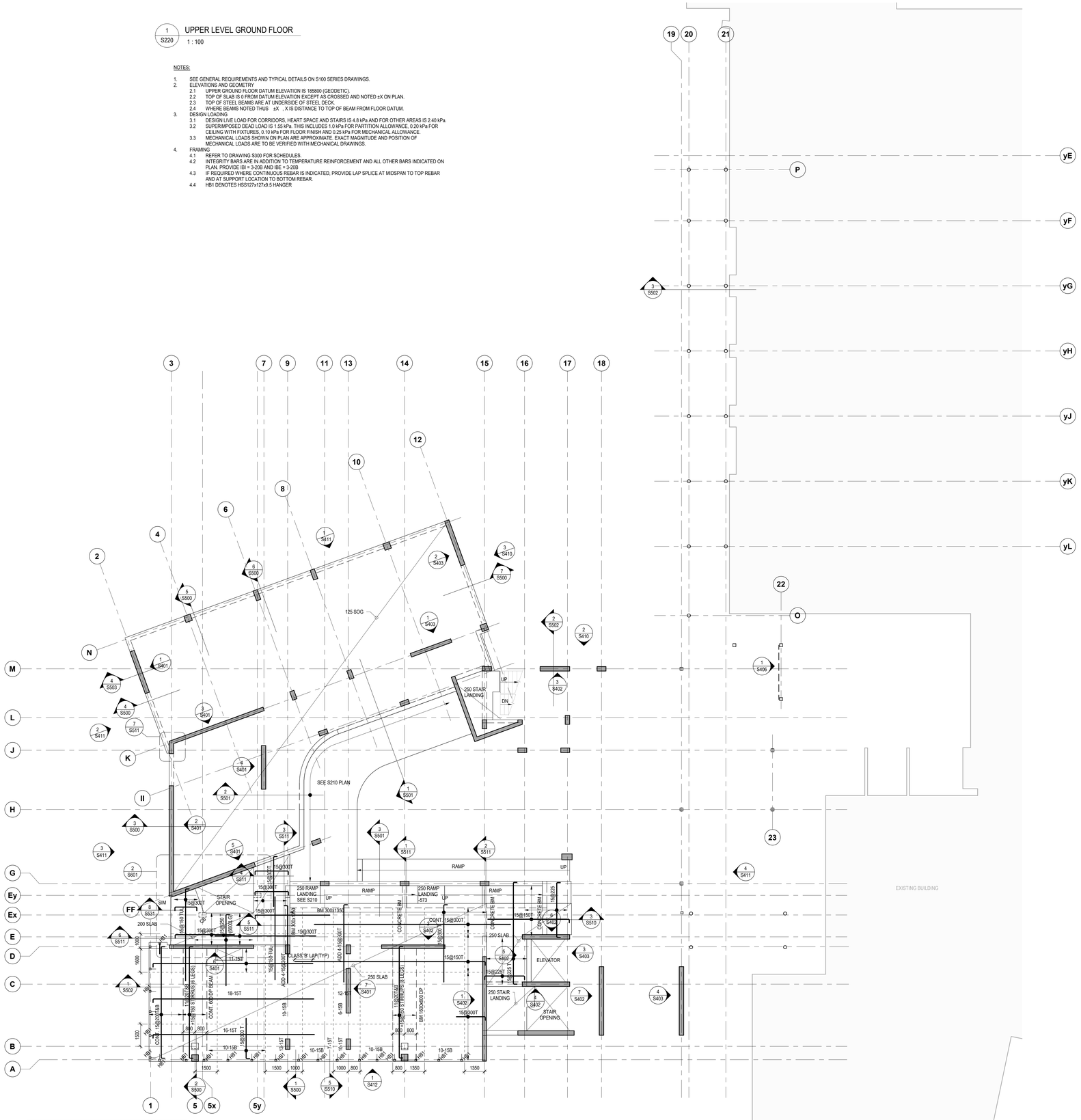
DRAWING LAST EDITED: 2026-07-03 3:05:02 PM

1 / S220 UPPER LEVEL GROUND FLOOR

1 : 100

NOTES:

1. SEE GENERAL REQUIREMENTS AND TYPICAL DETAILS ON S100 SERIES DRAWINGS.
2. ELEVATIONS AND GEOMETRY
   - 2.1 UPPER GROUND FLOOR DATUM ELEVATION IS 185800 (GEODETIC).
   - 2.2 TOP OF SLAB IS 0 FROM DATUM ELEVATION EXCEPT AS CROSSED AND NOTED ±X ON PLAN.
   - 2.3 TOP OF STEEL BEAMS ARE AT UNDERSIDE OF STEEL DECK.
   - 2.4 WHERE BEAMS NOTED THUS ±X , X IS DISTANCE TO TOP OF BEAM FROM FLOOR DATUM.
3. DESIGN LOADING
   - 3.1 DESIGN LIVE LOAD FOR CORRIDORS, HEART SPACE AND STAIRS IS 4.8 kPa AND FOR OTHER AREAS IS 2.40 kPa.
   - 3.2 SUPERIMPOSED DEAD LOAD IS 1.55 kPa. THIS INCLUDES 1.0 kPa FOR PARTITION ALLOWANCE, 0.20 kPa FOR CEILING WITH FIXTURES, 0.10 kPa FOR FLOOR FINISH AND 0.25 kPa FOR MECHANICAL ALLOWANCE.
   - 3.3 MECHANICAL LOADS SHOWN ON PLAN ARE APPROXIMATE. EXACT MAGNITUDE AND POSITION OF MECHANICAL LOADS ARE TO BE VERIFIED WITH MECHANICAL DRAWINGS.
4. FRAMING
   - 4.1 REFER TO DRAWING S300 FOR SCHEDULES.
   - 4.2 INTEGRITY BARS ARE IN ADDITION TO TEMPERATURE REINFORCEMENT AND ALL OTHER BARS INDICATED ON PLAN. PROVIDE IBl = 3-20B AND IBE = 3-20B
   - 4.3 IF REQUIRED WHERE CONTINUOUS REBAR IS INDICATED, PROVIDE LAP SPLICE AT MIDSPAN TO TOP REBAR AND AT SUPPORT LOCATION TO BOTTOM REBAR.
   - 4.4 HB1 DENOTES HSS127x127x9.5 HANGER



200 CONCRETE SLAB REINFORCE WITH 15@300 TEW AND 15@150 BEW,TYP. U/N	250 CONCRETE SLAB REINFORCE WITH 15@400BEW TYP. U/N
TLL BLL / TLL BUL	TLL BLL / TLL BUL

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-03 3:05:03 PM

Key to Detail Location

NO. / NO. — Detail Number / Drawing Number

If this sheet is not 46 7/8" x 33 1/8" (1189 x 841 mm) It is a reduced print - Read dwg. accordingly.

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Drawings should not be scaled.

No	Date	Revision/Issued
1	2025-12-15	ISSUED FOR 30% CONSTRUCTION DOCUMENTS
2	2026-03-13	ISSUED FOR BUILDING PERMIT
3	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

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structural engineer — LEA Consulting Ltd. 40 University Ave, Suite 503 Toronto, ON, M5J 1T1 Phone.905.470.0015

mechanical engineer — Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151

electrical engineer — Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151

landscape architect — Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

PROJECT NORTH

REGISTERED PROFESSIONAL ENGINEER 2026-07-08 E.J.C. HOMBURY 100072522 PROVINCE OF ONTARIO

LEA Consulting Ltd. 40 University Avenue, Suite 503 Toronto, Ontario M5J 1T1 Tel: 905 470 0015 Fax: 905 470 0030 WWW.LEA.CA

THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

UPPER GROUND FLOOR FRAMING PLAN

scale:	As indicated
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S220

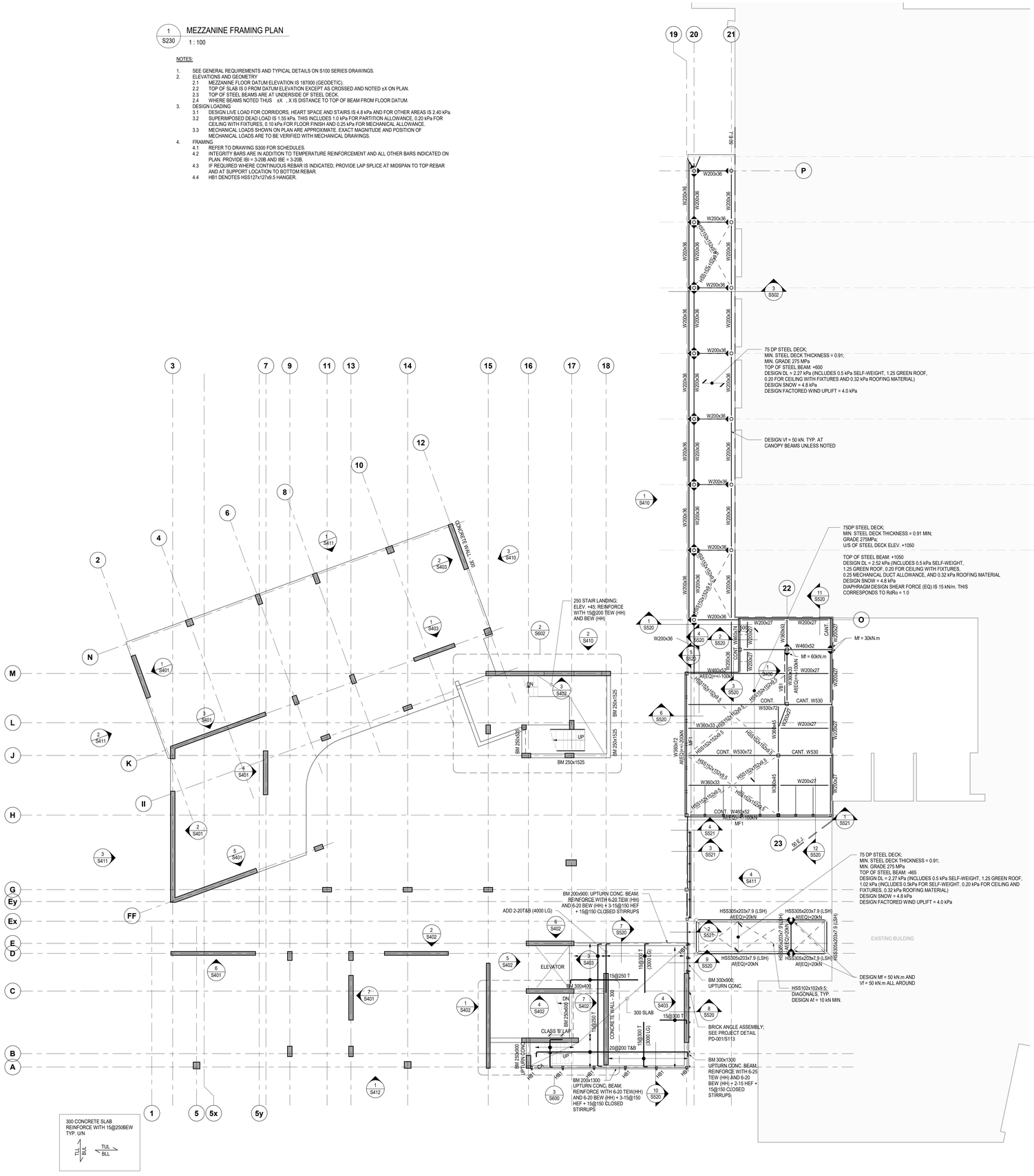
Revision: 5

1 / S230 MEZZANINE FRAMING PLAN

1 : 100

NOTES:

1. SEE GENERAL REQUIREMENTS AND TYPICAL DETAILS ON S100 SERIES DRAWINGS.
2. ELEVATIONS AND GEOMETRY
   - 2.1 MEZZANINE FLOOR DATUM ELEVATION IS 187000 (GEODETIC).
   - 2.2 TOP OF SLAB IS 0 FROM DATUM ELEVATION EXCEPT AS CROSSED AND NOTED ±X ON PLAN.
   - 2.3 TOP OF STEEL BEAMS ARE AT UNDERSIDE OF STEEL DECK.
   - 2.4 WHERE BEAMS NOTED THUS ±X , X IS DISTANCE TO TOP OF BEAM FROM FLOOR DATUM.
3. DESIGN LOADING
   - 3.1 DESIGN LIVE LOAD FOR CORRIDORS, HEART SPACE AND STAIRS IS 4.8 kPa AND FOR OTHER AREAS IS 2.40 kPa.
   - 3.2 SUPERIMPOSED DEAD LOAD IS 1.55 kPa. THIS INCLUDES 1.0 kPa FOR PARTITION ALLOWANCE, 0.20 kPa FOR CEILING WITH FIXTURES, 0.10 kPa FOR FLOOR FINISH AND 0.25 kPa FOR MECHANICAL ALLOWANCE.
   - 3.3 MECHANICAL LOADS SHOWN ON PLAN ARE APPROXIMATE. EXACT MAGNITUDE AND POSITION OF MECHANICAL LOADS ARE TO BE VERIFIED WITH MECHANICAL DRAWINGS.
4. FRAMING
   - 4.1 REFER TO DRAWING S300 FOR SCHEDULES.
   - 4.2 INTEGRITY BARS ARE IN ADDITION TO TEMPERATURE REINFORCEMENT AND ALL OTHER BARS INDICATED ON PLAN. PROVIDE IBI = 3-20B AND IBE = 3-20B.
   - 4.3 IF REQUIRED WHERE CONTINUOUS REBAR IS INDICATED, PROVIDE LAP SPLICE AT MIDSPAN TO TOP REBAR AND AT SUPPORT LOCATION TO BOTTOM REBAR.
   - 4.4 HB1 DENOTES HSS127x127x9.5 HANGER.



Key to Detail Location

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1	2025-12-15	ISSUED FOR 30% CONSTRUCTION DOCUMENTS
2	2026-03-13	ISSUED FOR BUILDING PERMIT
3	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

consultants

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mechanical engineer — Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151

electrical engineer — Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151

landscape architect — Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

PROJECT NORTH

REGISTERED PROFESSIONAL ENGINEER 2026-07-08 E.U.L. HOMBURG 100072522 PROVINCE OF ONTARIO

LEA Consulting Ltd. 40 University Avenue, Suite 503 Toronto, Ontario M5J 1T1 Tel: 905 470 0015 Fax: 905 470 0030 WWW.LEA.CA

THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

MEZZANINE LEVEL FRAMING PLAN

scale:	As indicated
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S230
	Revision: 5

A0

FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

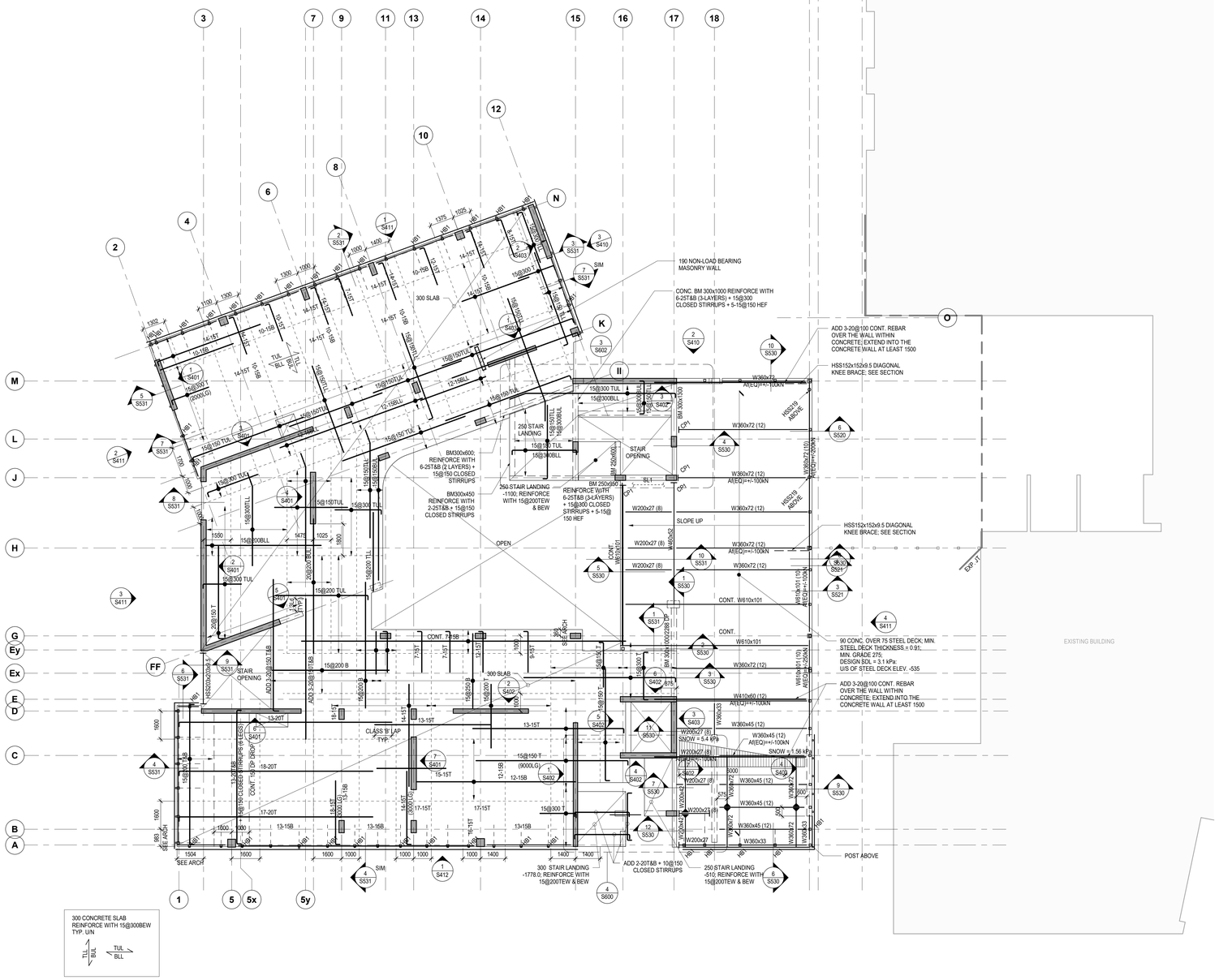
DRAWING LAST EDITED: 2026-07-03 3:05:04 PM

1 / S240 LEVEL 2 FRAMING PLAN

1 : 100

NOTES:

1. SEE GENERAL REQUIREMENTS AND TYPICAL DETAILS ON S100 SERIES DRAWINGS.
2. ELEVATIONS AND GEOMETRY
   - 2.1 SECOND FLOOR DATUM ELEVATION IS 190600 (GEODETIC).
   - 2.2 TOP OF SLAB IS 0 FROM DATUM ELEVATION EXCEPT AS CROSSED AND NOTED ±X ON PLAN.
   - 2.3 TOP OF STEEL BEAMS ARE AT UNDERSIDE OF STEEL DECK
   - 2.4 WHERE BEAMS NOTED THUS ±X ...X IS DISTANCE TO TOP OF BEAM FROM FLOOR DATUM.
3. DESIGN LOADING
   - 3.1 DESIGN LIVE LOAD FOR CORRIDORS, HEART SPACE AND STAIRS IS 4.8 kPa AND FOR OTHER AREAS IS 2.40 kPa.
   - 3.2 SUPERIMPOSED DEAD LOAD IS 1.55 kPa. THIS INCLUDES 1.0 kPa FOR PARTITION ALLOWANCE, 0.20 kPa FOR CEILING WITH FIXTURES, 0.10 kPa FOR FLOOR FINISH AND 0.25 kPa FOR MECHANICAL ALLOWANCE.
   - 3.3 MECHANICAL LOADS SHOWN ON PLAN ARE APPROXIMATE. EXACT MAGNITUDE AND POSITION OF MECHANICAL LOADS ARE TO BE VERIFIED WITH MECHANICAL DRAWINGS.
4. FRAMING
   - 4.1 REFER TO DRAWING S300 FOR SCHEDULES.
   - 4.2 INTEGRITY BARS ARE IN ADDITION TO TEMPERATURE REINFORCEMENT AND ALL OTHER BARS INDICATED ON PLAN. PROVIDE IBI = 3-20B AND IBE = 3-20B.
   - 4.3 IF REQUIRED WHERE CONTINUOUS REBAR IS INDICATED, PROVIDE LAP SPLICE AT MIDSPAN TO TOP REBAR AND AT SUPPORT LOCATION TO BOTTOM REBAR.
   - 4.4 HB1 DENOTES HSS127x127x6.5 HANGER
   - 4.5 PROVIDE STEEL ALLOWANCE FOR FOLDING PARTITIONS IN COSTING. SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT.



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2	2026-03-13	ISSUED FOR BUILDING PERMIT
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4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

consultants

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electrical engineer — Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151

landscape architect — Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

PROJECT NORTH

REGISTERED PROFESSIONAL ENGINEER 2026-07-08 E.J.C. HOWDHURY 100072672 PROVINCE OF ONTARIO

LEA Consulting Ltd. 40 University Avenue, Suite 503 Toronto, Ontario M5J 1T1 — Tel: 905 470 0015 Fax: 905 470 0030 WWW.LEA.CA

THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

SECOND FLOOR FRAMING PLAN

scale:	As indicated
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S240
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

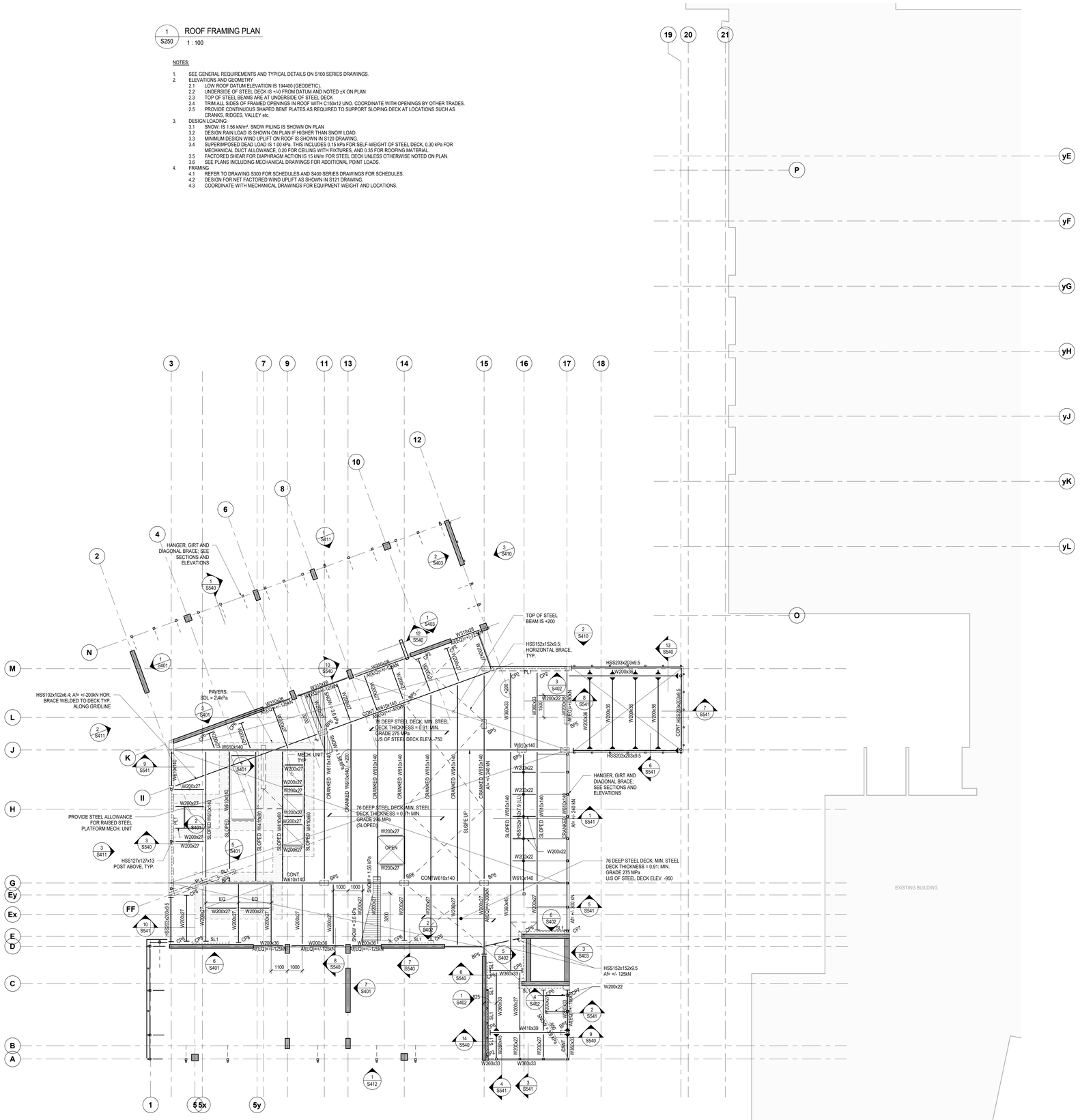
DRAWING LAST EDITED: 2026-07-03 3:05:05 PM

1 / S250 ROOF FRAMING PLAN

1 : 100

NOTES:

1. SEE GENERAL REQUIREMENTS AND TYPICAL DETAILS ON S100 SERIES DRAWINGS.
2. ELEVATIONS AND GEOMETRY
   - 2.1 LOW ROOF DATUM ELEVATION IS 194400 (GEODETIC).
   - 2.2 UNDERSIDE OF STEEL DECK IS +/-0 FROM DATUM AND NOTED ±X ON PLAN
   - 2.3 TOP OF STEEL BEAMS ARE AT UNDERSIDE OF STEEL DECK
   - 2.4 TRIM ALL SIDES OF FRAMED OPENINGS IN ROOF WITH C150x12 UNO. COORDINATE WITH OPENINGS BY OTHER TRADES.
   - 2.5 PROVIDE CONTINUOUS SHAPED BENT PLATES AS REQUIRED TO SUPPORT SLOPING DECK AT LOCATIONS SUCH AS CRANKS, RIDGES, VALLEY etc.
3. DESIGN LOADING:
   - 3.1 SNOW: IS 1.56 kN/m². SNOW PILING IS SHOWN ON PLAN
   - 3.2 DESIGN RAIN LOAD IS SHOWN ON PLAN IF HIGHER THAN SNOW LOAD.
   - 3.3 MINIMUM DESIGN WIND UPLIFT ON ROOF IS SHOWN IN S120 DRAWING.
   - 3.4 SUPERIMPOSED DEAD LOAD IS 1.00 kPa. THIS INCLUDES 0.15 kPa FOR SELF-WEIGHT OF STEEL DECK, 0.30 kPa FOR MECHANICAL DUCT ALLOWANCE, 0.20 FOR CEILING WITH FIXTURES, AND 0.35 FOR ROOFING MATERIAL.
   - 3.5 FACTORED SHEAR FOR DIAPHRAGM ACTION IS 15 kN/m FOR STEEL DECK UNLESS OTHERWISE NOTED ON PLAN.
   - 3.6 SEE PLANS INCLUDING MECHANICAL DRAWINGS FOR ADDITIONAL POINT LOADS.
4. FRAMING
   - 4.1 REFER TO DRAWING S300 FOR SCHEDULES AND S400 SERIES DRAWINGS FOR SCHEDULES.
   - 4.2 DESIGN FOR NET FACTORED WIND UPLIFT AS SHOWN IN S121 DRAWING.
   - 4.3 COORDINATE WITH MECHANICAL DRAWINGS FOR EQUIPMENT WEIGHT AND LOCATIONS.



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4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER

consultants

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electrical engineer — Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151

landscape architect — Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

PROJECT NORTH N

REGISTERED PROFESSIONAL ENGINEER 2026-07-08 E.J. CHOWDHURY 100200100 PROVINCE OF ONTARIO

SHS LEA

LEA Consulting Ltd. 40 University Avenue, Suite 503 Toronto, Ontario M5J 1T1 — Tel. 905 470 0015 Fax. 905 470 0030 WWW.LEA.CA

THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

LOW ROOF FRAMING PLAN

scale:	As indicated
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S250
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

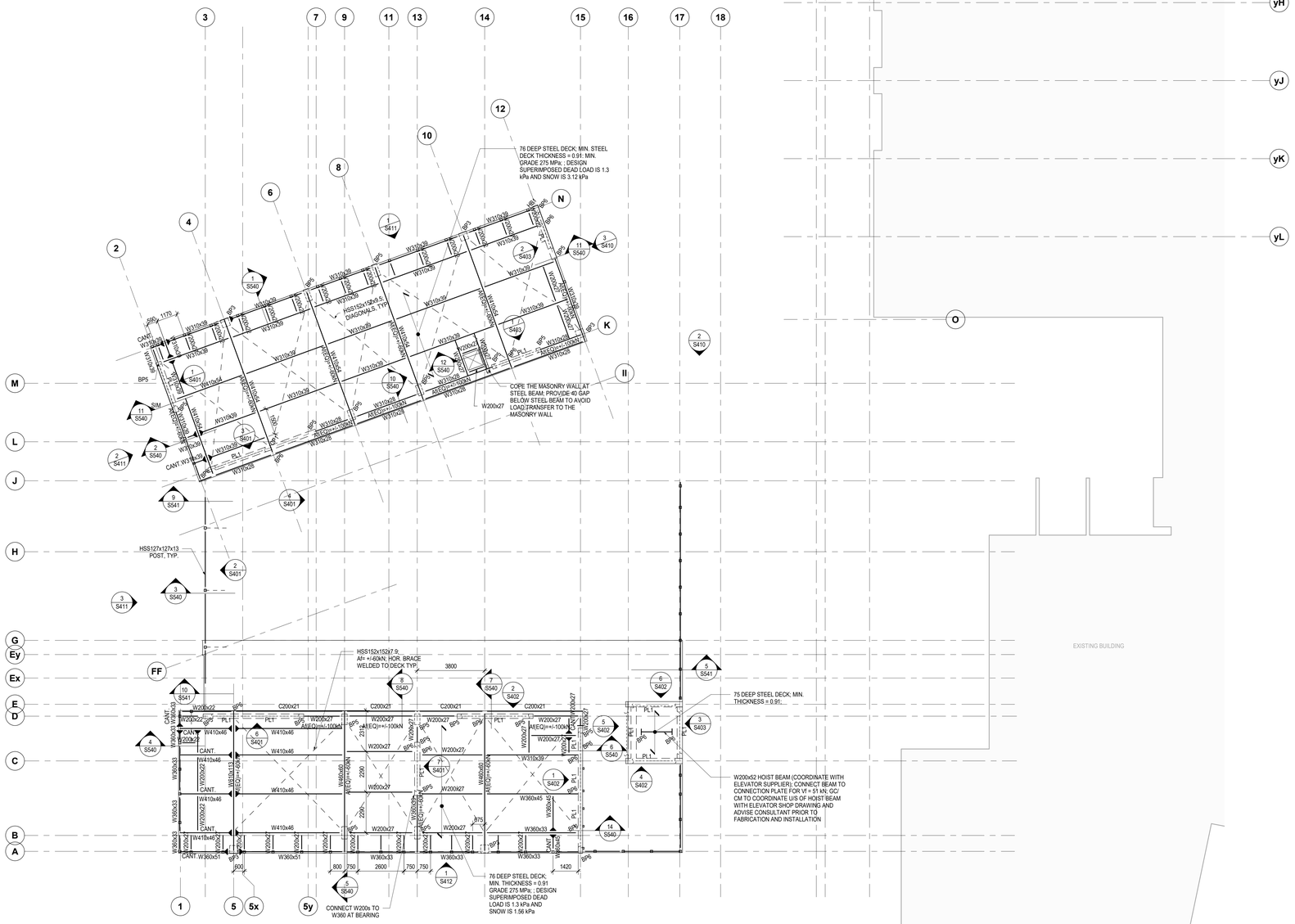
DRAWING LAST EDITED: 2026-07-03 3:05:06 PM

1 / S260 HIGH ROOF FRAMING PLAN

1 : 100

NOTES:

1. SEE GENERAL REQUIREMENTS AND TYPICAL DETAILS ON S100 SERIES DRAWINGS.
2. ELEVATIONS AND GEOMETRY
   - 2.1 HIGH ROOF DATUM ELEVATION IS 11400 (GEODETIC).
   - 2.2 UNDERSIDE OF STEEL DECK IS +/-0 FROM DATUM AND NOTED ±X ON PLAN
   - 2.3 TOP OF STEEL BEAMS ARE AT UNDERSIDE OF STEEL DECK
   - 2.4 TRIM ALL SIDES OF FRAMED OPENINGS IN ROOF WITH C150x12 UNO. COORDINATE WITH OPENINGS BY OTHER TRADES.
   - 2.5 PROVIDE CONTINUOUS SHAPED BENT PLATES AS REQUIRED TO SUPPORT SLOPING DECK AT LOCATIONS SUCH AS CRANKS, RIDGES, VALLEY etc.
3. DESIGN LOADING:
   - 3.1 SNOW: IS 1.56 kN/m². SNOW PILING IS SHOWN ON PLAN
   - 3.2 DESIGN RAIN LOAD IS SHOWN ON PLAN IF HIGHER THAN SNOW LOAD.
   - 3.3 MINIMUM DESIGN WIND UPLIFT ON ROOF IS SHOWN IN S120 DRAWING.
   - 3.4 SUPERIMPOSED DEAD LOAD IS 1.00 kPa. THIS INCLUDES 0.15 kPa FOR SELF-WEIGHT OF STEEL DECK, 0.30 kPa FOR MECHANICAL DUCT ALLOWANCE, 0.20 FOR CEILING WITH FIXTURES, AND 0.35 FOR ROOFING MATERIAL.
   - 3.5 FACTORED SHEAR FOR DIAPHRAGM ACTION IS 15 kN/m FOR STEEL DECK UNLESS OTHERWISE NOTED ON PLAN.
   - 3.6 SEE PLANS INCLUDING MECHANICAL DRAWINGS FOR ADDITIONAL POINT LOADS.
4. FRAMING
   - 4.1 REFER TO DRAWING S300 FOR SCHEDULES AND S400 SERIES DRAWINGS FOR SCHEDULES.
   - 4.2 DESIGN FOR NET FACTORED WIND UPLIFT AS SHOWN IN S121 DRAWING.
   - 4.3 COORDINATE WITH MECHANICAL DRAWINGS FOR EQUIPMENT WEIGHT AND LOCATIONS.



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No	Date	Revision/Issued
1	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
2	2026-07-08	ISSUED FOR TENDER

consultants

architect — CS&P Architects Inc. 2345 Yonge St., Suite 200 Toronto, ON, M4P 2E5 Phone.416.482.5002 Fax.416.482.5040

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landscape architect — Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

PROJECT NORTH

REGISTERED PROFESSIONAL ENGINEER 2026-07-08 E.U. HOWSHURT 100072522 PROVINCE OF ONTARIO

LEA Consulting Ltd. 40 University Avenue, Suite 503 Toronto, Ontario M5J 1T1 Tel: 905 470 0015 Fax: 905 470 0030 WWW.LEA.CA

THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

HIGH ROOF FRAMING PLAN

scale:	As indicated
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S260
	Revision: 2

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-03 3:05:07 PM

COLUMN SCHEDULE

U/S OF HIGH ROOF (11400)																															U/S OF HIGH ROOF (11400)
U/S OF DECK (HP)																															U/S OF DECK (HP)
10600 SECOND LEVEL	450x450 12-25 V 10@200 TIES	450x450 12-20 V 10@300 TIES						600x300 12-20 V 10@300 TIES	600x300 12-20 V 10@300 TIES	600x300 12-20 V 10@300 TIES											HSS16x6.5										10600 SECOND LEVEL
190600 MEZZANINE			867x300 12-20 V 10@300 TIES		300x700 14-20 V 10@300 TIES	300x700 14-20 V 10@300 TIES						600x300 12-20 V 10@300 TIES	600x300 12-20 V 10@300 TIES	600x300 12-20 V 10@300 TIES		HSS203x203x6.5	HSS203x203x13			600x300 12-20 V 10@300 TIES	600x300 12-20 V 10@300 TIES			600x300 12-20 V 10@300 TIES	600x300 12-20 V 10@300 TIES	450x450 8-25 V 10@300 TIES	600x300 12-20 V 10@300 TIES	300x300 4-25 V 10@200 TIES	600x300 12-20 V 10@300 TIES	600x300 12-20 V 10@300 TIES	190600 MEZZANINE
187000 UPPER LEVEL GROUND FLOOR																															187000 UPPER LEVEL GROUND FLOOR
185800 GROUND LEVEL											HSS219x4.8				700x400 12-25 V 10@300 TIES		US B.PL. - 0	US B.PL. - 0	HSS203x203x7.9 US B.PL. - 0					HSS203x203x7.9 US B.PL. - 0							185800 GROUND LEVEL
184000 LOWER LEVEL				450x450 12-25 V 10@300 TIES			450x450 12-20 V 10@300 TIES		600x300 10-20 V 10@300 TIES	600x300 10-20 V 10@300 TIES												EXTEND THE VERTICAL REINFORCEMENT AND TIES TO TOP OF FOUNDATION	EXTEND THE VERTICAL REINFORCEMENT AND TIES TO TOP OF FOUNDATION	EXTEND THE VERTICAL REINFORCEMENT AND TIES TO TOP OF FOUNDATION								184000 LOWER LEVEL
181000 BASE PLATE (PL.) ANCHOR ROD SLS P (kN) ULS Pf (kN) Vf (kN)	SLS: P=527 ULS: Pf=750	SLS: P=300 ULS: Pf=400		SLS: P=660 ULS: Pf=900	SLS: P=1100 ULS: Pf=1400	SLS: P=315 ULS: Pf=430	SLS: P=650 ULS: Pf=850		SLS: P=500 ULS: Pf=700	SLS: P=400 ULS: Pf=550		SLS: P=675 ULS: Pf=850	SLS: P=550 ULS: Pf=700	SLS: P=800 ULS: Pf=1000	SLS: P=600 ULS: Pf=800	PL. BP2 - SLS: P=300 ULS: Pf=400 Vf=-	PL. BP2 - SLS: P=350 ULS: Pf=450 Vf=-	PL. BP2 - SLS: P=70 ULS: Pf=100 Vf=-	SLS: P=175 ULS: Pf=225	SLS: P=175 ULS: Pf=225	PL. - - SLS: P=- ULS: Pf=- Vf=-	PL. BP1 - SLS: P=215 ULS: Pf=350 Vf=-	SLS: P=550 ULS: Pf=700	SLS: P=500 ULS: Pf=650	SLS: P=175 ULS: Pf=250	PL. - - SLS: P=150 ULS: Pf=200 Vf=-	PL. - - SLS: P=100 ULS: Pf=150 Vf=-	PL. - - SLS: P=150 ULS: Pf=200 Vf=-	PL. - - SLS: P=225 ULS: Pf=275 Vf=-	181000
Column Locations	A-5	A-14	A-16	B-5	B-9	B-13	B-14	B(900)-17	D-9	D-13	D-20(150)	G-11	G-14	G-15	G(1798)-17	G-19	H-19	H-23	J-16	J-17	J-19	J-23	K-6	K-8	K-12	L(-450)-15	L(-300)-16(-300)	L-17	M-15		

COLUMN SCHEDULE

U/S OF HIGH ROOF (11400)																															U/S OF HIGH ROOF (11400)
U/S OF DECK (HP)																															U/S OF DECK (HP)
10600 SECOND LEVEL			HSS168x6.5															HSS152x152x7.9													10600 SECOND LEVEL
190600 MEZZANINE		HSS203x203x13					450x450 12-20 V 10@300 TIES	300x700 14-20 V 10@300 TIES	300x700 14-20 V 10@300 TIES	450x450 12-20 V 10@300 TIES										600x300 12-20 V 10@300 TIES	600x300 12-20 V 10@300 TIES										190600 MEZZANINE
187000 UPPER LEVEL GROUND FLOOR																															187000 UPPER LEVEL GROUND FLOOR
185800 GROUND LEVEL	600x300 12-20 V 10@300 TIES	US B.PL. - 0		HSS203x203x7.9 US B.PL. - 0	HSS203x203x7.9 US B.PL. - 0	HSS203x203x7.9 US B.PL. - 0					HSS219x6.5 US B.PL. - 250	HSS219x6.5 US B.PL. - 250	HSS219x6.5 US B.PL. - 250		HSS219x4.8					600x300 12-20 V 10@300 TIES			HSS219x6.5 US B.PL. - 250	HSS219x6.5 US B.PL. - 250	HSS219x6.5 US B.PL. - 250	HSS219x6.5 US B.PL. - 250	HSS219x6.5 US B.PL. - 250	HSS219x6.5 US B.PL. - 250	HSS219x6.5 US B.PL. - 250	HSS219x6.5 US B.PL. - 250	185800 GROUND LEVEL
184000 LOWER LEVEL														300x500 8-25 V 10@200 TIES		300x500 8-25 V 10@200 TIES	300x800 14-20 V 10@300 TIES														184000 LOWER LEVEL
181000 BASE PLATE (PL.) ANCHOR ROD SLS P (kN) ULS Pf (kN) Vf (kN)	PL. - - SLS: P=150 ULS: Pf=200 Vf=-	PL. BP1 - SLS: P=450 ULS: Pf=650 Vf=-	PL. - - SLS: P=- ULS: Pf=- Vf=-	PL. BP2 - SLS: P=- ULS: Pf=- Vf=-	PL. BP2 - SLS: P=70 ULS: Pf=100 Vf=-	PL. BP2 - SLS: P=- ULS: Pf=- Vf=-	PL. - - SLS: P=350 ULS: Pf=450 Vf=-	PL. - - SLS: P=375 ULS: Pf=500 Vf=-	PL. - - SLS: P=375 ULS: Pf=500 Vf=-	PL. - - SLS: P=450 ULS: Pf=650 Vf=-	PL. BP2 - SLS: P=70 ULS: Pf=100 Vf=-						SLS: P=350 ULS: Pf=450		SLS: P=300 ULS: Pf=400	SLS: P=350 ULS: Pf=450	SLS: P=500 ULS: Pf=650	PL. BP2 - SLS: P=70 ULS: Pf=100 Vf=-		PL. BP2 - SLS: P=70 ULS: Pf=100 Vf=-		PL. BP2 - SLS: P=70 ULS: Pf=100 Vf=-		PL. BP2 - SLS: P=70 ULS: Pf=100 Vf=-			181000
Column Locations	M-18	M-19	M(-486)-19	M(1600)-21(575)	M(1600)-22	M(-2043)-22	N-4	N-5y	N-8	N-10	O-20	P-20	P-21	Ex-5x	Ex-20(150)	Ey-5y	Ey-9	Ey-16	FF-11(-558)	II-11	II-14	yF-20	yF-21	yG-20	yG-21	yH-20	yH-21	yJ-20	yJ-21		

COLUMN SCHEDULE NOTES:

1. SEE GENERAL REQUIREMENTS AND TYPICAL DETAILS ON DRAWING S100 SERIES.
2. ALL LOADS ARE FACTORED ULTIMATE LIMIT STATE (ULS) UNLESS NOTED OTHERWISE.
3. COLUMN DIMENSION GIVEN FIRST TO BE IN NORTH-SOUTH DIRECTION.
4. PROVIDE GALVANIZED DOVETAIL ANCHOR SLOTS TO ALL FACES OF COLUMNS ABUTTING MASONRY, AND TO ALL FACES WITH MASONRY VENEER.
5. PROVIDE COMPRESSION LAP SPLICES FOR VERTICAL REINFORCEMENT UNLESS OTHERWISE NOTED.
6. "B" DENOTES CLASS B TENSION LAP SPLICE FOR VERTICAL REINFORCEMENT.
7. SPLICE 35M BARS AND GREATER USING MECHANICAL COUPLERS.
8. ALL LOADS ARE ULTIMATE LIMIT STATE (ULS) UNLESS NOTED OTHERWISE.
9. PROVIDE 4-19Ø ANCHOR RODS (400 EMBEDMENT PLUS 75 HOOK) FOR EACH BASE PLATE, UNLESS OTHERWISE NOTED IN SCHEDULE.
10. COLUMN BASES TO BE GROUTED WITH 50mm OF GROUT. SEE SPECIFICATIONS.
11. W-COLUMNS: BASE PLATE DIMENSION GIVEN FIRST TO BE PARALLEL TO COLUMN WEB.
12. HSS COLUMNS: BASE PLATE DIMENSION GIVEN FIRST TO BE IN NORTH-SOUTH DIRECTION.
13. PROVIDE LEVELLING SCREWS FOR BASE PLATES.
14. WELD BASE PLATES TO COLUMNS TO DEVELOP ANCHOR RODS.
15. COLUMN SPLICES SHALL BE WELDED TO DEVELOP FULL TENSION DEVELOPMENT.
16. PROVIDE CAP PLATES ON ALL HSS COLUMNS. PROVIDE CAP PLATES ON ALL OTHER COLUMNS WHERE REQUIRED FOR SUPPORT OF JOISTS, DECK, WINDOW WASHING ANCHORS ETC.
17. WHERE MOMENTS OR SHEAR FORCES ARE PRESENTED SINGULARLY: THE MOMENT/SHEAR FORCE IS IN THE STRONG DIRECTION. IF THE COLUMN IS SQUARE, THE MOMENT/SHEAR FORCE IS IN BOTH DIRECTIONS UNLESS NOTED OTHERWISE.
18. WHERE MOMENTS OR SHEARS ARE PRESENTED ABOUT TWO AXES: THE FIRST MOMENT/SHEAR FORCE IS IN THE STRONG DIRECTION AND THE SECOND IN THE WEAK DIRECTION. IF THE COLUMN IS SQUARE, THE FIRST MOMENT/SHEAR FORCE IS PARALLEL TO THE PROJECT NORTH NORTH-SOUTH DIRECTION.

CONCRETE COLUMN SCHEDULE LEGEND:

COLUMN TAG(S):

300x800	COLUMN DIMENSION(S)
8-20 V	VERTICAL REINFORCEMENT
10@300 TIES	TIE REINFORCEMENT
COMMENTS	COLUMN-SPECIFIC COMMENTS (IF APPLICABLE)

BASE TAG:

SLS P=1400 (800)	SLS (UNFACTORED) AXIAL REACTION (UPLIFT IN BRACKETS, IF APPLICABLE)
ULS Pf=1800 (800)	ULS (FACTORED) AXIAL REACTION (UPLIFT IN BRACKETS, IF APPLICABLE)

STEEL COLUMN SCHEDULE LEGEND:

COLUMN TAG(S):

W360x60	COLUMN SECTION
COMMENTS	COLUMN-SPECIFIC COMMENTS (IF APPLICABLE)

BASE TAG:

BP1	BASE PLATE (REFER TO BASE PLATE SCHEDULE)
SLS P=1000 (600)	SLS (UNFACTORED) AXIAL REACTION (UPLIFT IN BRACKETS, IF APPLICABLE)
ULS Pf=1800 (800)	ULS (FACTORED) AXIAL REACTION (UPLIFT IN BRACKETS, IF APPLICABLE)
Vf=250 / 150	ULS (FACTORED) SHEAR REACTION(S), IF APPLICABLE (REFER TO NOTES)
Mf=150 / 120	ULS (FACTORED) BENDING MOMENT REACTION(S), IF APPLICABLE (REFER TO NOTES)
Tmf=60	ULS (FACTORED) TORSIONAL MOMENT REACTION, IF APPLICABLE

STEEL COLUMN SYMBOL LEGEND

BASE CONNECTIONS:

- BASE PLATE
- STEEL BEAM BEARING
- CONCRETE BEAM BEARING

TOP CONNECTIONS:

- COLUMN SPLICE

Key to Detail Location

NO. / NO. — Detail Number / Drawing Number

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2	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
3	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
4	2026-07-08	ISSUED FOR TENDER

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Phone.905.475.1900
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structural engineer — LEA Consulting Ltd.
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landscape architect — Quinn Design Associates Inc.
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2026-07-08
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100175420
PROVINCE OF ONTARIO

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THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

COLUMN SCHEDULE

scale:	As indicated
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S300

Revision: 4

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-03 3:05:08 PM

COLUMN SCHEDULE

U/S OF DECK (HP)							U/S OF DECK (HP)
10600 SECOND LEVEL							10600 SECOND LEVEL
190600 MEZZANINE							190600 MEZZANINE
187000 UPPER LEVEL GROUND FLOOR							187000 UPPER LEVEL GROUND FLOOR
185800 GROUND LEVEL	HS219x9.5 U/S B PL -250	HS219x9.5 U/S B PL -250	HS219x9.5 U/S B PL -250	HS219x9.5 U/S B PL -250	HSS219x4.8 U/S B PL -493	HSS219x4.8 U/S B PL -493	185800 GROUND LEVEL
184000 LOWER LEVEL							184000 LOWER LEVEL
181000 BASE PLATE (PL.) ANCHOR ROD SLS P (kN) ULS Pf (kN) Vf (kN)	PL. BP2 - SLS: P=70 ULS: Pf=100 Vf=-		PL. BP2 - SLS: P=70 ULS: Pf=100 Vf=-				181000
Column Locations	yK-20	yK-21	yL-20	yL-21	C(2500)x13x(-417)	x12(-3021)-Ex	

Key to Detail Location

NO. — Detail Number
NO. — Drawing Number

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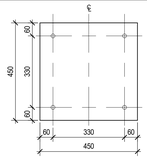
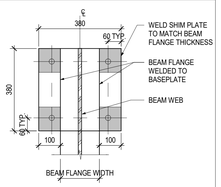
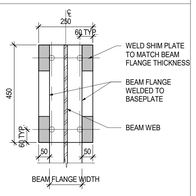
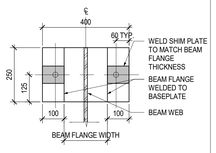
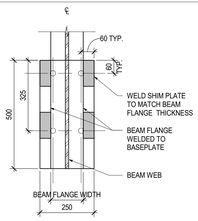
COLUMN SCHEDULE

scale:	1 : 75
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S301
	Revision: 4

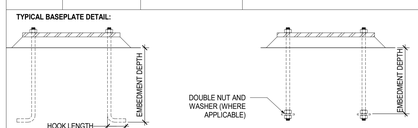
A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-03 3:05:08 PM

BASEPLATE SCHEDULE

MARK	SIZE	ANCHORS	DETAILS
BP1	450x25x450	4-19Ø ANCHOR RODS x 600 LG. EMBED + 75 HOOK AT EMBEDDED END	
BP2	450x30x450	4-25Ø ANCHOR RODS x 1000 LG. EMBED + DOUBLE NUT WITH WASHER AT EMBEDDED END	SEE BP1 DETAIL
BP3	380x20x380	4-19DIAM. ANCHOR RODS x600LG + 75HOOK	 NOTE: 1. PROVIDE 50 GROUT BELOW BASEPLATE
BP4	NOT USED	NOT USED	
BP5	450x20x250	4-19DIAM. ANCHOR RODS x600LG + 75HOOK	 NOTE: 1. PROVIDE 50 GROUT BELOW BASEPLATE 2. PROVIDE WALL POCKET
BP6	250x20x400	2-19DIAM. ANCHOR RODS x600LG + 75HOOK	 NOTE: 1. PROVIDE 50 GROUT BELOW BASEPLATE 2. PROVIDE WALL POCKET 3. CUT HOLES THRU FLANGES AS REQUIRED FOR ANCHOR INSTALLATION
BP7	250x30x500	4-25DIA. ANCHOR RODS x 1000 LG. EMBED: 75	

TYPICAL BASEPLATE DETAIL:

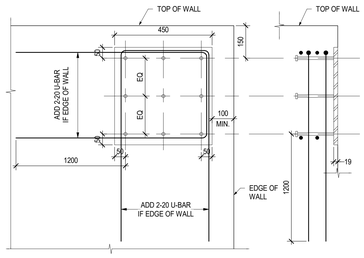
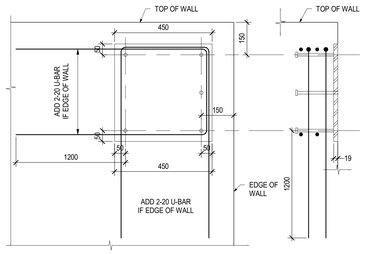
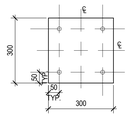
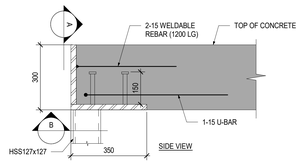
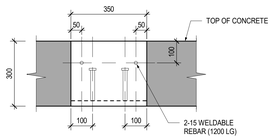
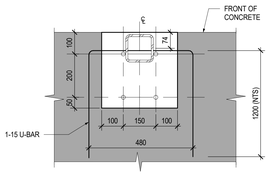
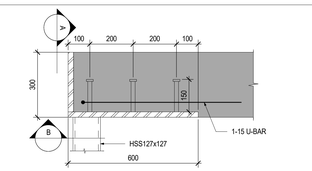
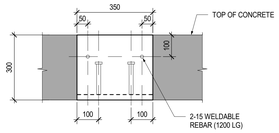
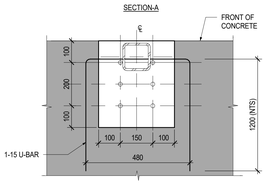
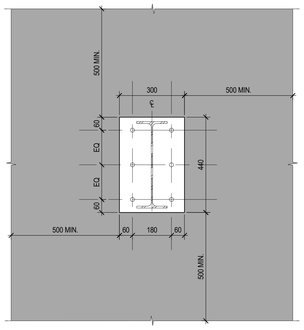
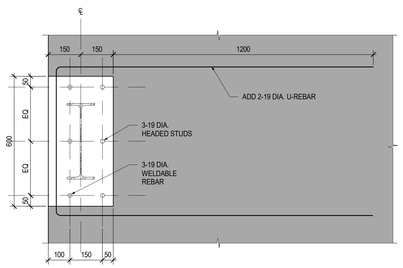
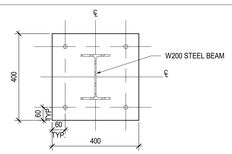


NOTE: PROVIDE ADDITIONAL COSTING ALLOWANCE OF 15% OF THE STRUCTURAL STEEL TONNAGE WEIGHT FOR STRUCTURAL STEEL CONNECTION (INCLUDING BASE PLATE AND CONNECTION PLATES). NOT ALL CONNECTION PLATES AND BASEPLATES ARE SHOWN IN THE SCHEDULE.

SHELF ANGLE AND PLATE SCHEDULE

MARK	DESCRIPTION	ANCHORS	REMARKS
SL1	CONT. L152x89x9.5 (LLV)	19Ø HILTI KWIK BOLT TZ2 @ 300 (121 EFFECTIVE EMBEDMENT)	
PL1	CONT. PL150x16	19Ø ANCHOR RODS WELDED @ 300 (1000 LG EMBEDMENT) WITH 75 HOOK	

CAST-IN PLATE SCHEDULE

MARK	SIZE	ANCHORS	DETAILS
CP1	600x25x450	9-19Ø x 175 LG. HEADED STUDS	 NOTE: BEAMS TO BE CENTERED TO THE PLATE HORIZONTALLY
CP2	450x25x450	4-19Ø x 175 LG. HEADED STUDS	**DETAILS**  NOTE: BEAMS TO BE CENTERED TO THE PLATE HORIZONTALLY
CP3	300x25x300	4-19 DIA x 150 LG. HEADED STUDS	 NOTE: BEAMS TO BE CENTERED TO THE PLATE
CP4	25 THICK CONNECTION PLATE SEE SECTION 2/S531	4-19 DIA x 150 LG. HEADED STUDS	 SIDE VIEW  SECTION-A  SECTION-B
CP5	25 THICK CONNECTION PLATE SEE SECTION 6/S531	4-19 DIA x 150 LG. HEADED STUDS	  SECTION-A  SECTION-B
CP6	300x25x440	6-19 DIA. x 150 LG. HEADED STUDS	
CP7	300x25x600	3-19 DIA. WELDABLE REBAR (1000 LG) + 3-19 DIA. x 150 LG. HEADED STUDS	
CP8	300x25x600	6-19 DIA. x 150 LG. HEADED STUDS	SEE DETAIL CP6
CP9	400x20x400	4-19 DIA. HILTI KWIK BOLT TZ2: 125 EFFECTIVE EMBEDMENT	

Key to Detail Location

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NO. — Drawing Number

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civil engineer — SCS Consulting Group Ltd. 30 Centurian Drive, Suite 100 Markham, ON, L3R 8B8 Phone.905.475.1900 Fax.905.475.8335

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landscape architect — Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

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SCHEDULES

scale:	1 : 10
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S310
	Revision: 4

A0 FILENAME: Autodesk Docs://2032 Sterling Hall/22317_SHS-STR-R25.rvt

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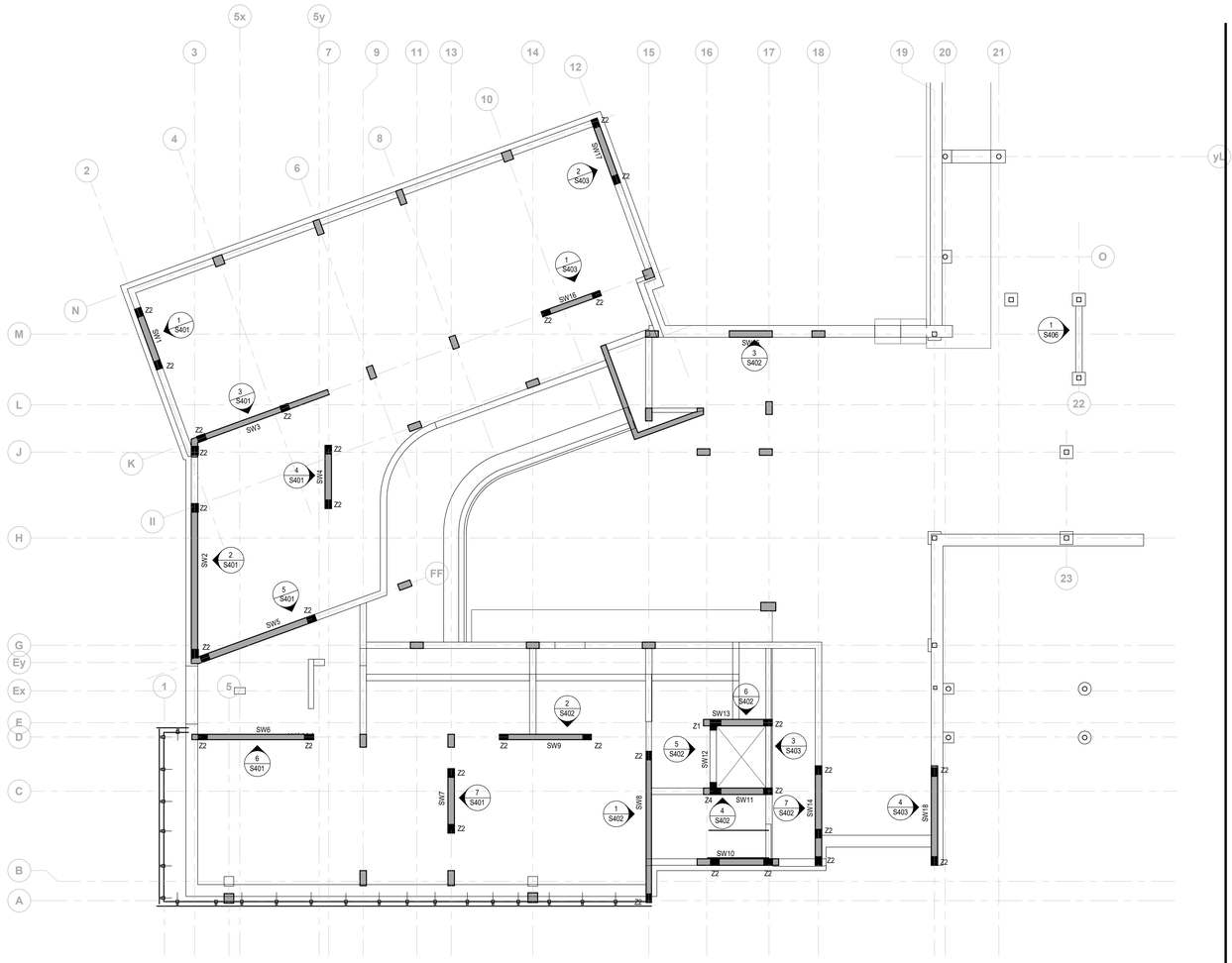
CONCRETE SHEAR WALL SCHEDULE

LEVEL / SW#	SW1	SW2	SW3	SW4	SW5	SW6	SW7	SW8	SW9
HIGH ROOF 11350									
ROOF 11050									
ELEV. 10400									
SECOND FLOOR 6600									
MEZZANINE 3000	300 WALL 15@300 VEF 15@300 HEF	300 WALL 15@300 VEF 15@300 HEF	250 WALL 15@325 VEF 15@325 HEF	300 WALL 15@300 VEF 15@300 HEF	250 WALL 15@325 VEF 15@325 HEF	250 WALL 15@325 VEF 15@325 HEF	300 WALL 15@300 VEF 15@300 HEF	250 WALL 15@325 VEF 15@325 HEF	250 WALL 15@325 VEF 15@325 HEF
UPPER GROUND FLOOR 1800									
GROUND FLOOR 0									
NATIVE SOIL -2000	FOUNDATION WALL; SEE PLAN AND SECTION	FOUNDATION WALL; SEE PLAN AND SECTION	FOUNDATION WALL; SEE PLAN AND SECTION	FOUNDATION WALL; SEE PLAN AND SECTION	FOUNDATION WALL; SEE PLAN AND SECTION				
TOP OF FOUNDATION LEVEL -3000									
REMARKS									

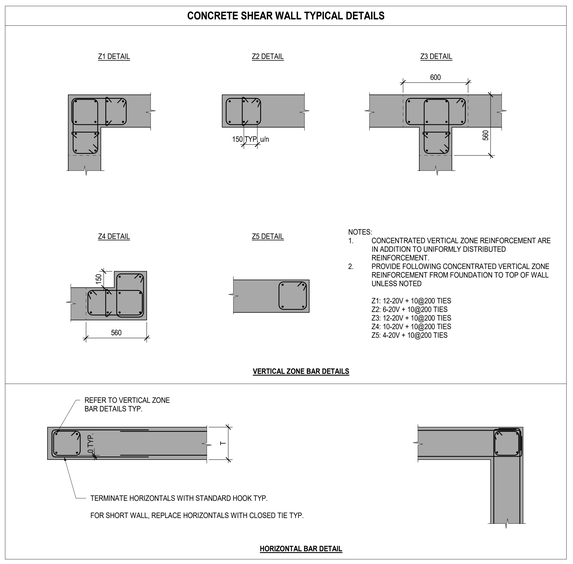
CONCRETE SHEAR WALL SCHEDULE

LEVEL / SW#	SW10	SW11	SW12	SW13	SW14	SW15	SW16	SW17	SW18
HIGH ROOF 11350									
ROOF 11050									
ELEV. 10400									
SECOND FLOOR 6600	ELEV. 5840	300 WALL 15@300 VEF 15@300 HEF	250 WALL 15@325 VEF 15@325 HEF	250 WALL 15@325 VEF 15@325 HEF	ELEV. 6018		250 WALL 15@325 VEF 15@325 HEF		ELEV. 6018
MEZZANINE 3000	300 WALL 15@300 VEF 15@300 HEF				300 WALL 15@300 VEF 15@300 HEF	300 WALL 15@400 VEF 15@400 HEF		300 WALL 15@300 VEF 15@300 HEF	300 WALL 15@300 VEF 15@300 HEF
UPPER GROUND FLOOR 1800									
GROUND FLOOR 0	FOUNDATION WALL; SEE PLAN AND SECTION				FOUNDATION WALL; SEE PLAN AND SECTION	FOUNDATION WALL; SEE PLAN AND SECTION		FOUNDATION WALL; SEE PLAN AND SECTION	
NATIVE SOIL -2000									
TOP OF FOUNDATION LEVEL -3000									
REMARKS									



1 S400 GROUND FLOOR SHEAR WALL KEY PLAN
1 : 100

CONCRETE SHEAR WALL TYPICAL DETAILS



CONCRETE SHEAR WALL SCHEDULE NOTES

1. FOR GENERAL REQUIREMENTS AND TYPICAL DETAILS SEE DRAWINGS S100 SERIES.
2. FOR LOCATIONS OF SCHEDULED SHEAR WALLS, SEE KEY PLAN.
3. CONCENTRATED VERTICAL REINFORCEMENT IS IN ADDITION TO TYPICAL DISTRIBUTED REINFORCEMENT. CONCENTRATED VERTICAL REINFORCED TO BE SPACED AT 150, UNLESS NOTED OTHERWISE, BUT NOT LESS THAN 100 APART, CENTRE TO CENTRE. CONCENTRATED VERTICAL REINFORCEMENT REFERS TO VERTICAL WALL REINFORCEMENT TIED AS A COLUMN.
4. SEE FOUNDATION PLANS AND SECTIONS FOR ADDITIONAL REINFORCEMENT AT AND BELOW THE GRADE LEVEL.
5. CARRY ALL WALLS AND REINFORCEMENT TO TOP OF FOOTINGS.
6. PROVIDE "L" DOWELS FROM WALL INTO FLOOR SLABS TO MATCH SIZE AND SPACING OF TYPICAL DISTRIBUTED WALL REINFORCEMENT UNLESS NOTED OTHERWISE. PROVIDE CLASS 'B' TENSION LAP WITH SLAB AND WALL STEEL REINFORCEMENT.
7. PROVIDE DOWELS FROM FOOTING TO WALL TO MATCH THE CONCENTRATED AND DISTRIBUTED VERTICAL WALL REINFORCEMENT. ALL DOWELS TO EXTEND TO THE BOTTOM OF THE MAT FOOTING. PROVIDE CLASS 'B' TENSION LAP WITH WALL REINFORCEMENT ABOVE. DOWELS FOR THE CONCENTRATED VERTICAL REINFORCEMENT SHALL TERMINATE WITH A 90 DEGREE HOOK.
8. IF TIES ARE SHOWN FOR CONCENTRATED VERTICAL REINFORCEMENT BARS, EVERY CORNER AND ALTERNATE LONGITUDINAL BAR SHALL HAVE LATERAL SUPPORT PROVIDED BY THE CORNER OF A TIE HAVING AN INCLUDED ANGLE OF NOT MORE THAN 135 DEGREE. BEND TYPICAL HORIZONTAL BARS, CARRY INTO CONCENTRATED VERTICAL REINFORCEMENT BAR CAGE AND HOOK AT FAR FACE.
9. PROVIDE CLASS 'B' TENSION LAPS FOR HORIZONTAL AND VERTICAL REINFORCEMENT.

Key to Detail Location

NO. / NO. — Detail Number / Drawing Number

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3	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
4	2026-07-08	ISSUED FOR TENDER

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THE STERLING HALL SCHOOL

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SHEAR WALL KEY PLAN AND SCHEDULE

scale:	As indicated
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S400 Revision: 4

A0 FILENAME: Autodesk Docs://2032 Sterling Hall/22317_SHS-STR-R25.rvt

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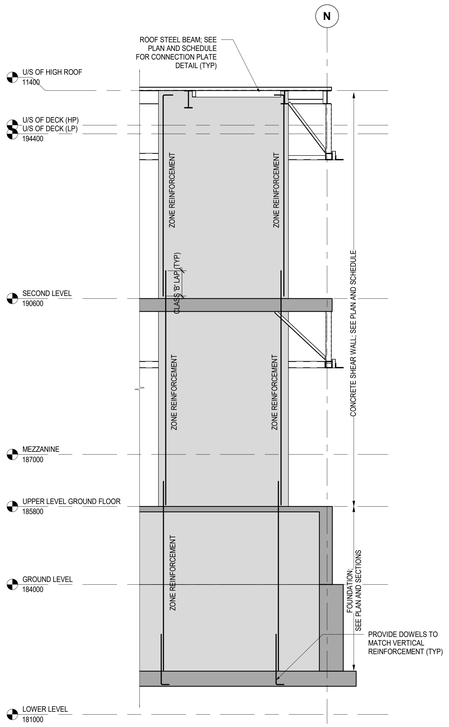

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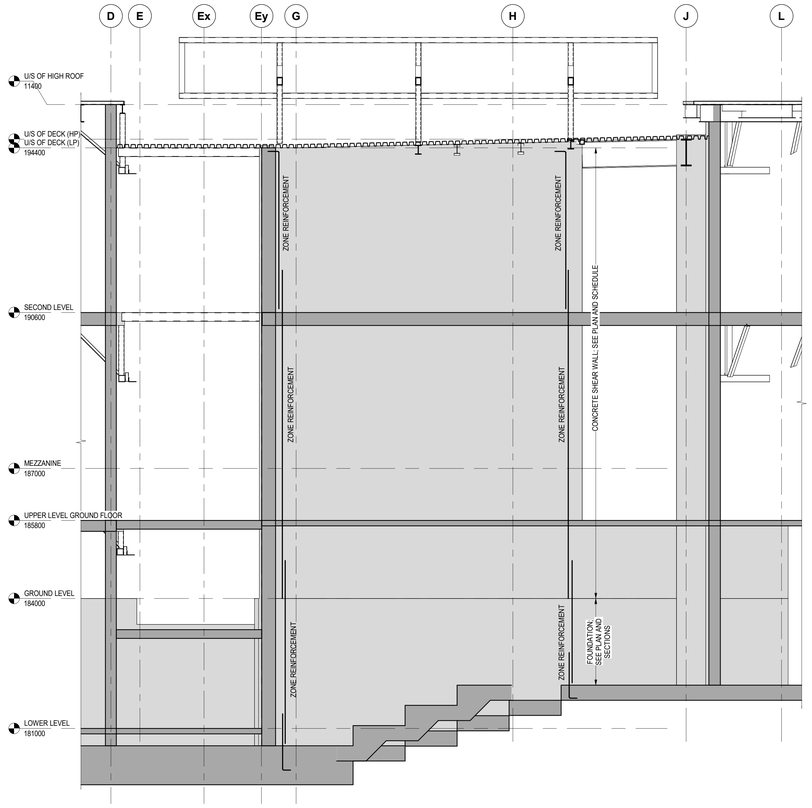
1 : 50



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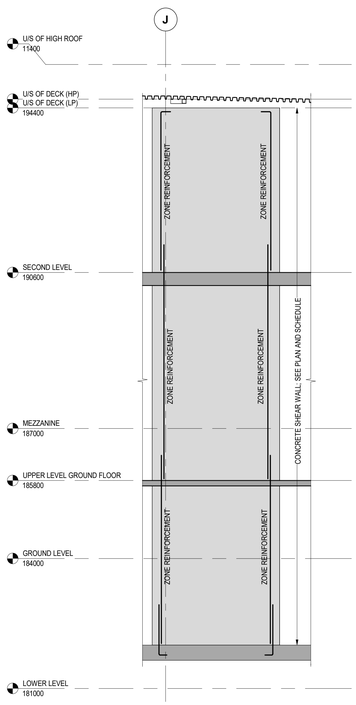
1 : 50



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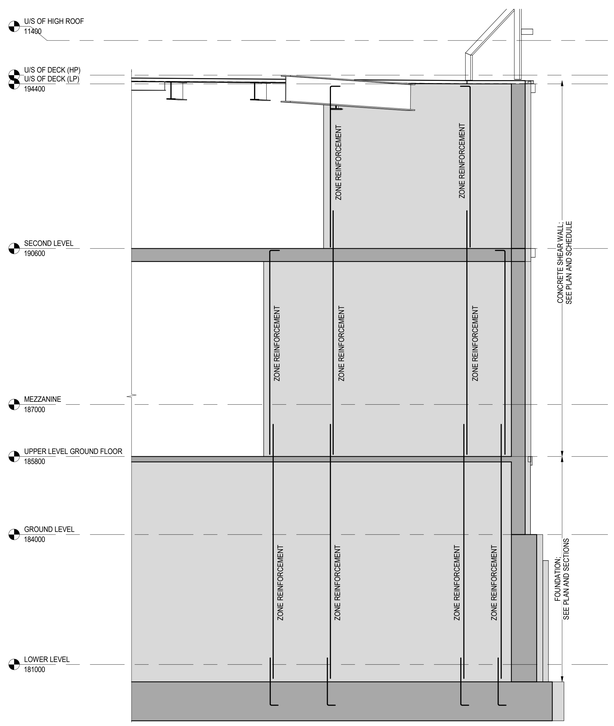

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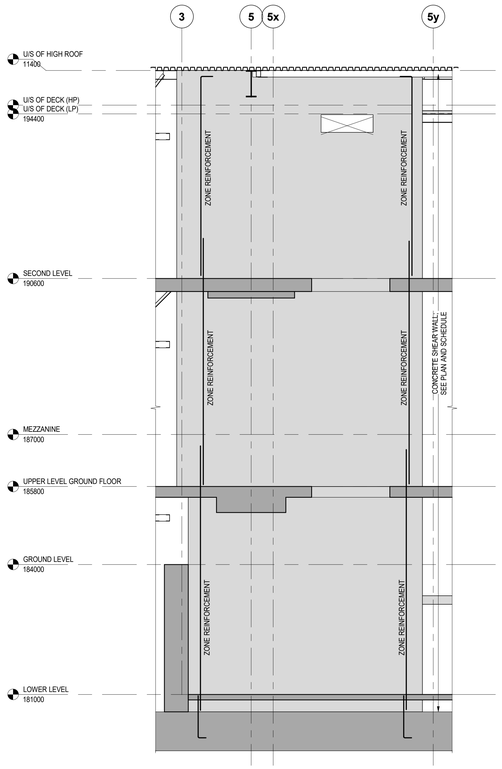

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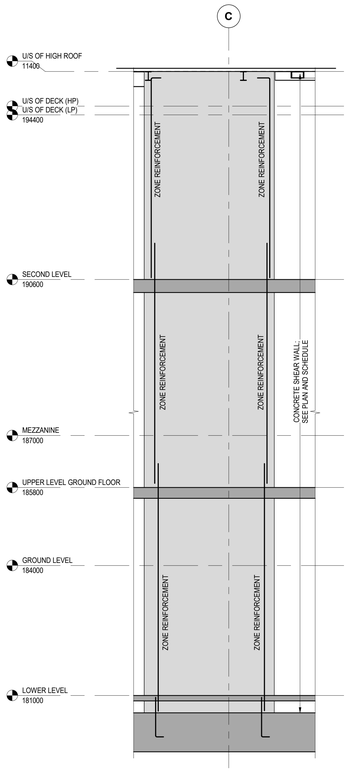

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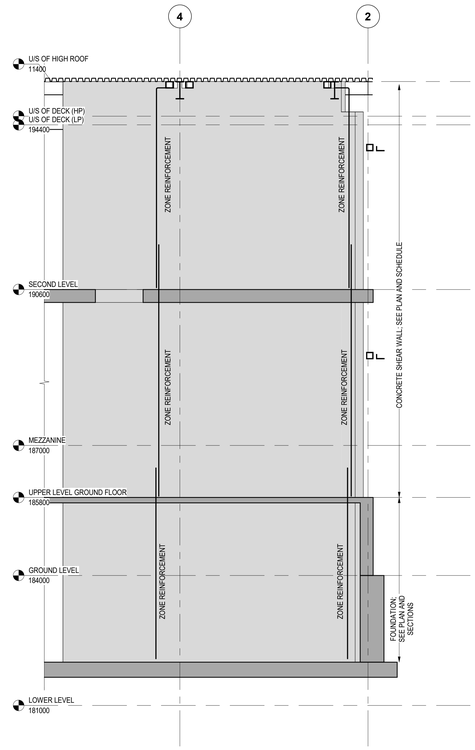

3 S401 SW3 ELEVATION
1 : 50

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SHEAR WALL ELEVATIONS

scale:	1 : 50
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S401
	Revision: 4

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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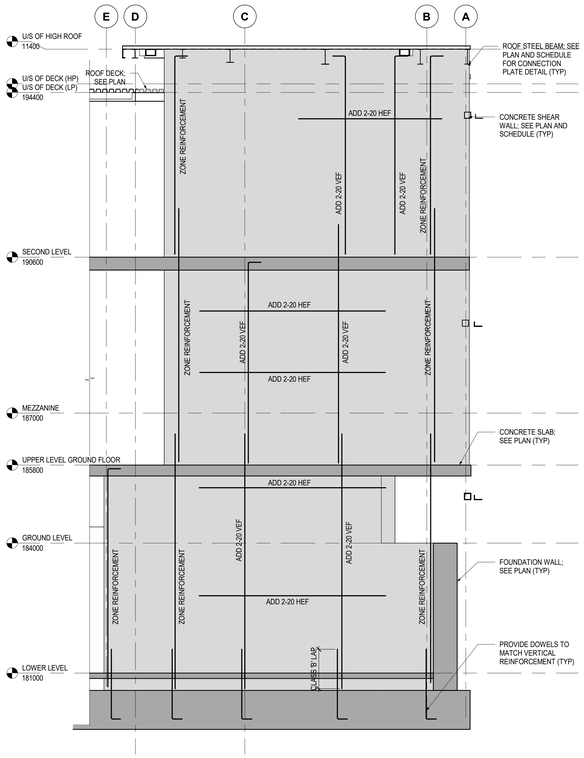

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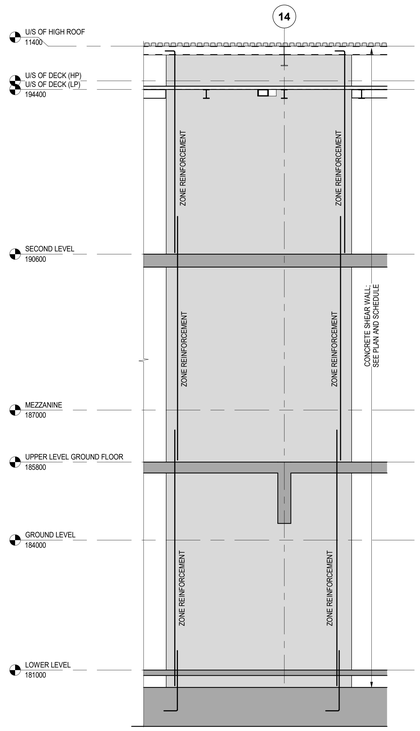

2 S402 ELEVATION
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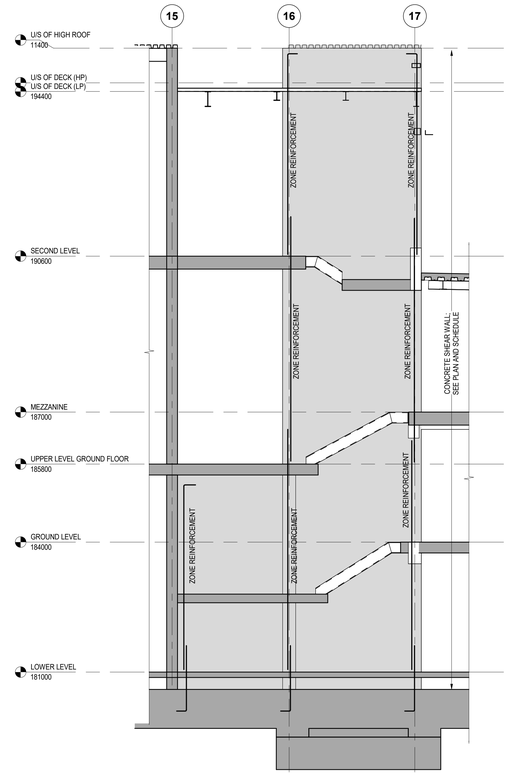
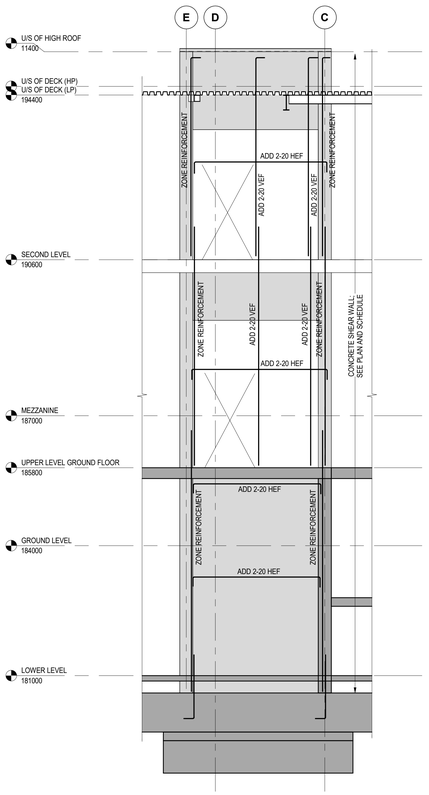
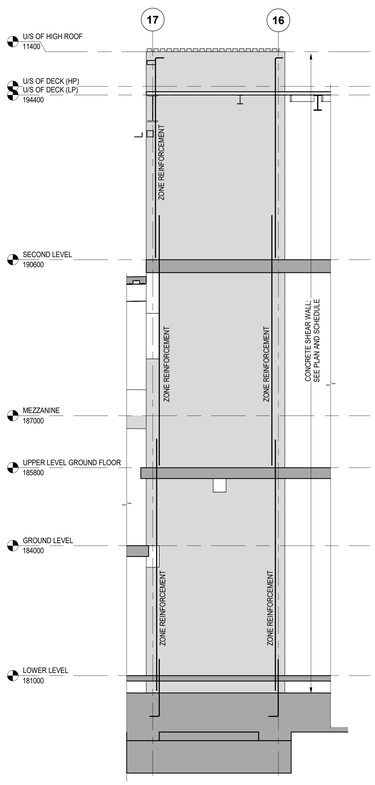
NOT USED



4 S402 ELEVATION
1 : 50



5 S402 ELEVATION
1 : 50



6 S402 ELEVATION
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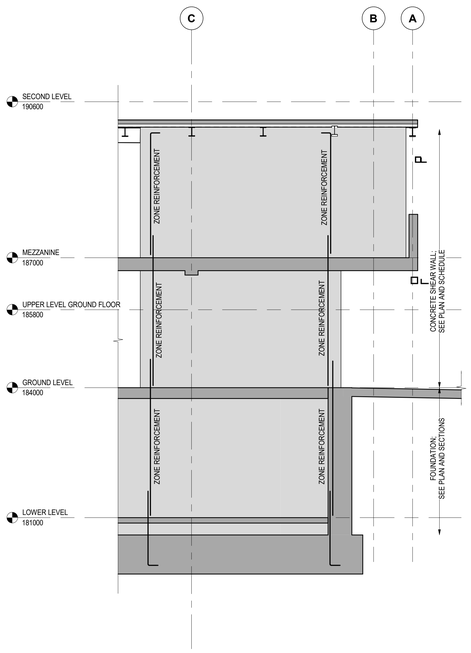

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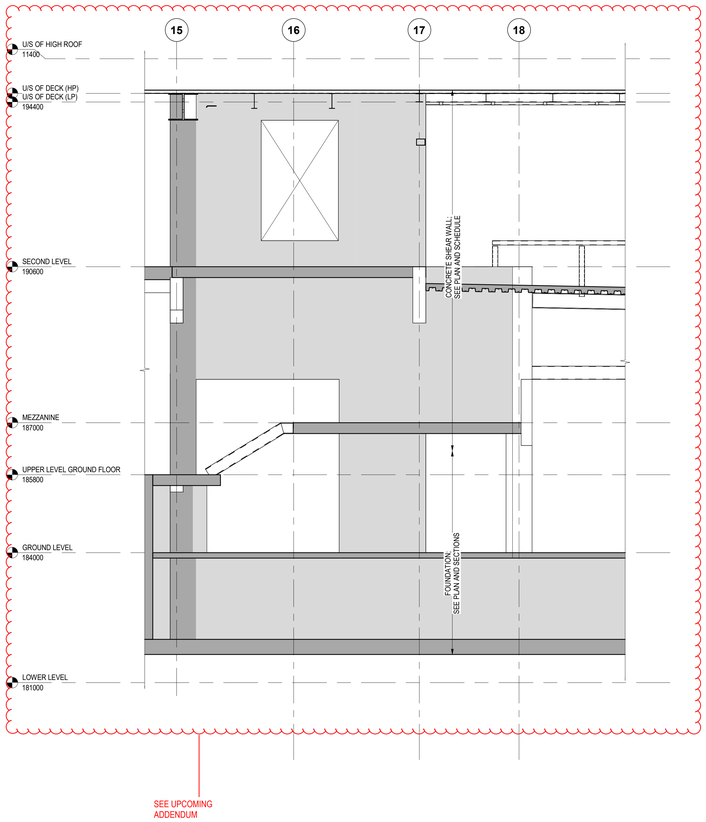

3 S402 ELEVATION
1 : 50

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SHEAR WALL ELEVATIONS

scale:	1 : 50
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S402
	Revision: 4

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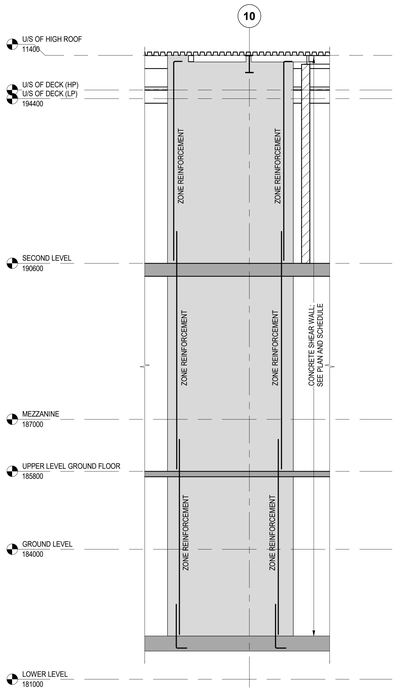
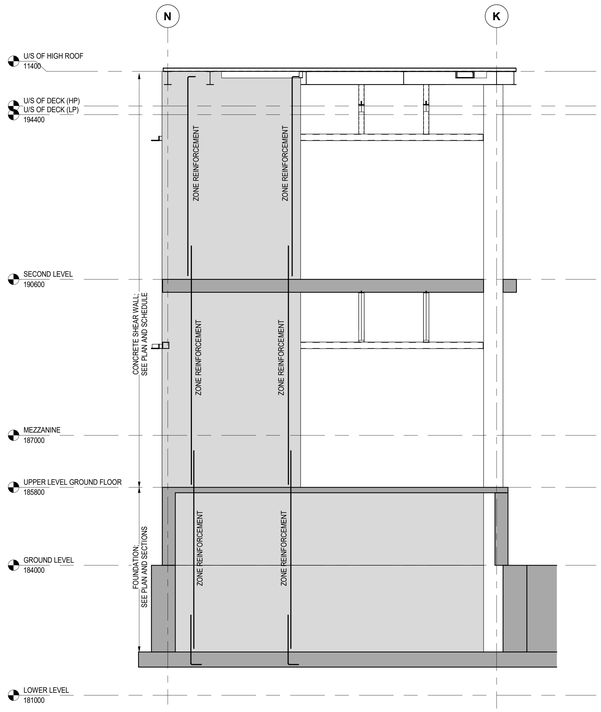


1 S403 ELEVATION
1 : 50



2 S403 ELEVATION
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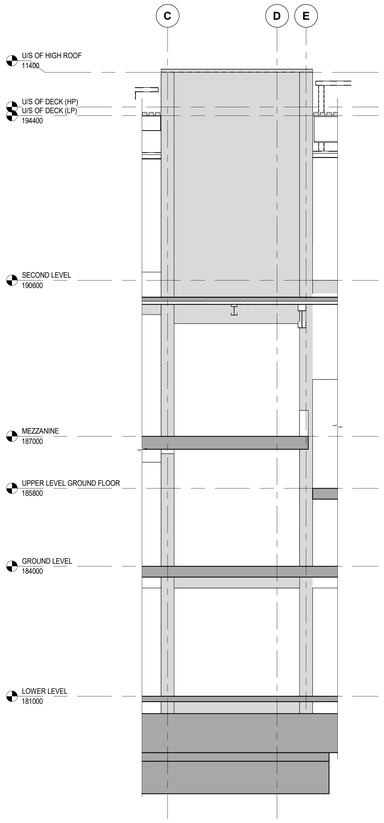

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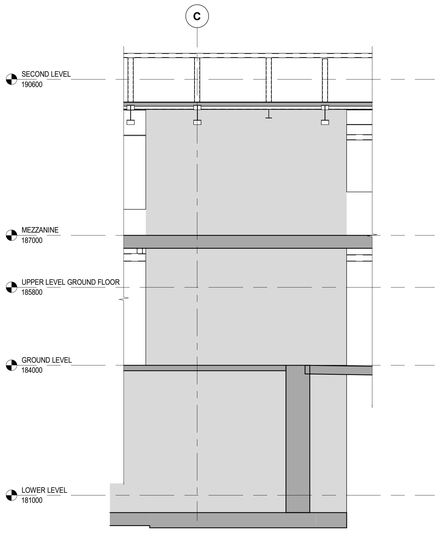

4 S403 ELEVATION
1 : 50

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SHEAR WALL ELEVATIONS

scale:	1 : 50
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S403
	Revision: 4

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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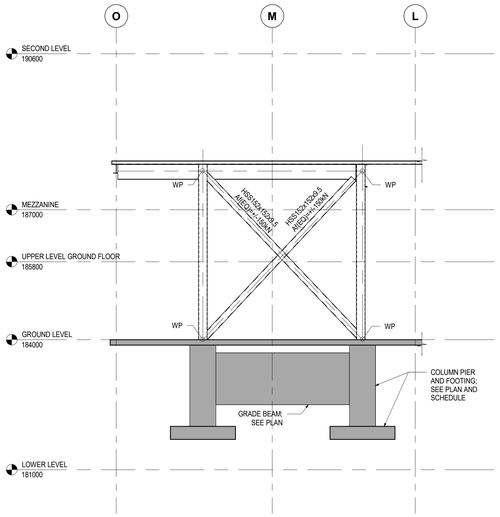

1
S406

VB1 ELEVATION

1 : 50

STEEL ELEVATION NOTES

1. SEE DRAWING S100 SERIES FOR GENERAL NOTES AND TYPICAL DETAILS.
2. SEE DRAWING S300 SERIES FOR SCHEDULES
3. PROVIDE MIN. 6 CAP PLATE AND END PLATE TO THE ENDS OF ALL HSS COLUMNS.

Key to Detail Location

NO. — Detail Number
NO. — Drawing Number

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Fax. 905 470 0030
WWW.LEA.CA

THE STERLING HALL SCHOOL

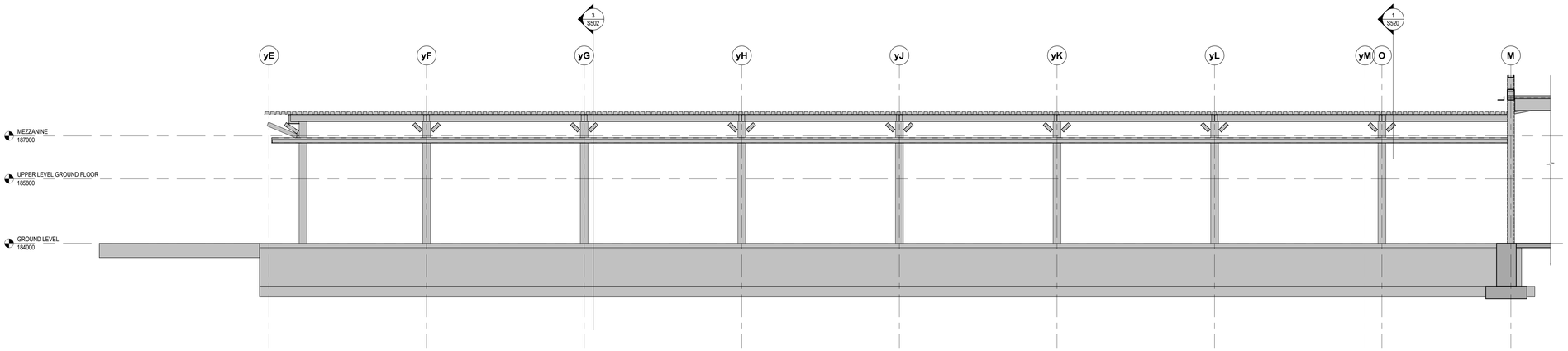
99 Cartwright Avenue, Toronto, ON M6A 1V4

FRAMING ELEVATIONS

scale:	1 : 50
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S406
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-03 3:05:13 PM



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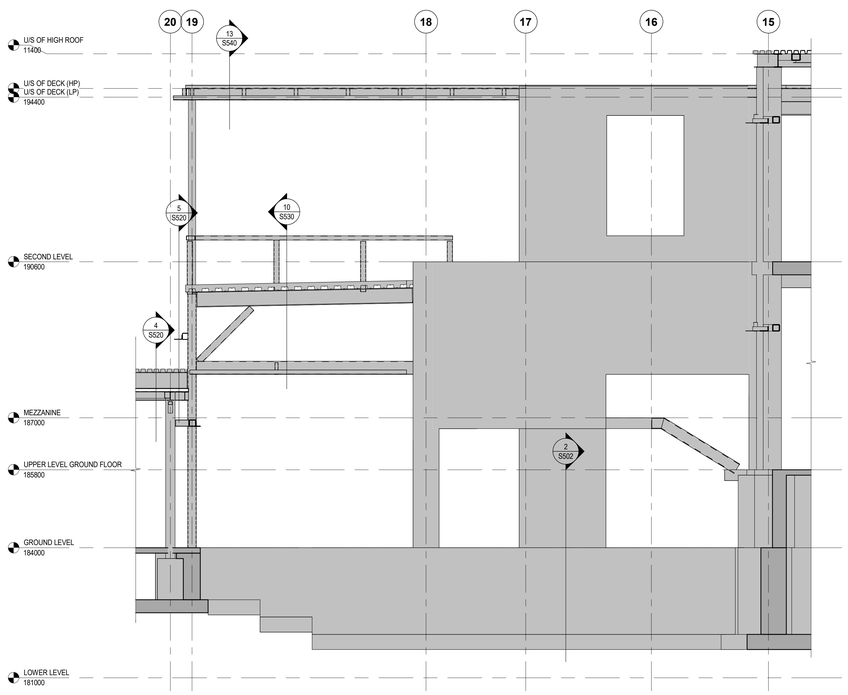
1 : 50



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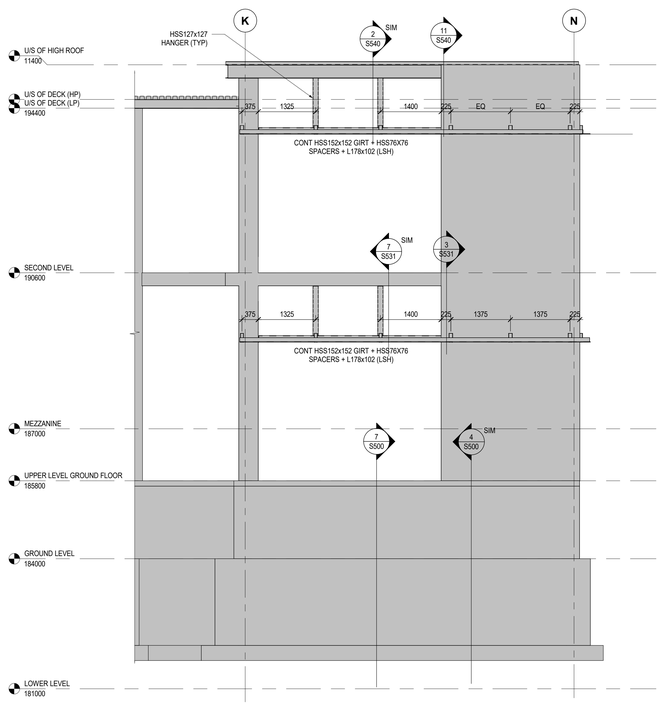

3 S410 ELEVATION
1 : 50

Key to Detail Location

NO. / NO. — Detail Number / Drawing Number

If this sheet is not 46 7/8" x 33 1/8" (1189 x 841 mm) It is a reduced print - Read dwg. accordingly.

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No	Date	Revision/Issued
1	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
2	2026-07-08	ISSUED FOR TENDER

consultants

architect	CS&P Architects Inc. 2345 Yonge St., Suite 200 Toronto, ON, M4P 2E5 Phone.416.482.5002 Fax.416.482.5040
civil engineer	SCS Consulting Group Ltd. 30 Centurian Drive, Suite 100 Markham, ON, L3R 8B8 Phone.905.475.1900 Fax.905.475.8335
structural engineer	LEA Consulting Ltd. 40 University Ave, Suite 503 Toronto, ON, M5J 1T1 Phone.905.470.0015
mechanical engineer	Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151
electrical engineer	Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151
landscape architect	Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

REGISTERED PROFESSIONAL ENGINEER
2026-07-08
E.U. CHOWDHURY
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THE STERLING HALL SCHOOL

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BUILDING ELEVATIONS

scale:	1 : 50
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S410
	Revision: 2

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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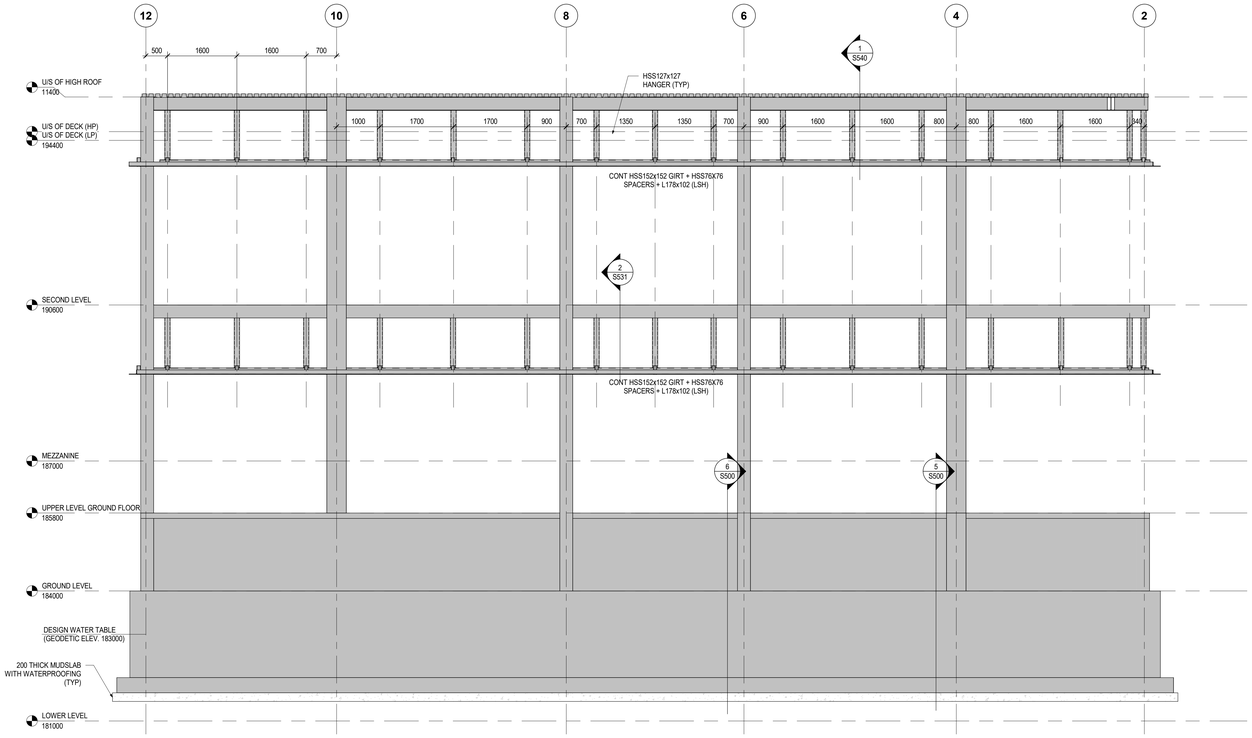

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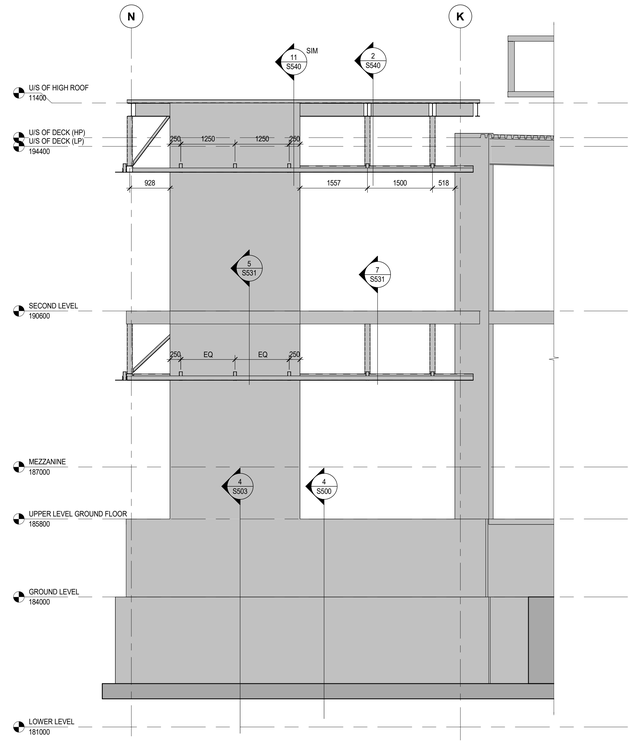

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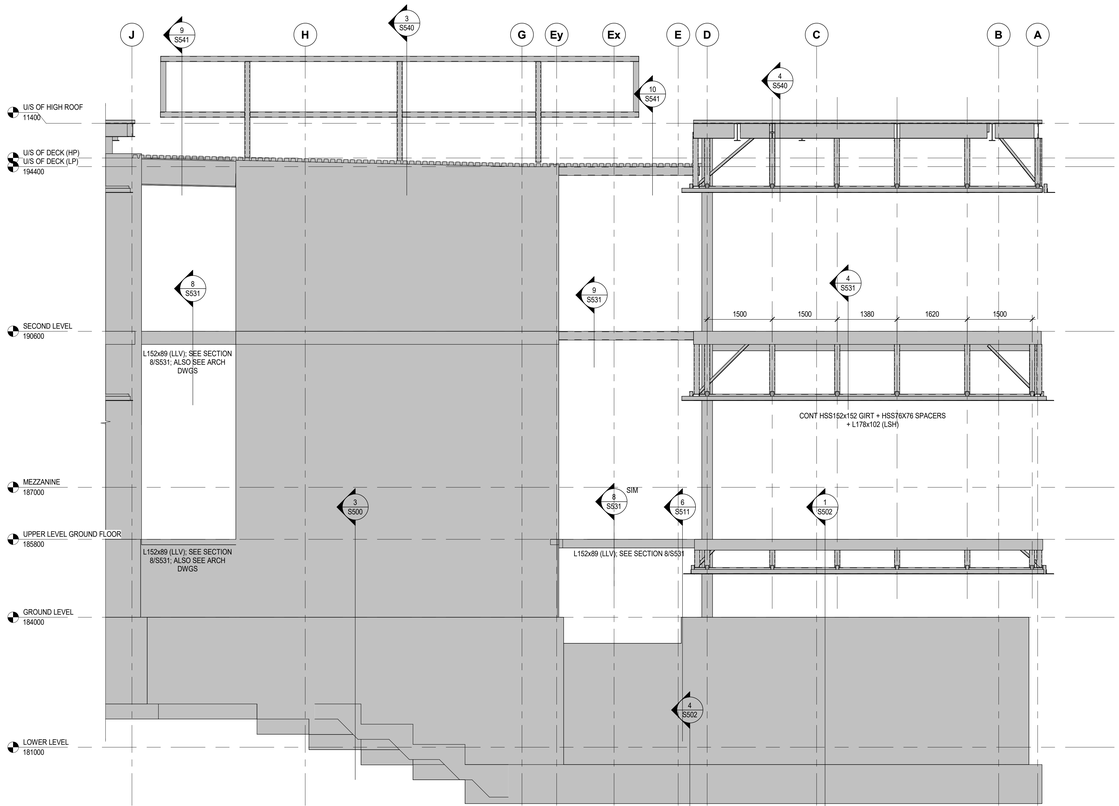

3 S411 ELEVATION
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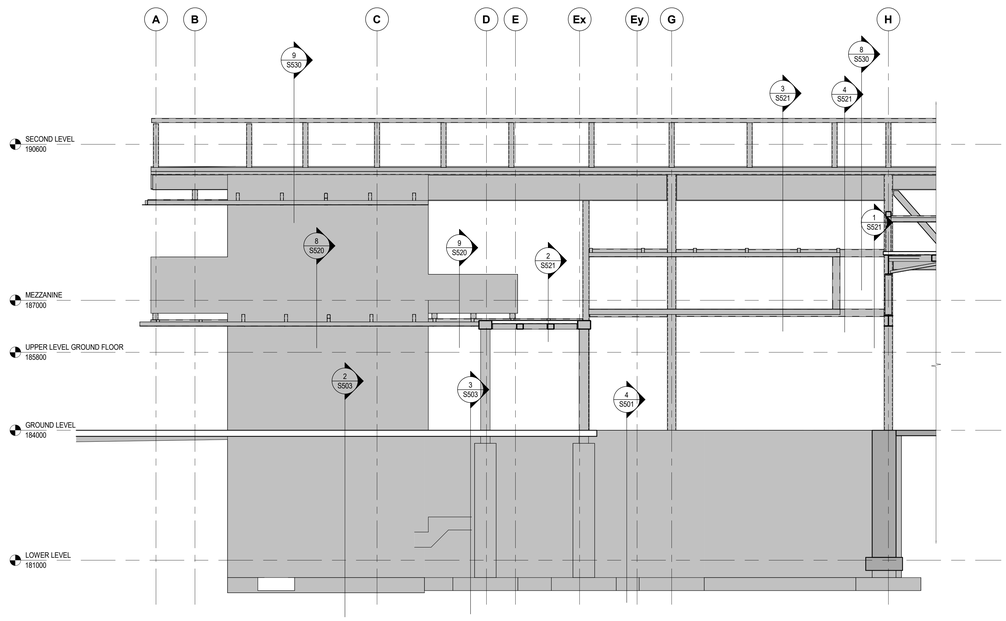

4 S411 ELEVATION
1 : 50

Key to Detail Location

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NO. — Drawing Number

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1	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
2	2026-07-08	ISSUED FOR TENDER

consultants

architect — CS&P Architects Inc.
2345 Yonge St., Suite 200
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structural engineer — LEA Consulting Ltd.
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mechanical engineer — Smith + Andersen
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electrical engineer — Smith + Andersen
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Phone.416.487.8151

landscape architect — Quinn Design Associates Inc.
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REGISTERED PROFESSIONAL ENGINEER
2026-07-08
E.U. CHOWDHURY
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THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

BUILDING ELEVATIONS

scale:	1 : 50
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S411
	Revision: 2

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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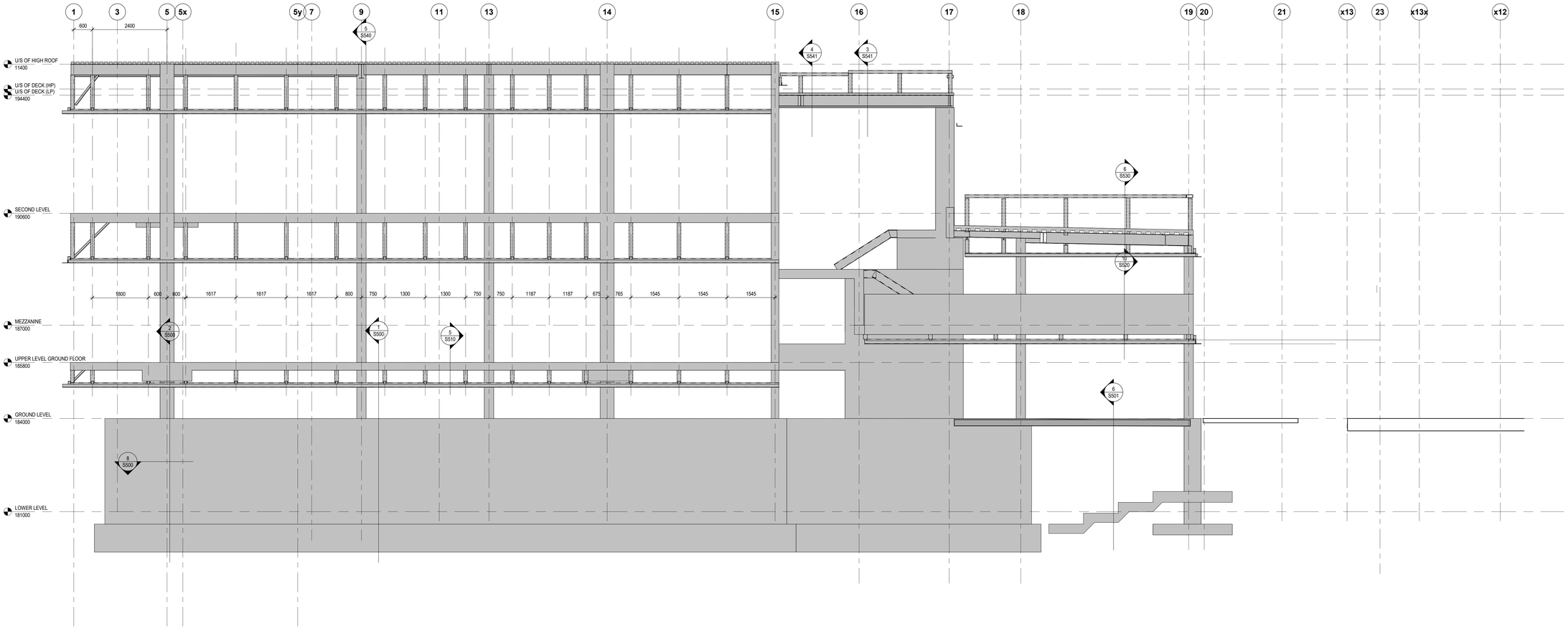

1 S412 ELEVATION
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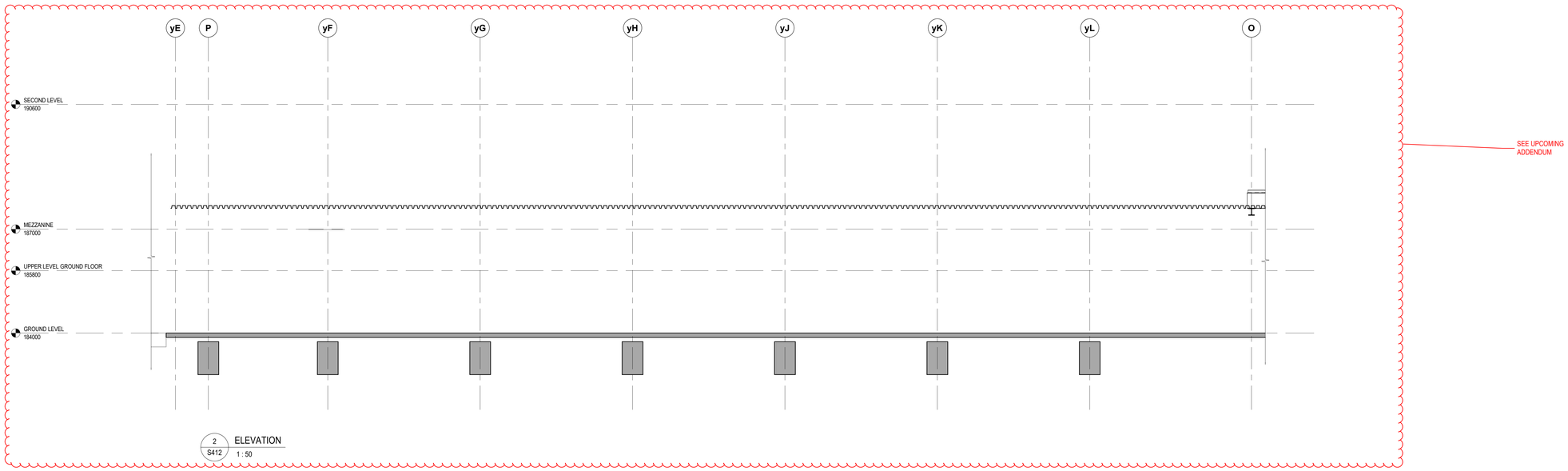

2 S412 ELEVATION
1 : 50

FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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landscape architect — Quinn Design Associates Inc.
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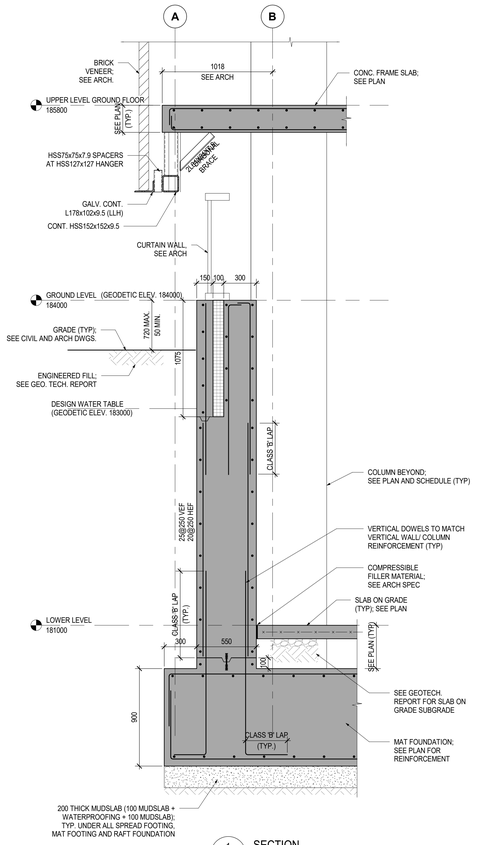
THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

BUILDING ELEVATIONS

scale:	1 : 50
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S412
	Revision: 2



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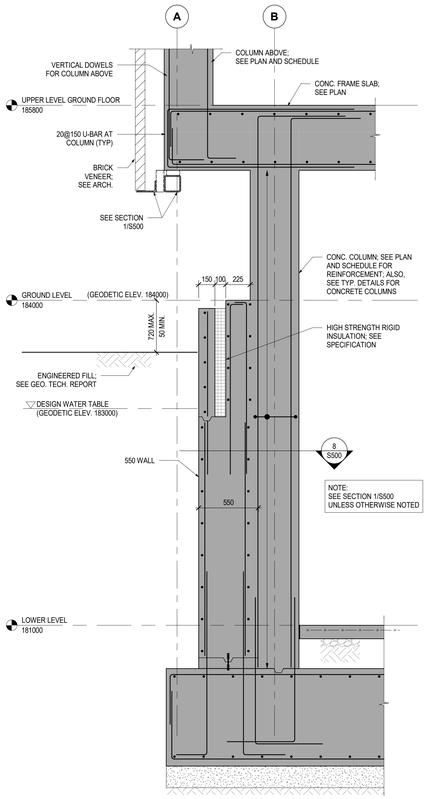

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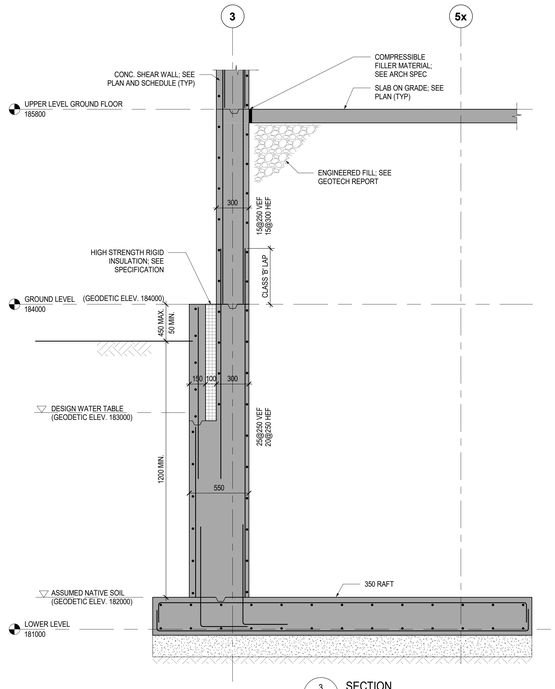

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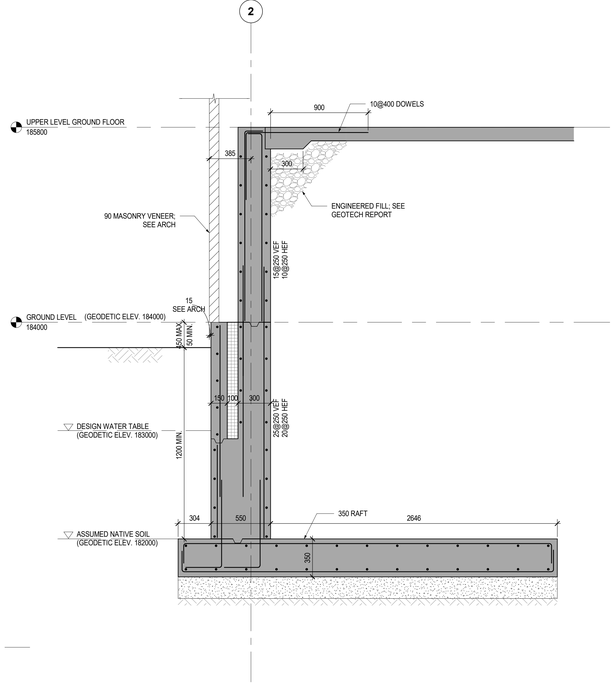

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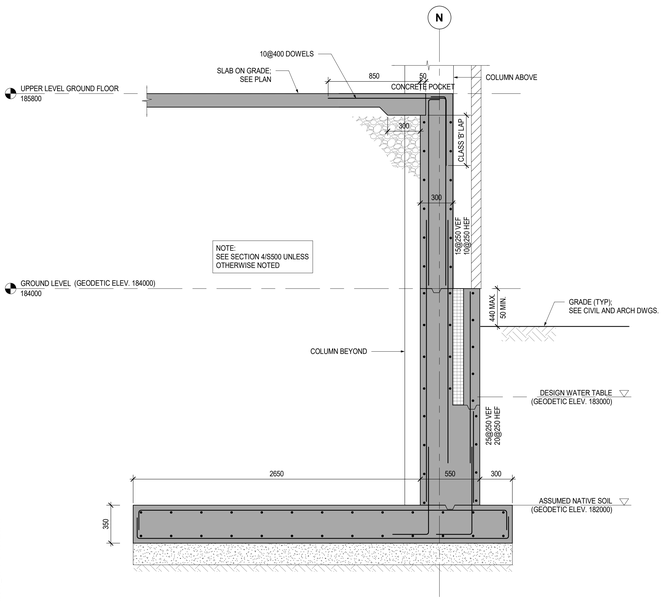

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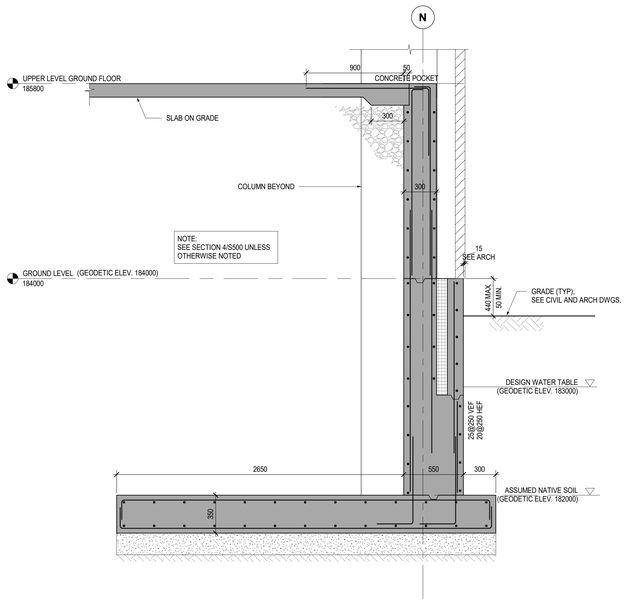

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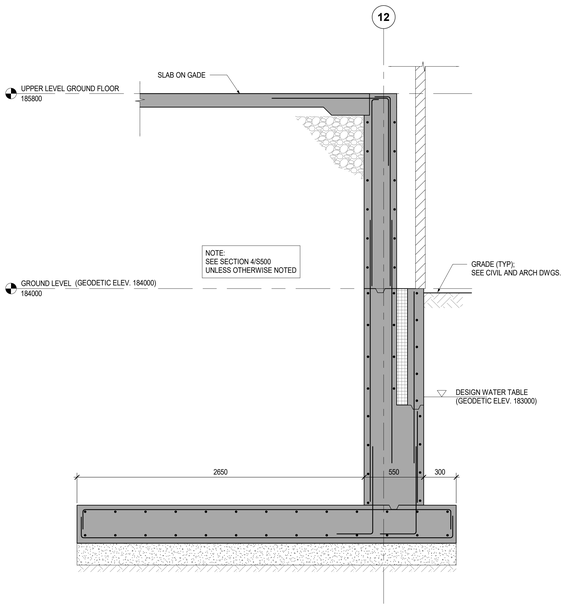

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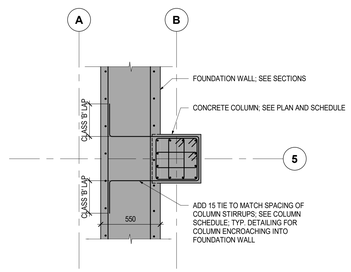

8 S500 PLAN DETAIL 1 : 20

Key to Detail Location

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1	2026-03-13	ISSUED FOR BUILDING PERMIT
2	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
3	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
4	2026-07-08	ISSUED FOR TENDER

consultants

architect — CS&P Architects Inc. 2345 Yonge St., Suite 200 Toronto, ON, M4P 2E5 Phone.416.482.5002 Fax.416.482.5040

civil engineer — SCS Consulting Group Ltd. 30 Centurian Drive, Suite 100 Markham, ON, L3R 8B8 Phone.905.475.1900 Fax.905.475.8335

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electrical engineer — Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151

landscape architect — Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

REGISTERED PROFESSIONAL ENGINEER 2026-07-08 ELIZABETH CHUNG-R 100072622 PROVINCE OF ONTARIO

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THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

FOUNDATION SECTIONS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S500
Revision:	4

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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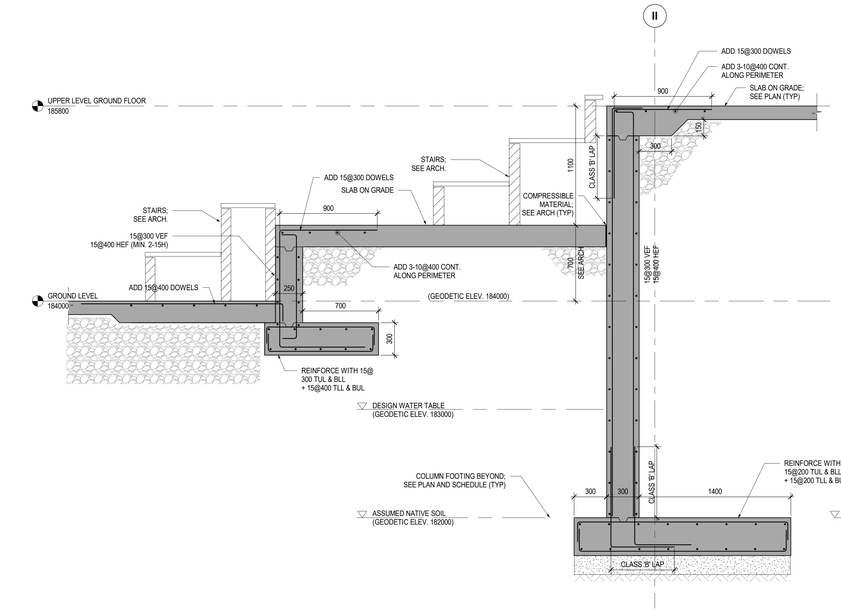

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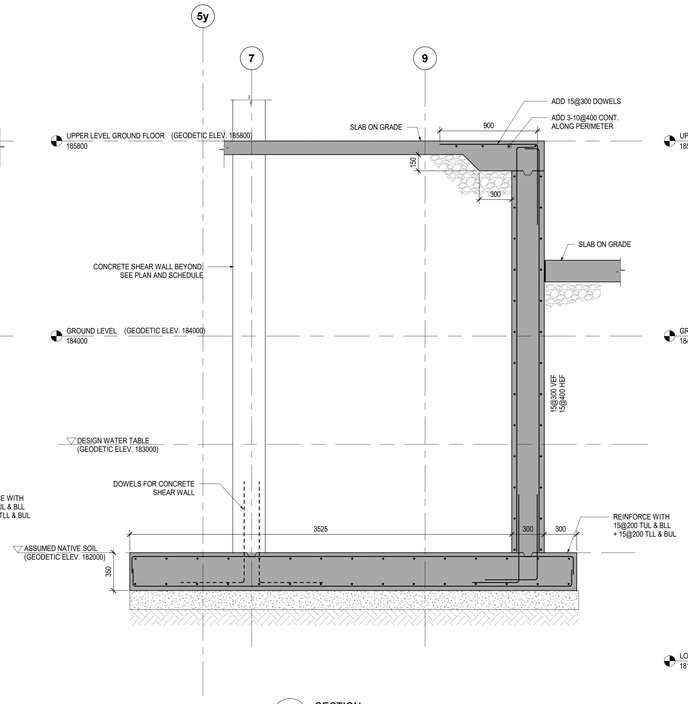

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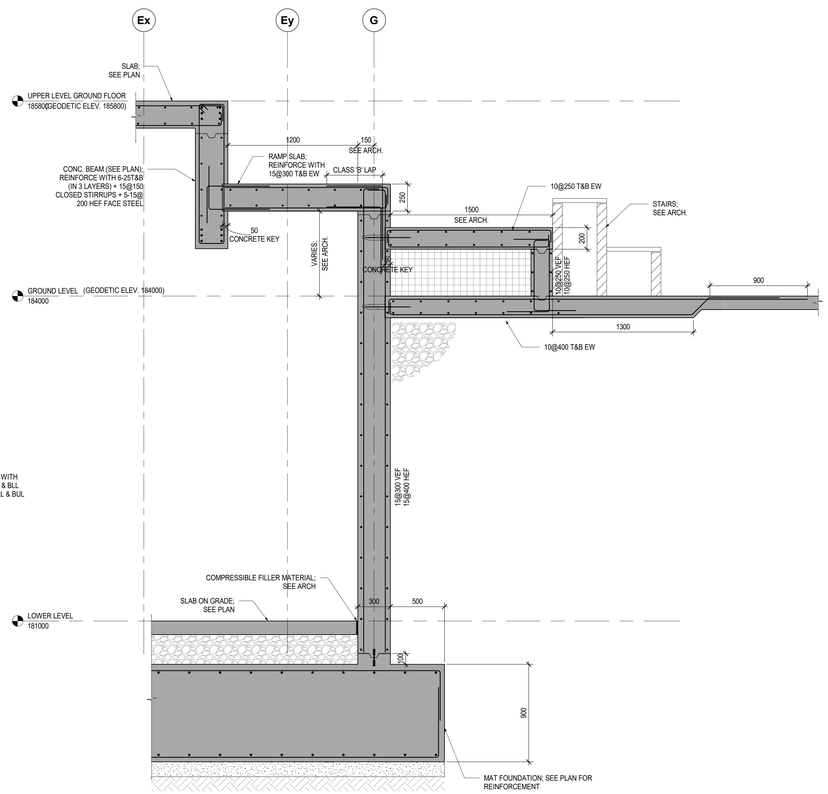

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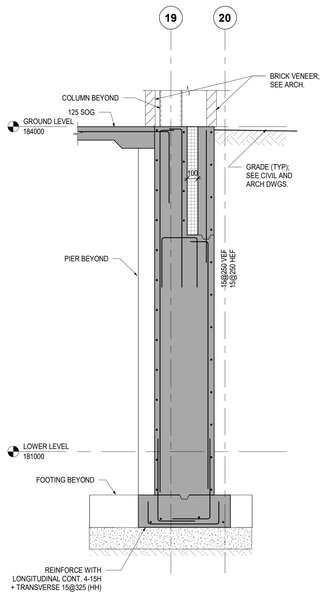

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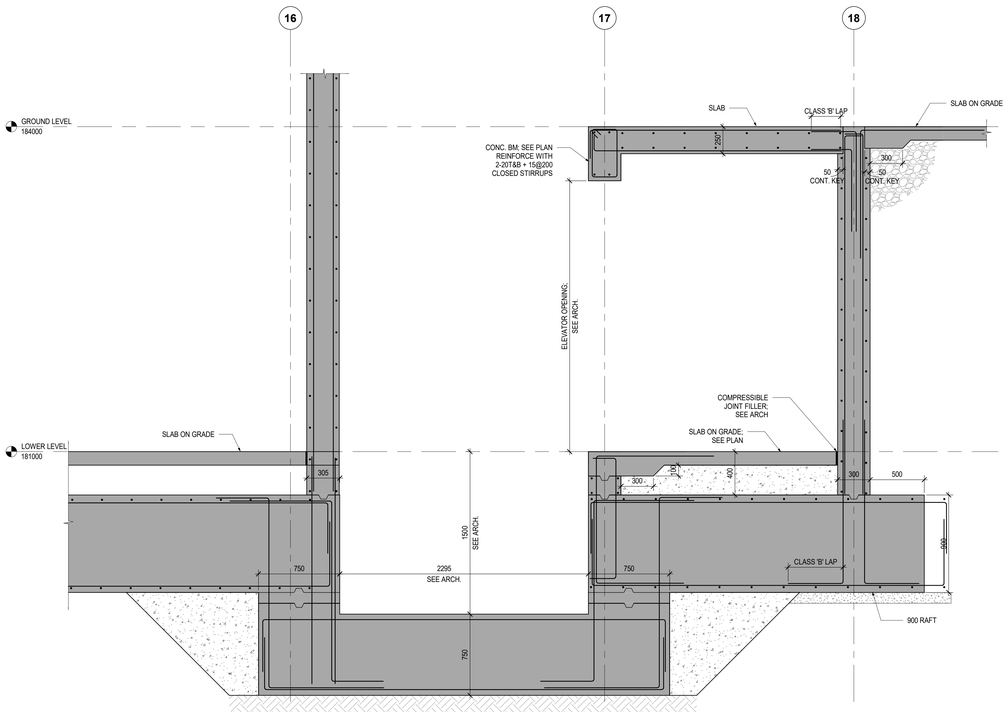

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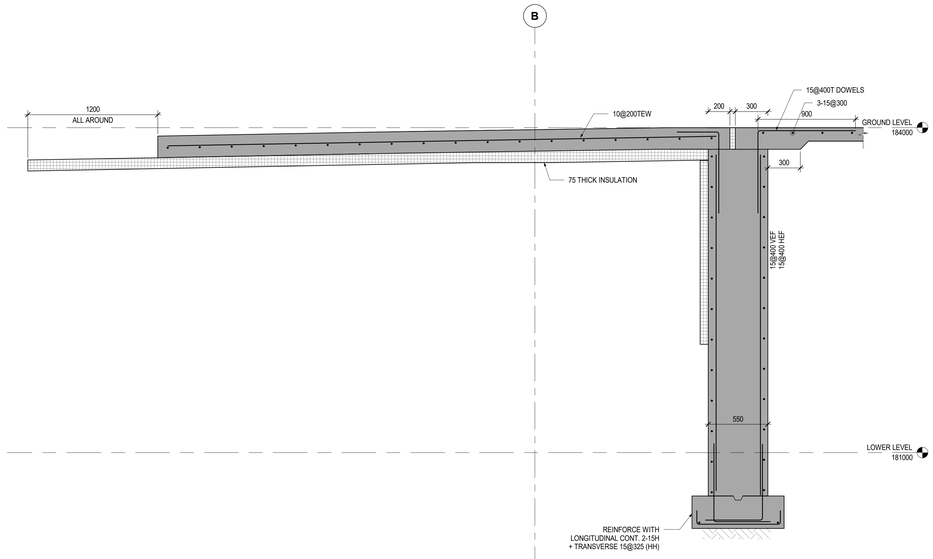

6 SECTION
S501 1 : 20

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REGISTERED PROFESSIONAL ENGINEER
2026-07-08
E.U.C. HOWDHURY
100072622
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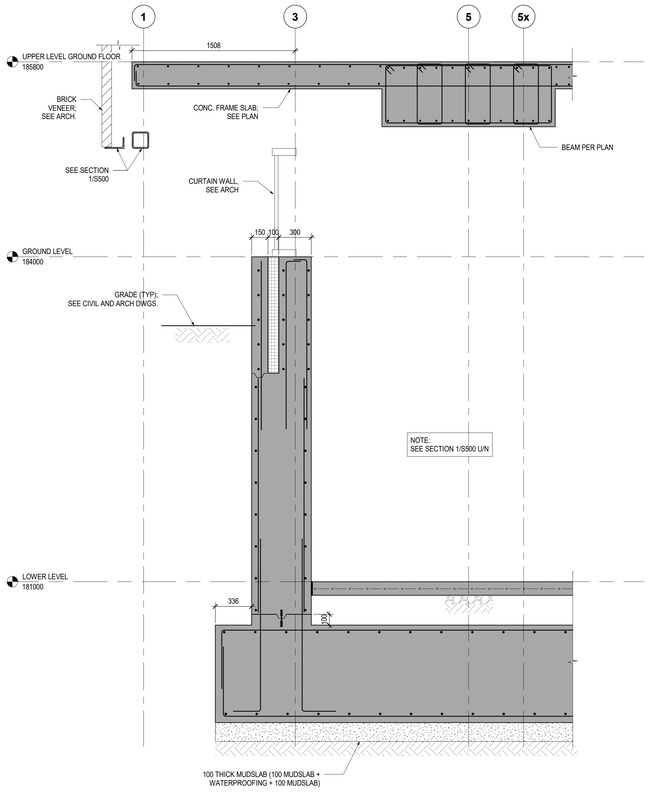

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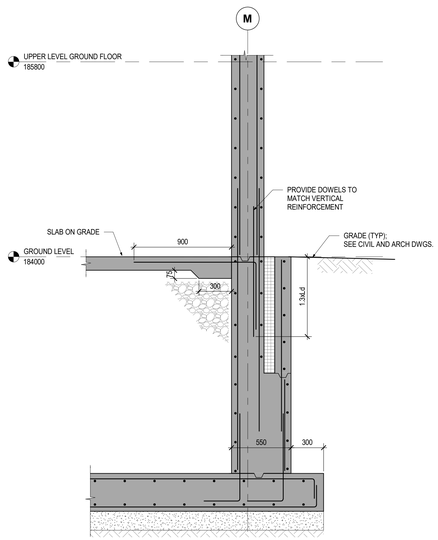
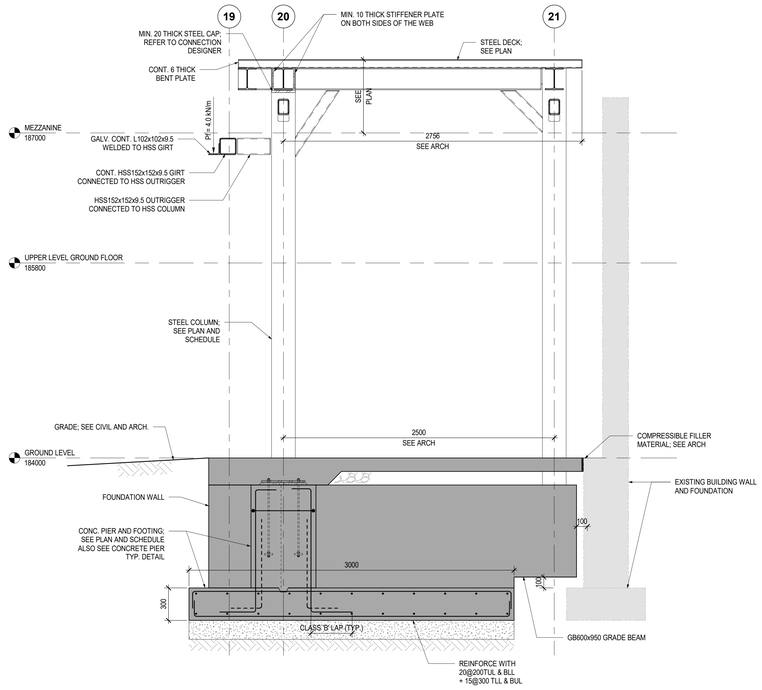
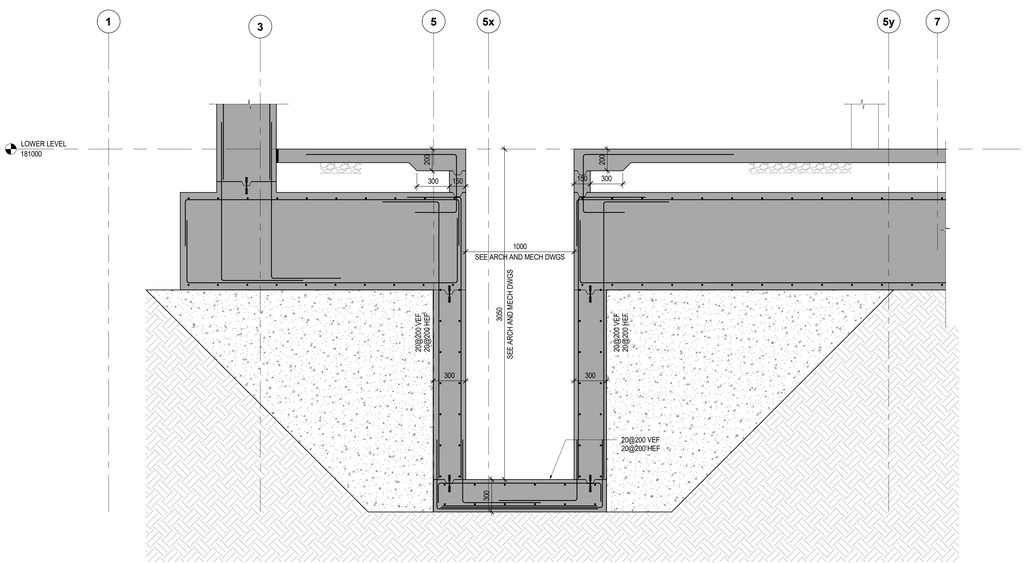


2 S502 SECTION
1 : 20



3 S502 SECTION
1 : 20



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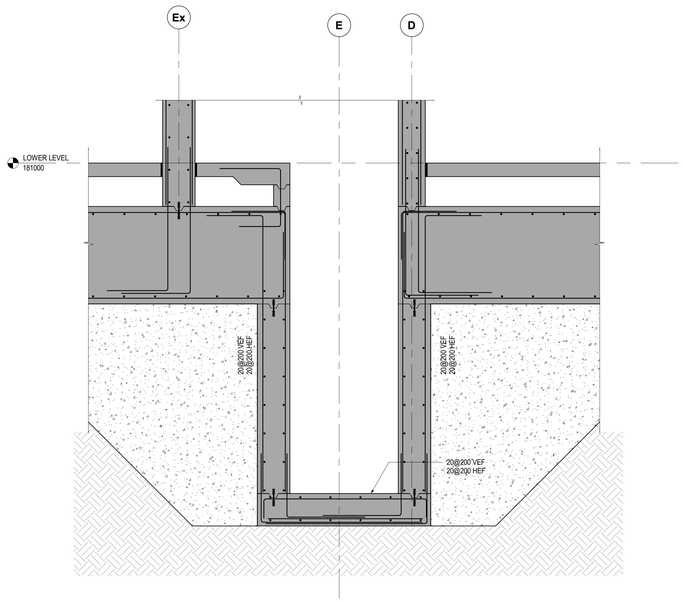

5 S502 SECTION
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4	2026-07-08	ISSUED FOR TENDER

consultants

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2345 Yonge St., Suite 200
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Fax.416.482.5040

civil engineer — SCS Consulting Group Ltd.
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structural engineer — LEA Consulting Ltd.
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mechanical engineer — Smith + Andersen
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landscape architect — Quinn Design Associates Inc.
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Phone.416.962.8700

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2026-07-08
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Tel. 905 470 0015
Fax. 905 470 0030
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THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

FOUNDATION SECTIONS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S502
Revision:	4

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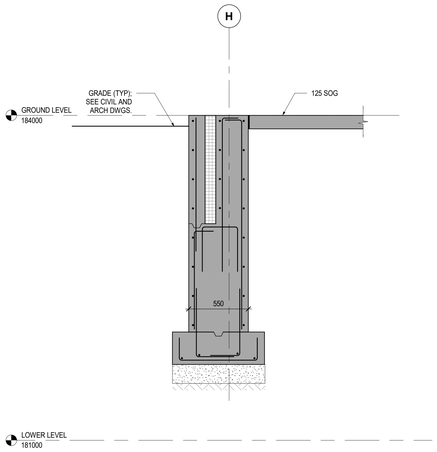

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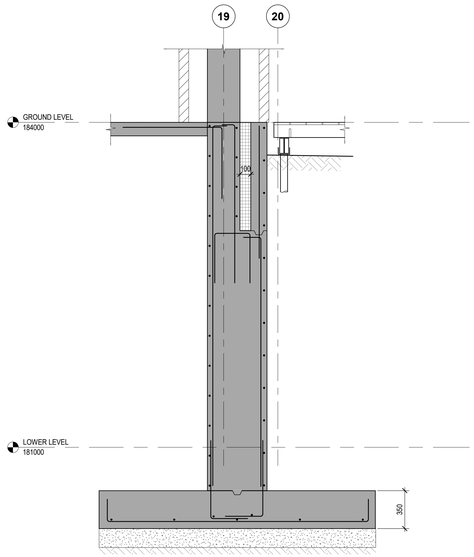

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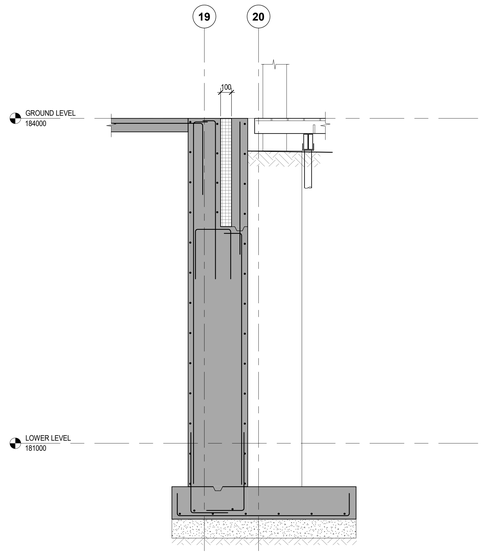

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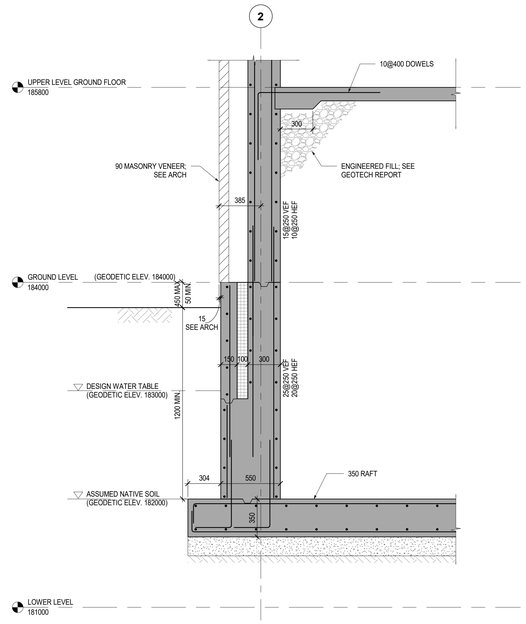

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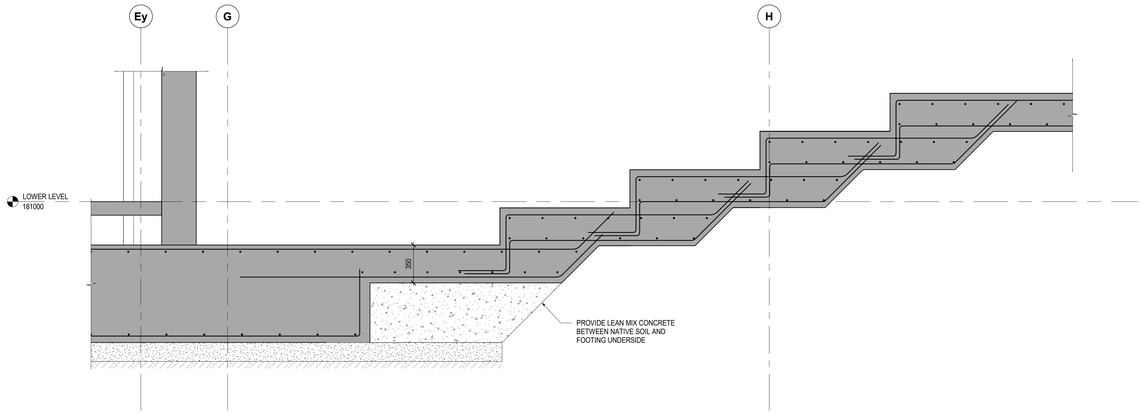

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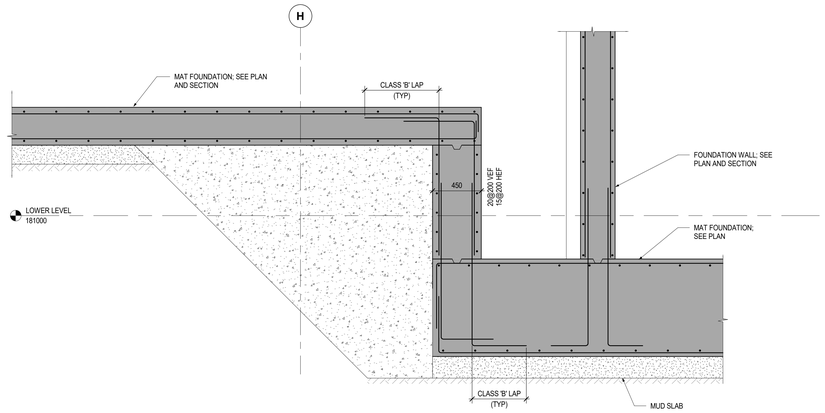

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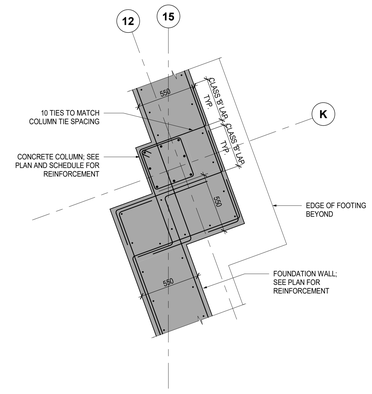
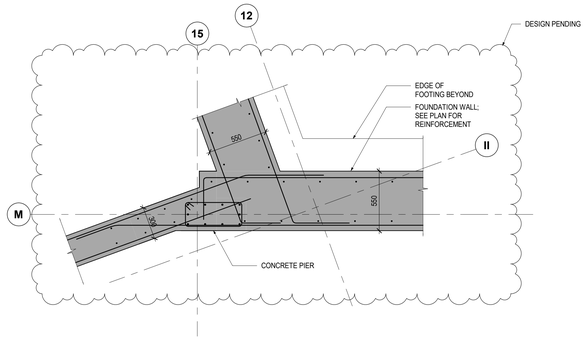


7 / S503 PLAN DETAIL 1 : 20



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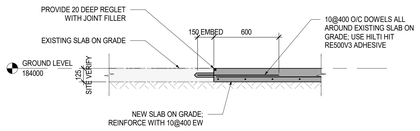

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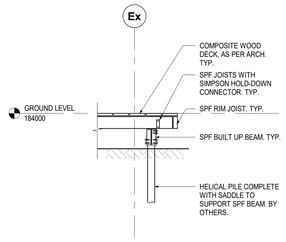

10 / S503 SECTION 1 : 20

Key to Detail Location

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1	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
2	2026-07-08	ISSUED FOR TENDER

consultants

architect — CS&P Architects Inc. 2345 Yonge St., Suite 200 Toronto, ON, M4P 2E5 Phone.416.482.5002 Fax.416.482.5040

civil engineer — SCS Consulting Group Ltd. 30 Centurian Drive, Suite 100 Markham, ON, L3R 8B8 Phone.905.475.1900 Fax.905.475.8335

structural engineer — LEA Consulting Ltd. 40 University Ave, Suite 503 Toronto, ON, M5J 1T1 Phone.905.470.0015

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REGISTERED PROFESSIONAL ENGINEER 2026-07-08 E.H. CHOWDHURY 100120 PROVINCE OF ONTARIO

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THE STERLING HALL SCHOOL

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FOUNDATION SECTIONS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S503
	Revision: 2

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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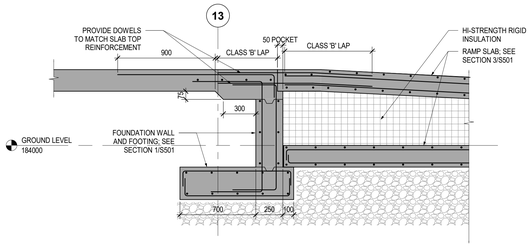
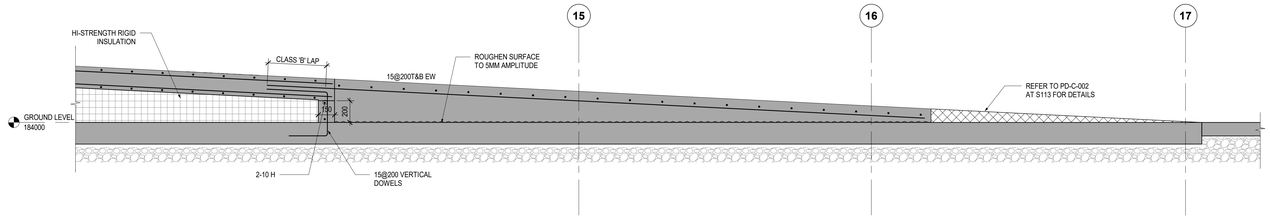


1 / S510 SECTION 1 : 20



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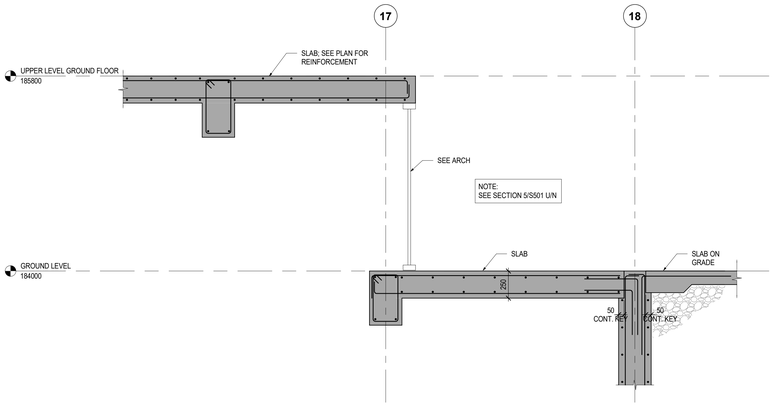

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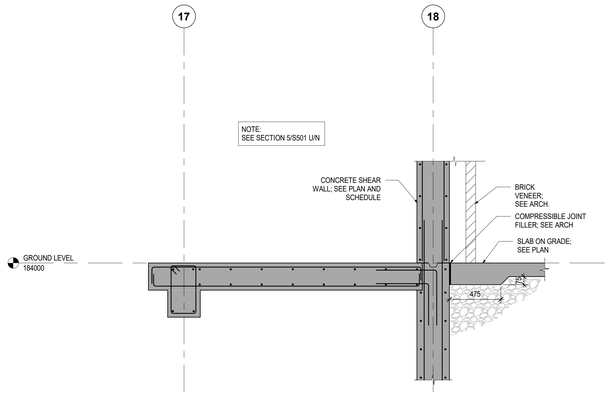

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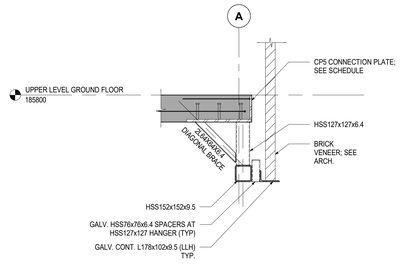

5 / S510 SECTION 1 : 20

Key to Detail Location

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1	2026-03-13	ISSUED FOR BUILDING PERMIT
2	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
3	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
4	2026-07-08	ISSUED FOR TENDER

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landscape architect — Quinn Design Associates Inc.
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THE STERLING HALL SCHOOL

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GROUND FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S510
Revision:	4

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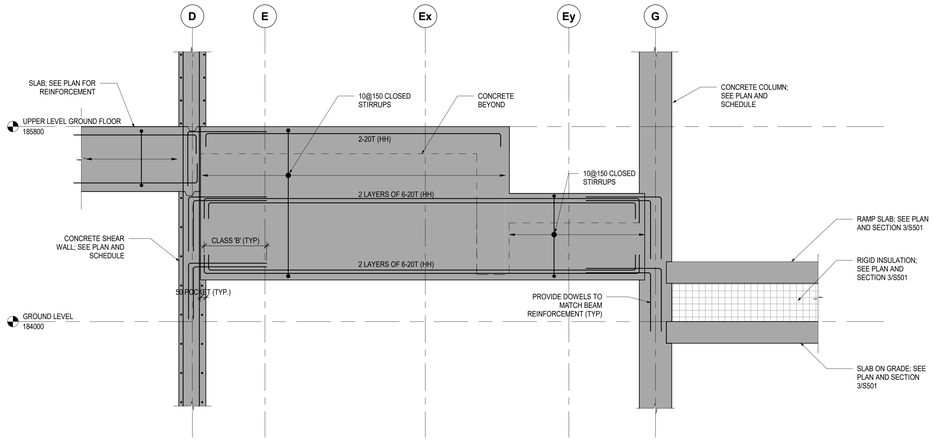

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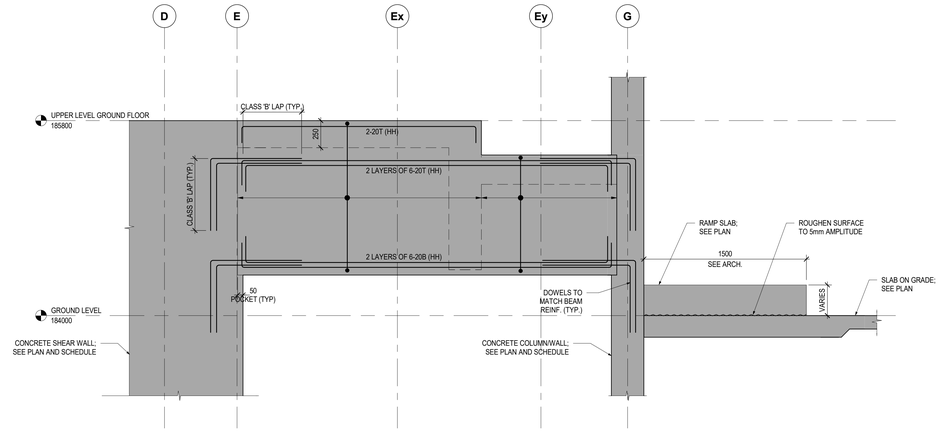

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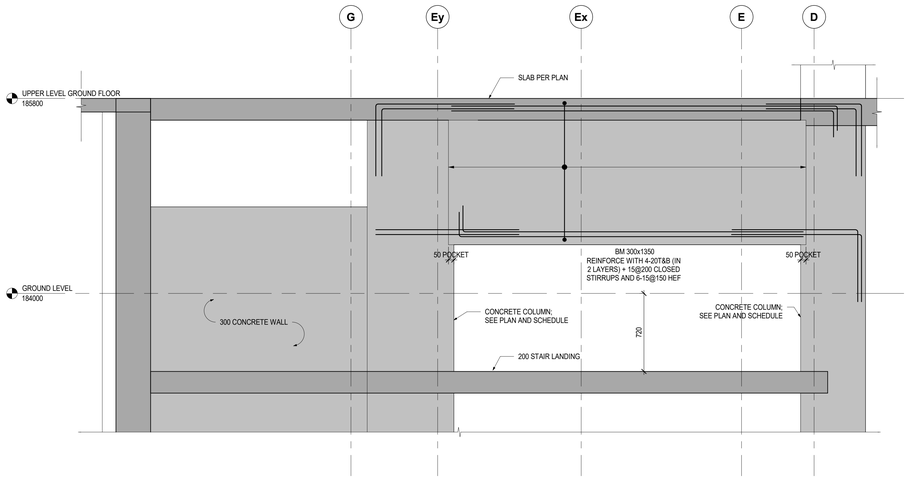
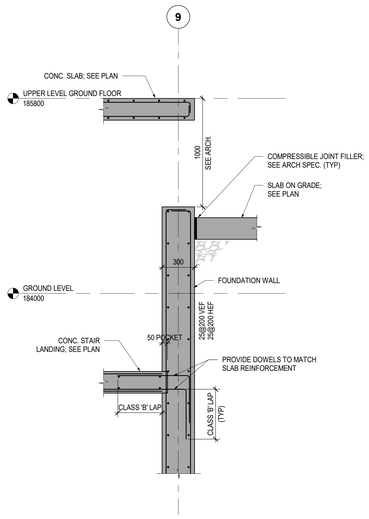
1 : 20



3 / S511 SECTION
1 : 20



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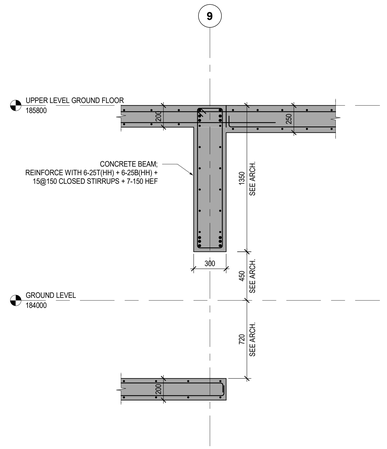
1 : 20



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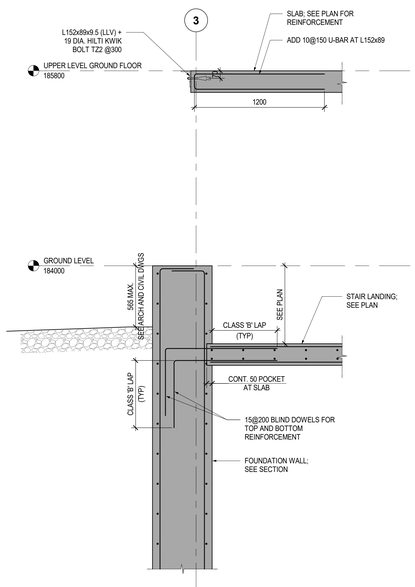

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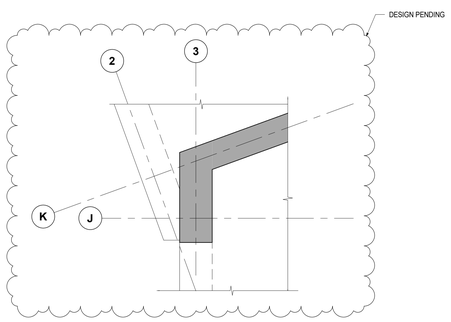

7 / S511 PLAN DETAIL
1 : 20

Key to Detail Location

NO. / NO. — Detail Number / Drawing Number

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REGISTERED PROFESSIONAL ENGINEER
2026-07-08
E.U.I.CHOWDHURY
100072522
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THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

GROUND FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S511
	Revision: 4

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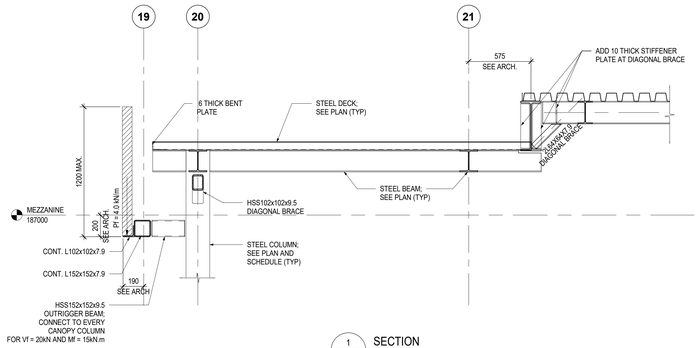

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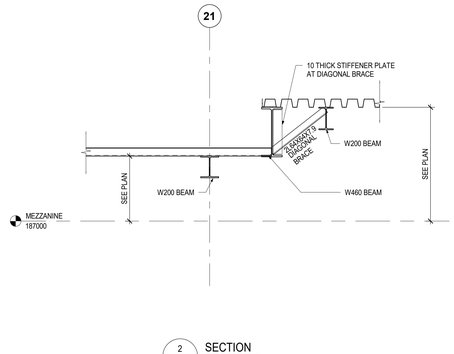

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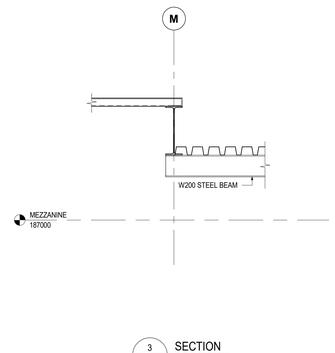

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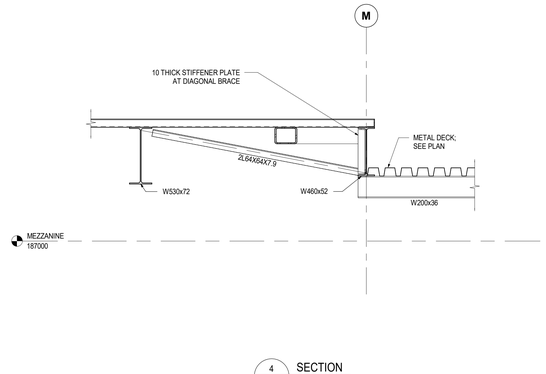

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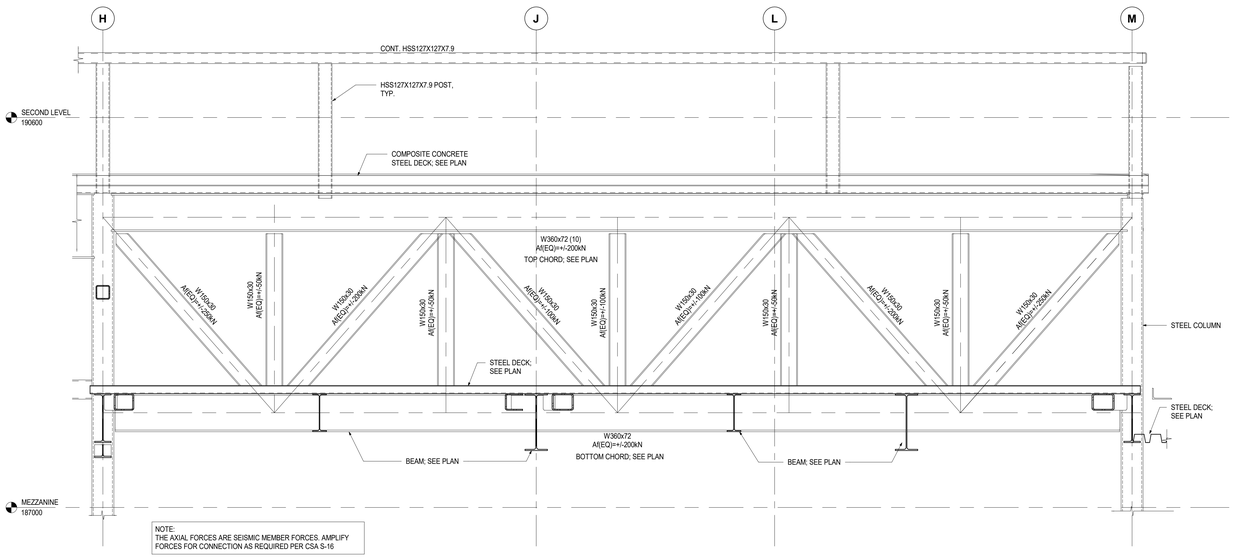

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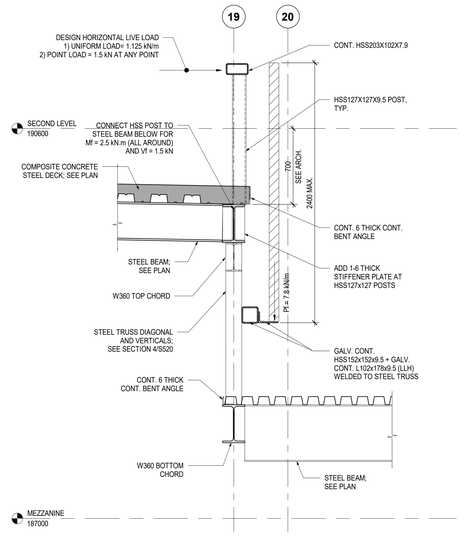

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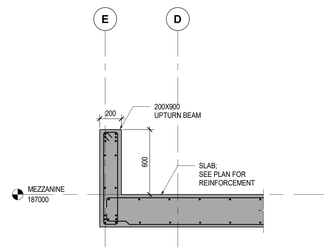

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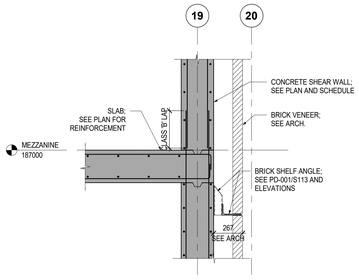

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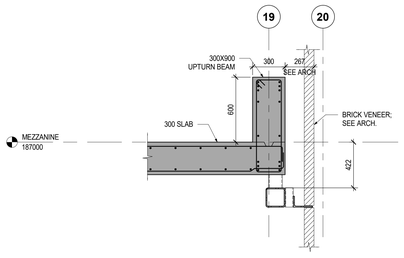

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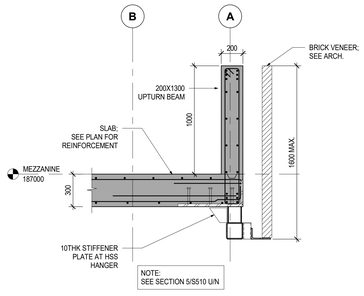

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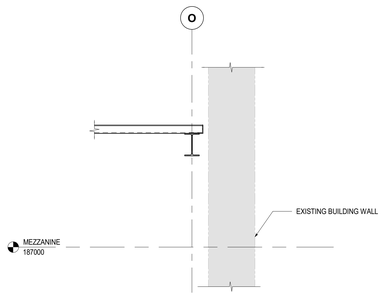

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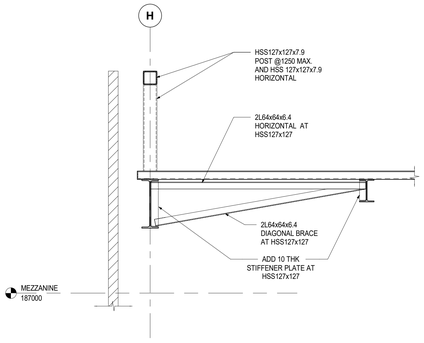

12 / S520 SECTION 1 : 20

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mechanical engineer — Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151

electrical engineer — Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151

landscape architect — Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

REGISTERED PROFESSIONAL ENGINEER 2026-07-08 E.H.J. CHOWDHURY 100125421 PROVINCE OF ONTARIO

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THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

MEZZANINE FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S520
	Revision: 2

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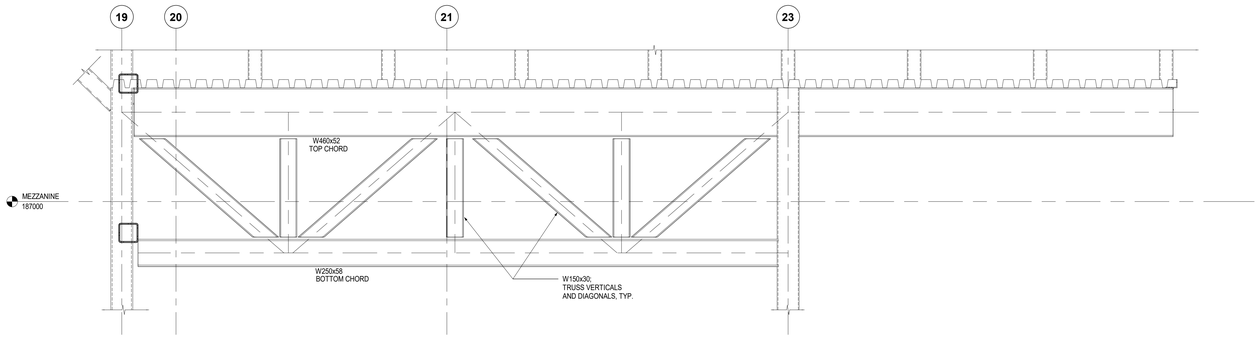

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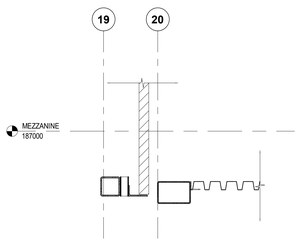

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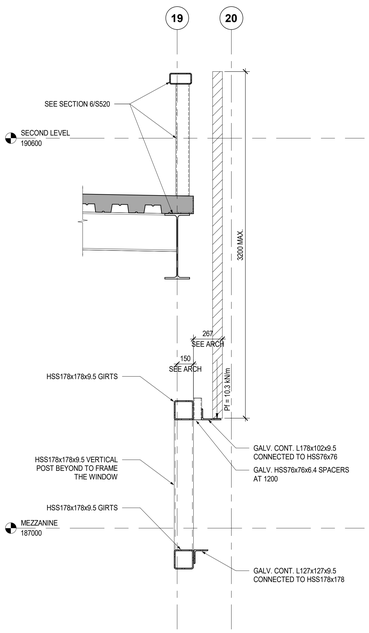

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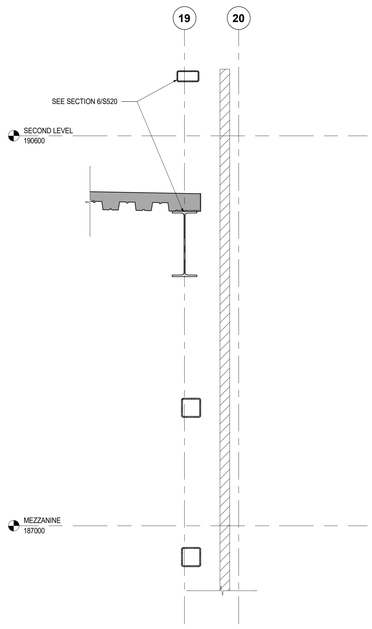

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Key to Detail Location

NO. / NO. — Detail Number / Drawing Number

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No	Date	Revision/Issued
1	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
2	2026-07-08	ISSUED FOR TENDER

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landscape architect — Quinn Design Associates Inc.
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2026-07-08
E.U.C. CHOWDHURY
100072422
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THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

MEZZANINE FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S521
	Revision: 2

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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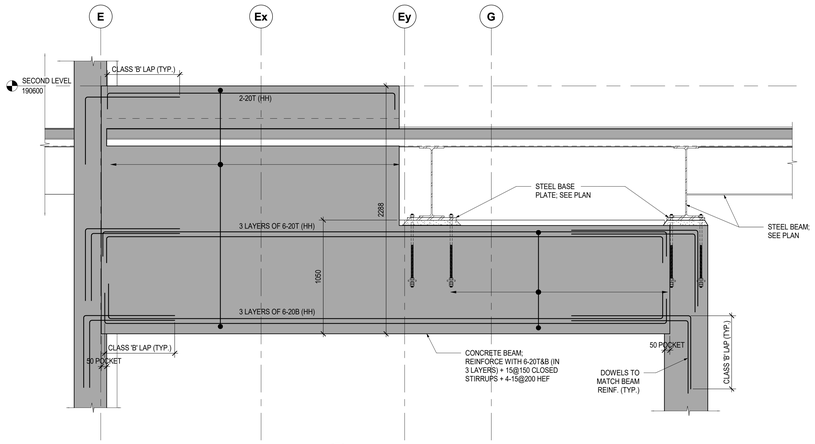

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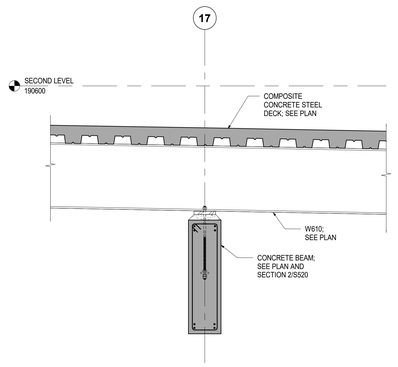

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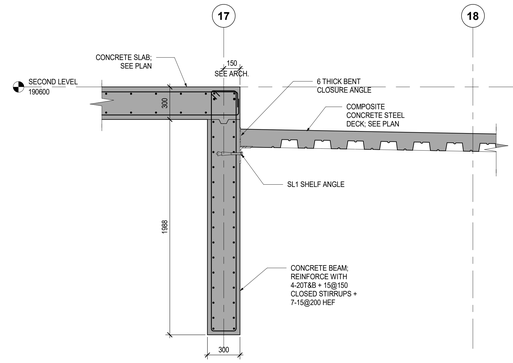

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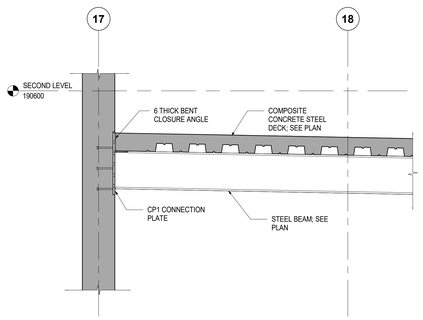

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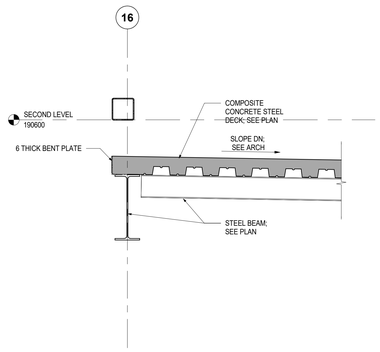

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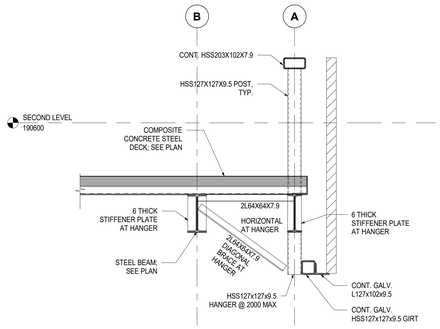

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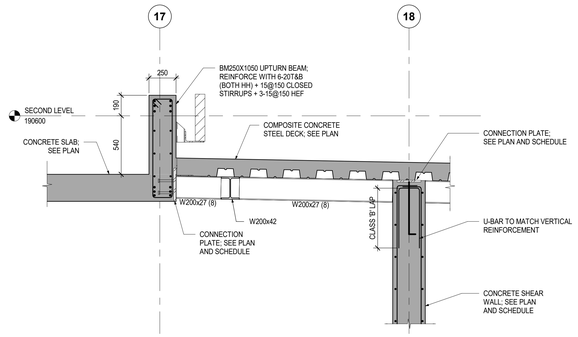

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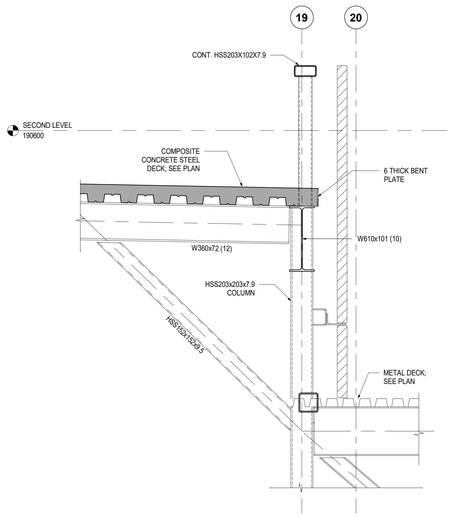

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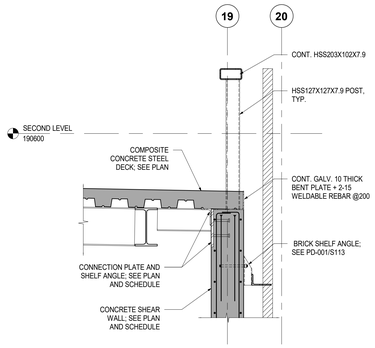

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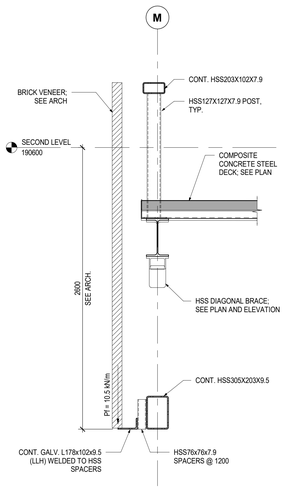

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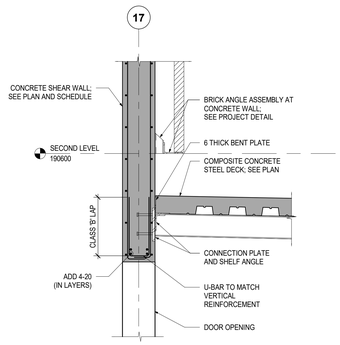

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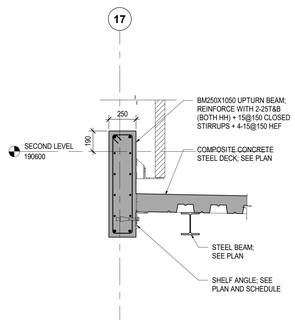

12 / S530 SECTION 1 : 20

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No	Date	Revision/Issued
1	2026-03-13	ISSUED FOR BUILDING PERMIT
2	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
3	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
4	2026-07-08	ISSUED FOR TENDER

consultants

architect — CS&P Architects Inc. 2345 Yonge St., Suite 200 Toronto, ON, M4P 2E5 Phone.416.482.5002 Fax.416.482.5040

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landscape architect — Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

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ELIZ CHOMEJKO
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THE STERLING HALL SCHOOL

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SECOND FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S530
Revision:	4

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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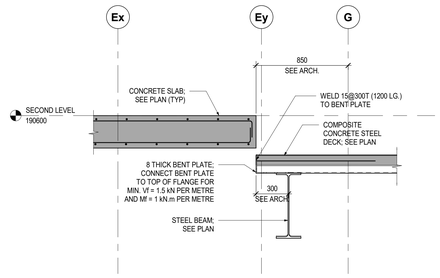

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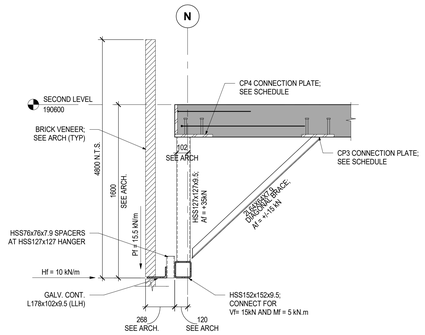

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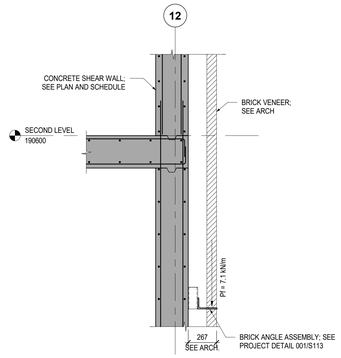

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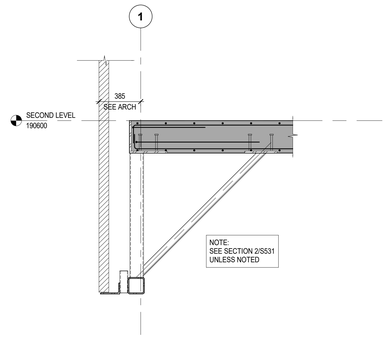

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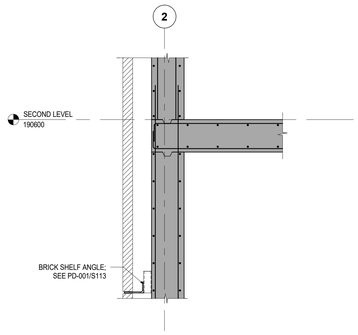

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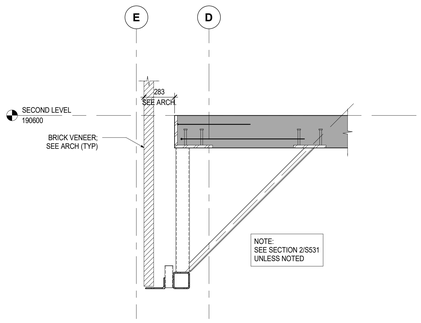

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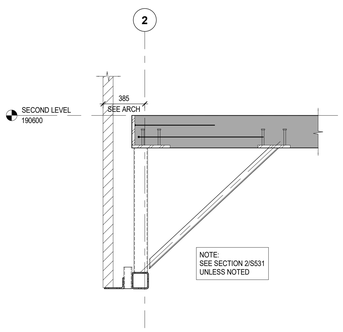

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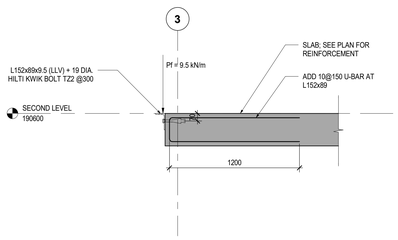

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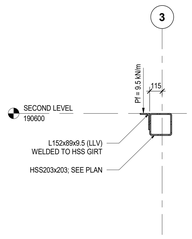

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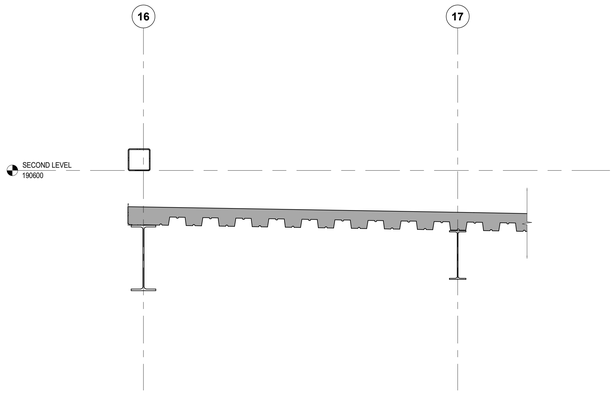

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consultants

architect	CS&P Architects Inc. 2345 Yonge St., Suite 200 Toronto, ON, M4P 2E5 Phone.416.482.5002 Fax.416.482.5040
civil engineer	SCS Consulting Group Ltd. 30 Centurian Drive, Suite 100 Markham, ON, L3R 8B8 Phone.905.475.1900 Fax.905.475.8335
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electrical engineer	Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151
landscape architect	Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

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100071420
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THE STERLING HALL SCHOOL

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SECOND FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S531
	Revision: 2

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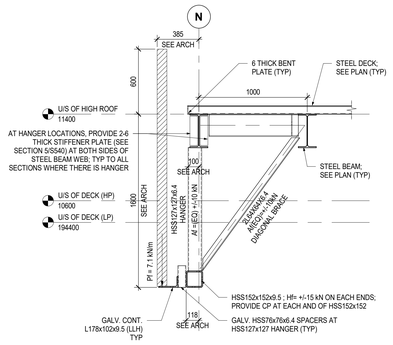

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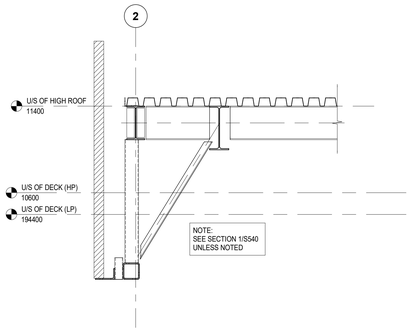

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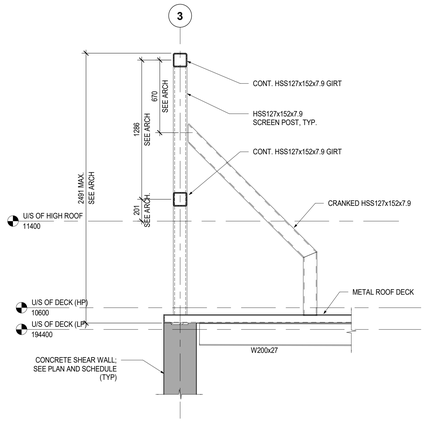

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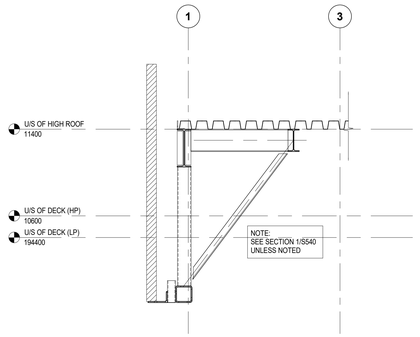

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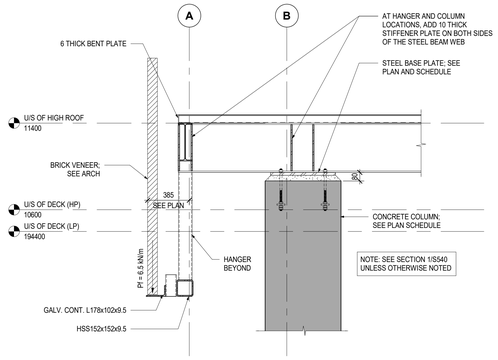

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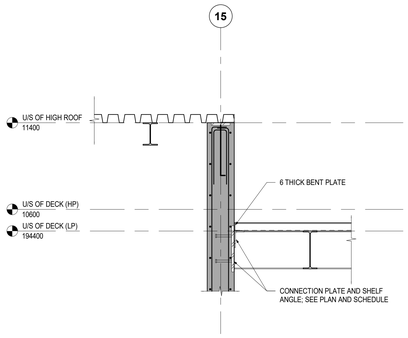

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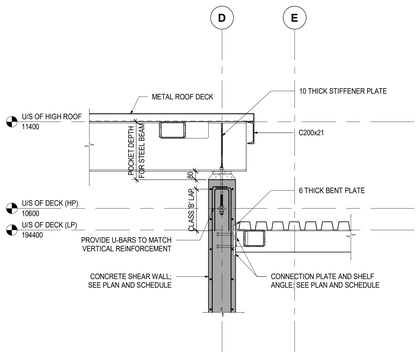

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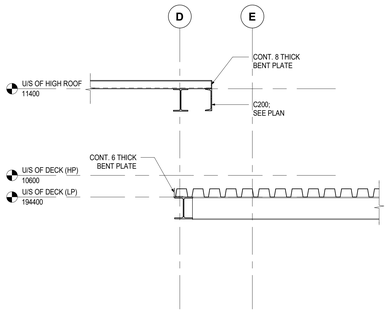

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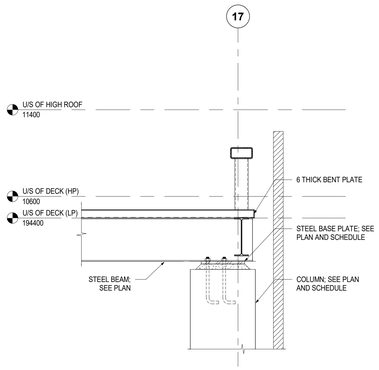

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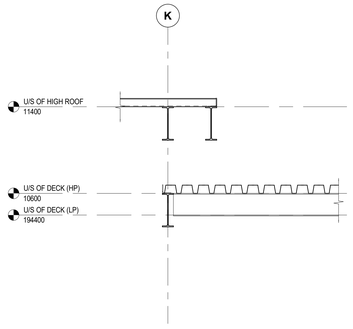

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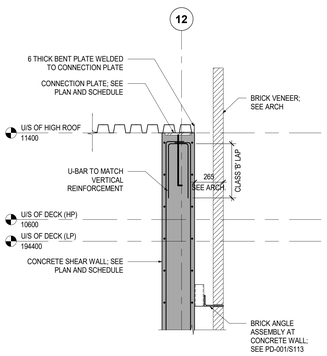

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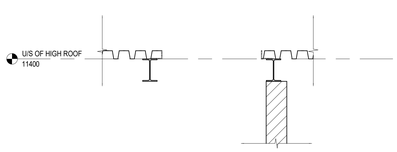

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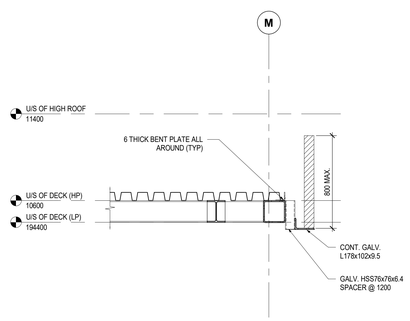

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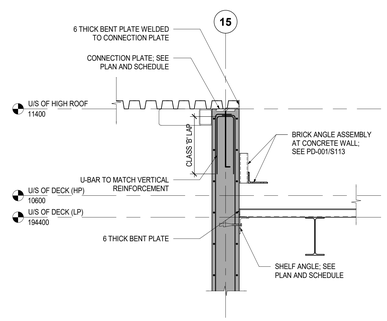

14 S540 SECTION 1 : 20

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2	2026-07-08	ISSUED FOR TENDER

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architect — CS&P Architects Inc. 2345 Yonge St., Suite 200 Toronto, ON, M4P 2E5 Phone.416.482.5002 Fax.416.482.5040

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mechanical engineer — Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151

electrical engineer — Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151

landscape architect — Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

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THE STERLING HALL SCHOOL

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ROOF FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S540
	Revision: 2

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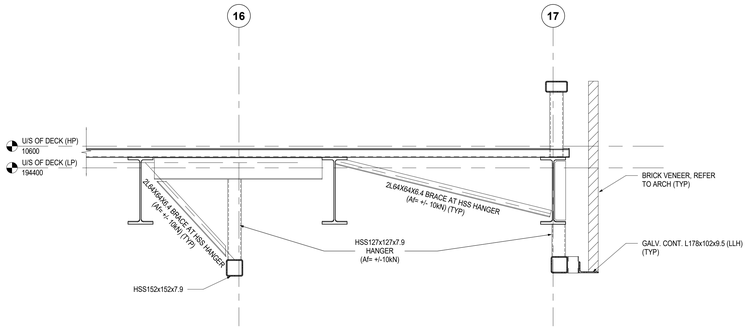

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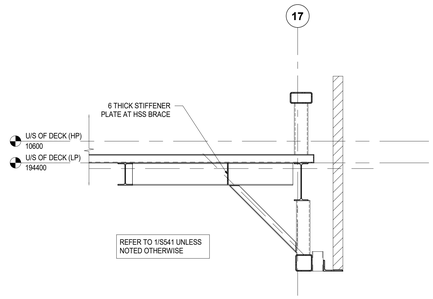

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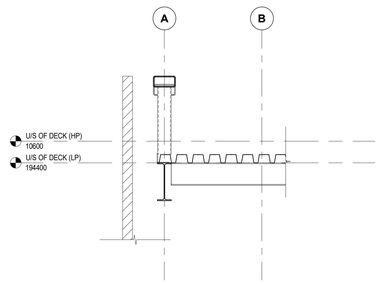

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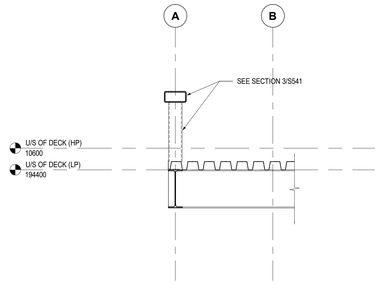

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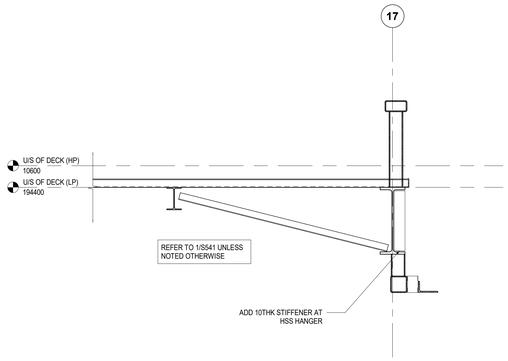

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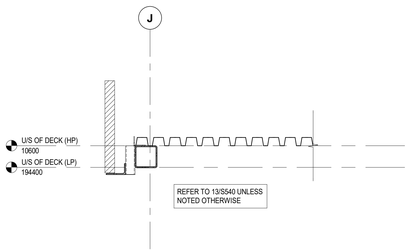

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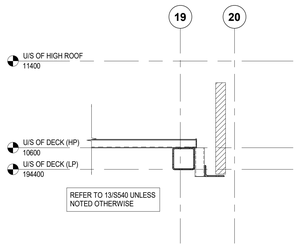

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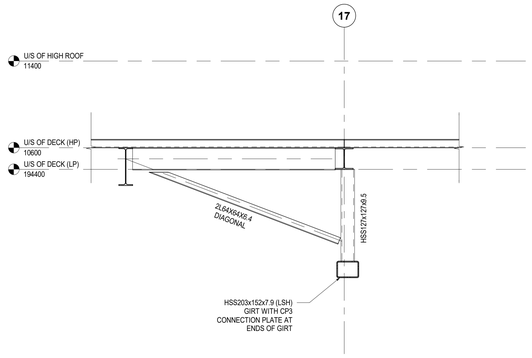

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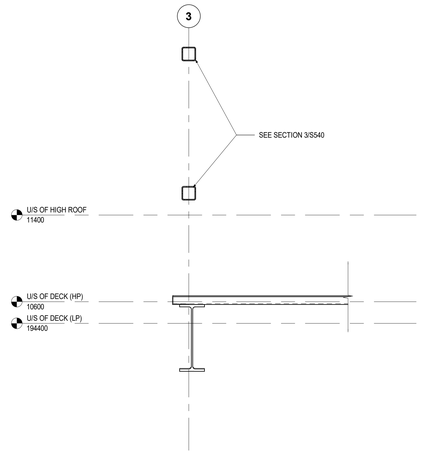

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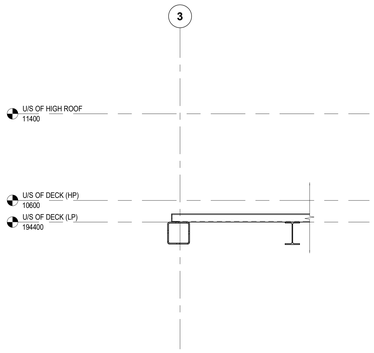

10 / S541 SECTION
1 : 20

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1	2026-07-08	ISSUED FOR TENDER

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ROOF FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S541
	Revision: 1

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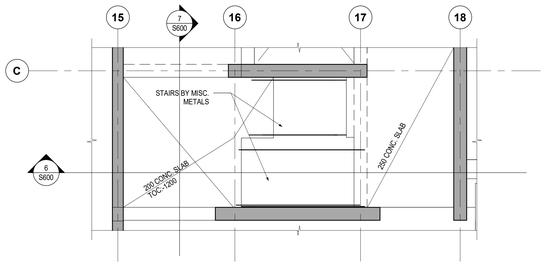
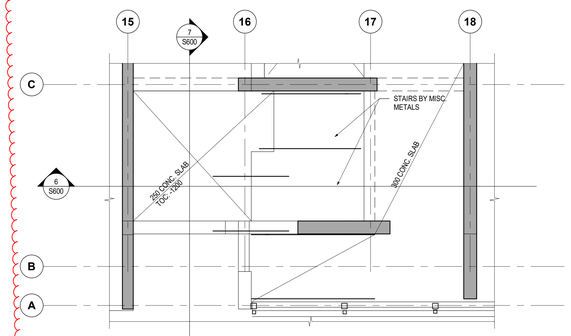
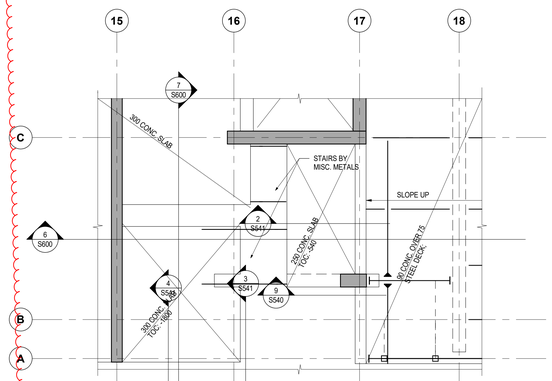


2 S600 GROUND - STAIR PLAN
1 : 50



3 S600 UPPER GROUND TO MEZZANINE - STAIR PLAN
1 : 50



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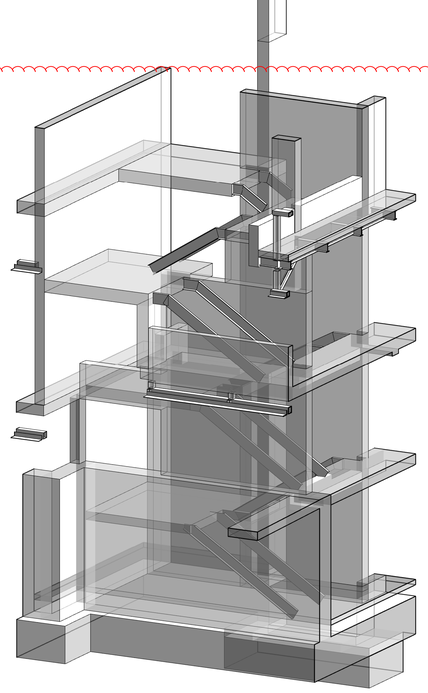
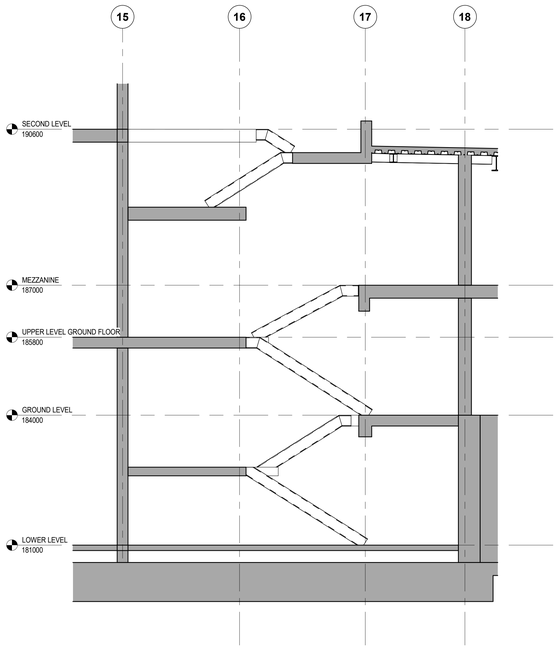
1 : 50



8 S600 STAIR 3D



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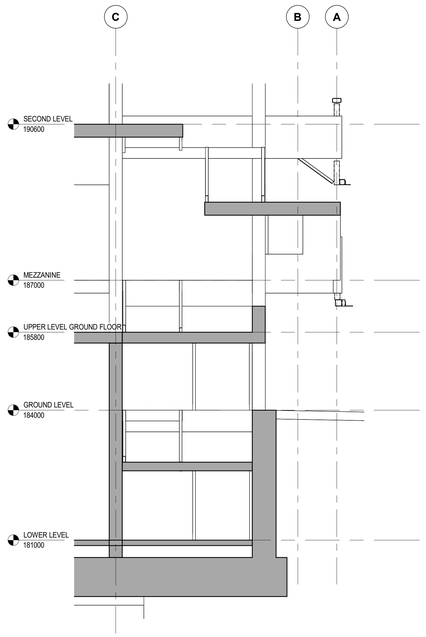

7 S600 SECTION
1 : 50

SEE UPCOMING ADDENDUM

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STAIR PLANS AND SECTIONS

scale:	1 : 50
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S600
	Revision: 2

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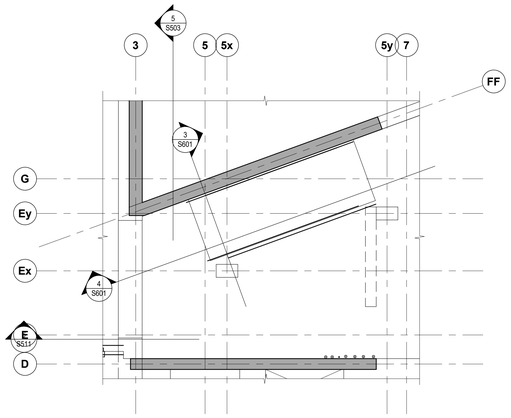

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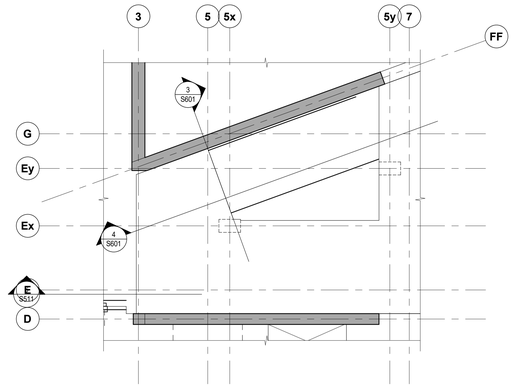
1 : 50



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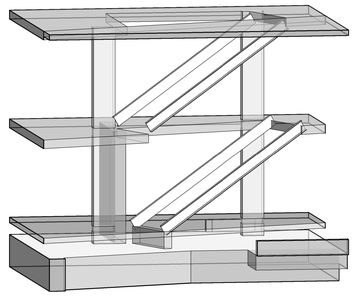
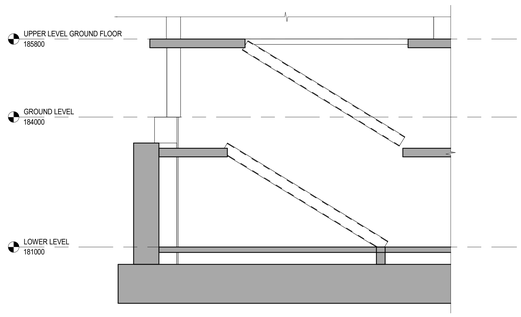
1 : 50



5 S601 STAIR 3D



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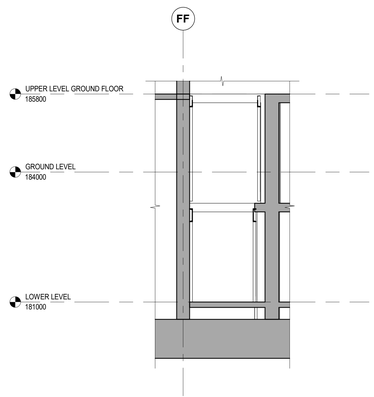

3 S601 SECTION
1 : 50

SEE UPCOMING ADDENDUM

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2026-07-08
E.U.CHOWDHURY
100072622
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STAIR PLANS AND SECTIONS

scale:	1 : 50
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S601
	Revision: 2

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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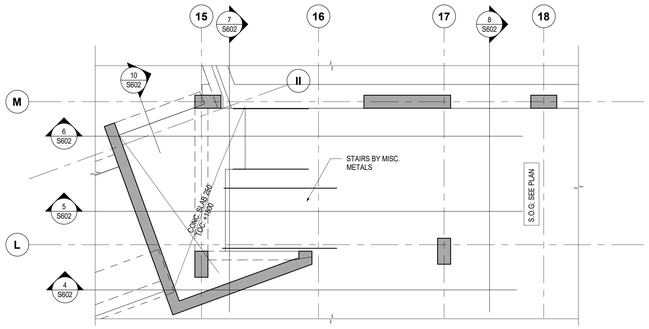

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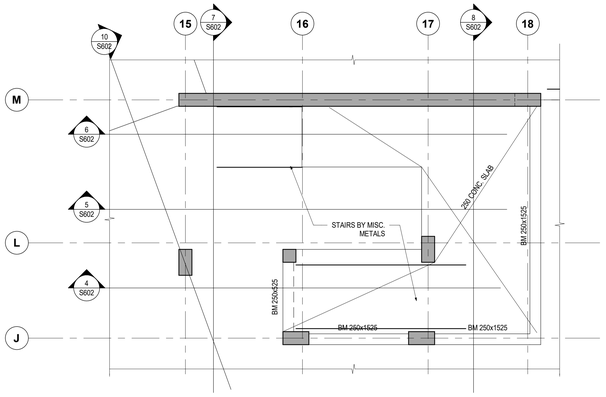
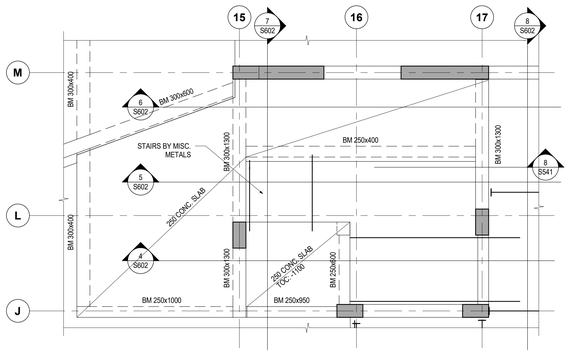


2 S602 MEZZANINE FLOOR STAIR PLAN
1 : 50



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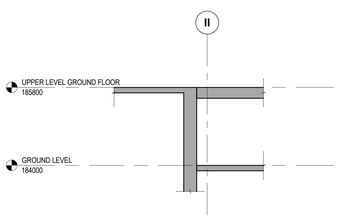

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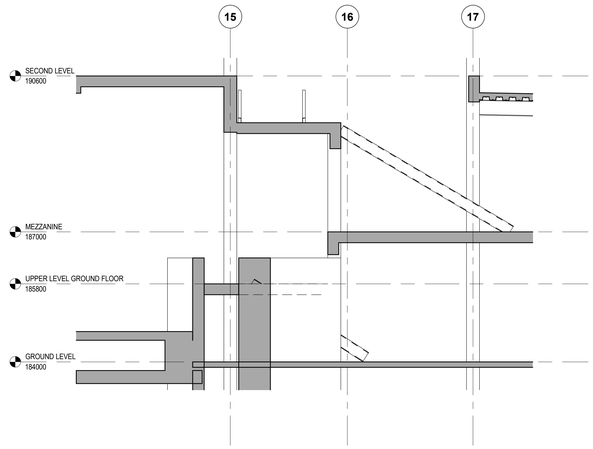

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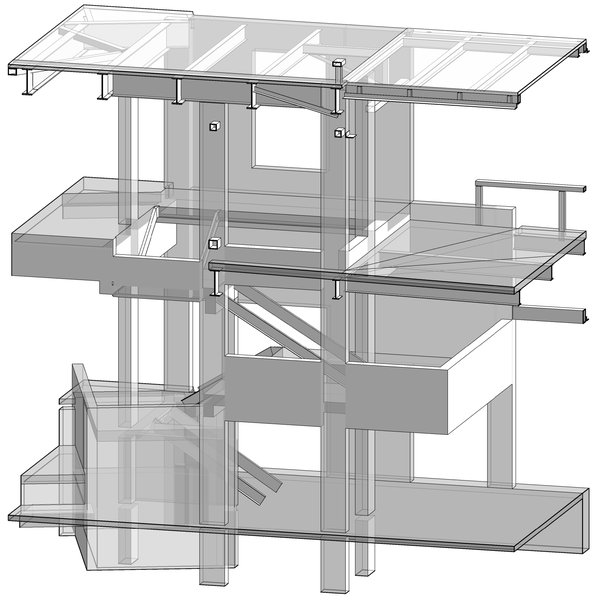

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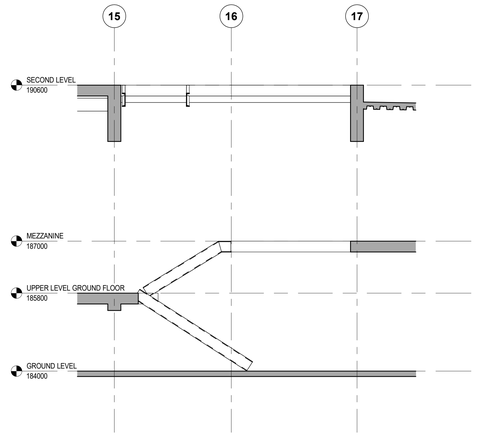

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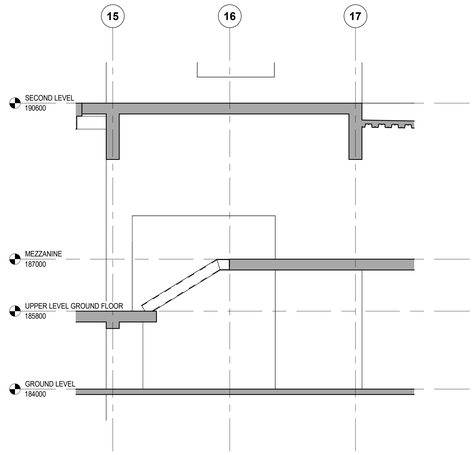

6 S602 SECTION
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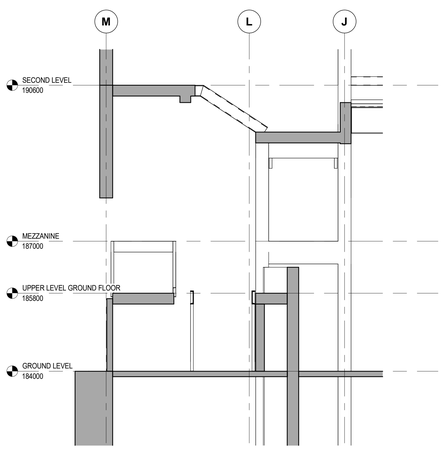
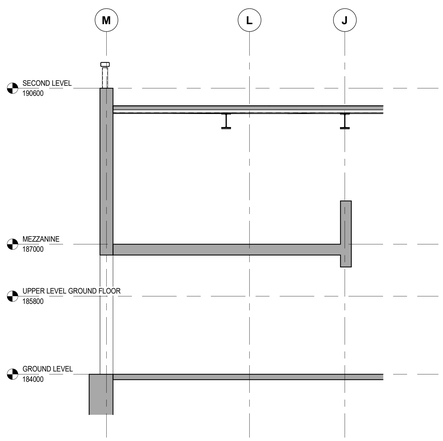


7 S602 SECTION
1 : 50



8 S602 SECTION
1 : 50

Key to Detail Location

NO. — Detail Number
NO. — Drawing Number

If this sheet is not 46 7/8" x 33 1/8" (1189 x 841 mm) It is a reduced print - Read dwg. accordingly.

Contractors must check and verify all dimensions on the job and report any discrepancies to the Architect before proceeding with the work.

All prints and specifications are the property of the Architect and must be returned at the completion of the work.

Drawings should not be scaled.

No	Date	Revision/Issued
1	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
2	2026-07-08	ISSUED FOR TENDER

consultants

architect	CS&P Architects Inc. 2345 Yonge St., Suite 200 Toronto, ON, M4P 2E5 Phone.416.482.5002 Fax.416.482.5040
civil engineer	SCS Consulting Group Ltd. 30 Centurian Drive, Suite 100 Markham, ON, L3R 8B8 Phone.905.475.1900 Fax.905.475.8335
structural engineer	LEA Consulting Ltd. 40 University Ave, Suite 503 Toronto, ON, M5J 1T1 Phone.905.470.0015
mechanical engineer	Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151
electrical engineer	Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151
landscape architect	Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

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STAIR PLANS AND SECTIONS

scale:	1 : 50
date:	2026-07-08
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S602
	Revision: 2

A0 FILENAME: Autodesk Docs://2102 Sterling Hall/22317_SHS-STR-R25.rvt

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